PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

PREPARED BY: JDG Jaredian Design Group — Architects, Engineers and Construction Managers

PREPARED FOR: GOVERNMENT OF THE UNITED STATES VIRGIN ISLANDS

WITH: OFFICE OF DISASTER RECOVERY

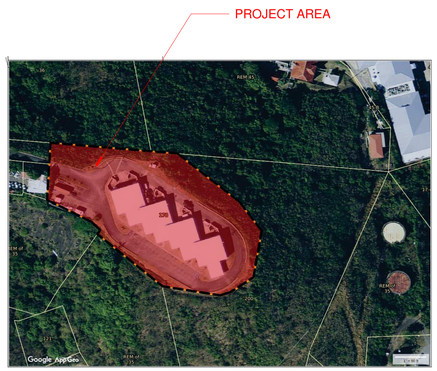
ECONOMIC DEVELOPMENT AUTHORITY

PROJECT NO.: 21-003.C

ISSUED FOR: 90% PROGRESS SET

ISSUE DATE: 02.14.2025

VICINITY MAP



PROJECT INFORMATION

OWNER: GOVERNMENT OF THE VIRGIN ISLANDS

LOCATION: NISKY 35 SOUTHSIDE QUARTER SUBBASE, CROWN BAY, STT, VI

JURISDICTION: ST. THOMAS, U.S. VIRGIN ISLANDS

ARCHITECT: JAREDIAN DESIGN GROUP

APPLICABLE CODES

BUILDING: 2021 INTERNATIONAL BUILDING CODE
FIRE PROTECTION: NFPA
MECHANICAL: 2021 IMC
ELECTRICAL: 2021 IEC
PLUMBING: 2021 IPC
ACCESSIBILITY:
ENERGY: 2021 IEEC
ICC 500 FOR SAFE ROOMS

PROJECT DESCRIPTION

PRIMARY CLASS USE:	MIXED USE
ZONING:	
CONSTRUCTION TYPE:	FRAMED CONSTRUCTION
NO. OF STORIES:	2
BUILDING HEIGHT:	41'-0"
BUILDING AREA:	12,231 SF GROSS AREA 10,841 SF NET AREA
SPRINKLED:	NO
AGE:	

TABLE 803.5 (IBC)
INTERIOR WALL & CEILING FINISH REQUIREMENTS BY OCCUPANCY

GROUP B: NON-SPRINKLED:
VERTICAL EXITS: 2
EXIT PASSAGEWAYS: 2
EXIT ACCESS CORRIDORS: 2
ROOMS & ENCLOSED SPACES:

PROJECT DIRECTORY

ARCHITECT:
JDG
JAREDIAN DESIGN GROUP

STRUCTURAL:
TLC ENGINEERING SOLUTIONS

MEP:
BW&A — Atlanta, Charlotte, Raleigh — Established 1987

CIVIL:
BUILDTEC

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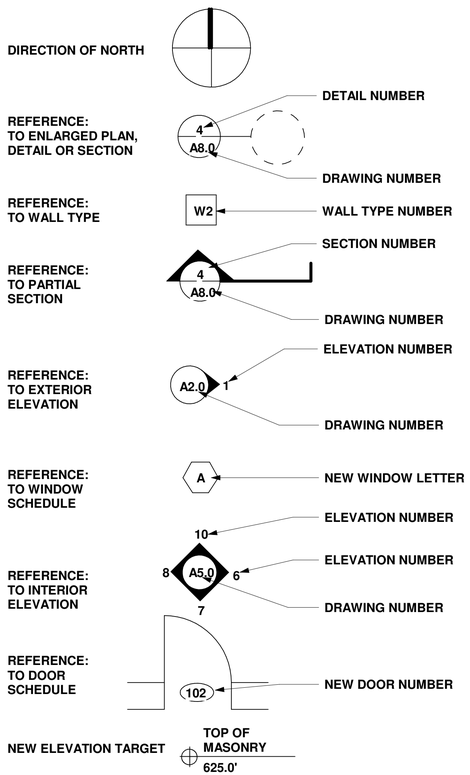
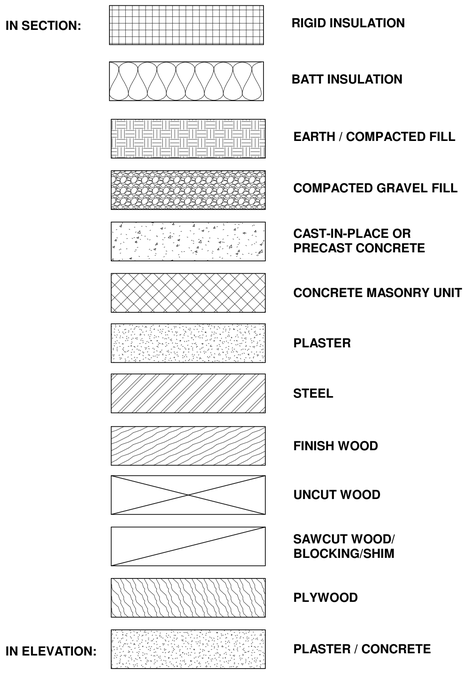
ARCHITECTURAL SYMBOLS:

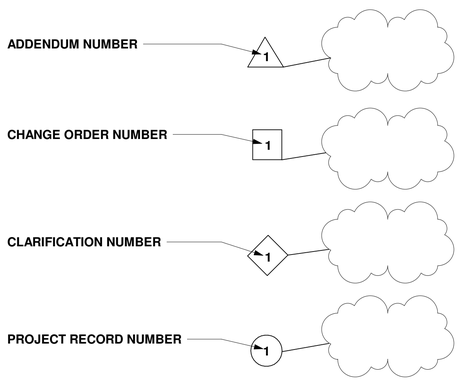


MATERIALS LEGEND:



ARCHITECTURAL ABBREVIATIONS:

ACT.	ACOUSTIC CEILING TILE
AES.	ARCHITECTURALLY EXPOSED STEEL
A.F.F.	ARCHITECTURALLY EXPOSED STEEL
B.T.W.	BOTTOM OF WALL
BLDG.	BUILDING
CONC.	CONCRETE
CMU.	CONCRETE MASONRY UNIT
CWB.	CEMENT WALL BOARD
DIA.	DIAMETER
EA.	EACH
EL.	ELEVATION
ES.	EXPOSED STRUCTURE
EQ.	EQUAL
EXIST.	EXISTING
F.F.	FINISH FLOOR
FIN.	FINISH
FLG.	FLOORING
FTG.	FOOTING
GALV.	GALVANIZED
GWB.	GYPSUM WALL BOARD
GYP.	GYPSUM
HOR.	HORIZONTAL
MTL.	METAL
MO.	MASONRY OPENING
O.C.	ON CENTER
O.D.	OUTSIDE DIAMETER
PLYW.	PLYWOOD
PNL.	PANEL
PNT.	PAINT
R.W.L.	RAIN WATER LEADER
PTD.	PAINTED
T.O.W.	TOP OF WALL
T.S.	TUBULAR STEEL
U.N.O.	UNLESS NOTED OTHERWISE
VBC.	VINYL BASECOVE
VCT.	VINYL COMPOSITION TILE
WD.	WOOD



REVISIONS

NO.	DATE	REMARKS

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DATE:	02.14.2025
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CHECKED BY:	J.P.W.
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	G0.1

DRAWING LIST & LIFE SAFETY

GENERAL DEMOLITION AND CONSTRUCTION NOTES

1. CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS AND BE FAMILIAR WITH ALL SYSTEMS BEFORE DEMOLITION. ALL DEMOLITION SHALL COMPLY WITH THE RULES & REGULATIONS OF THE FEDERAL, STATE, COUNTRY AND CITY GOVERNMENTS. EXISTING SERVICES SHALL BE MAINTAINED DURING DEMOLITION. ANY SHUTDOWN OF SYSTEMS MUST BE COORDINATED & APPROVED BY THE OWNER. CONTRACTOR SHOULD COORDINATE WORK WITH ALL TRADES & PROVIDE SUPERVISION WHILE WORK IS BEING PERFORMED.
2. THE CONTRACTOR SHALL VERIFY WITH THE OWNER, ALL MATERIALS TO BE RETAINED BY THE OWNER & DELIVER & STORE SAME WHERE DIRECTED BY THE OWNER. ALL REMAINING ITEMS BECOME THE PROPERTY OF THE CONTRACTOR & SHALL BE DISPOSED OF IN A LEGAL MANNER.
3. DO NOT INTERFERE WITH USE OF ADJACENT AREAS. MAINTAIN FREE & SAFE PASSAGE TO & FROM ADJACENT AREAS. PROVIDE, ERECT & MAINTAIN BARRICADES & LIGHTING AS REQUIRED BY APPLICABLE REGULATORY ADVISORY TO PROTECT OCCUPANTS OF THE BUILDING & WORKERS.
4. REMOVE EXISTING ELEMENTS AS INDICATED BY LEGEND. SURFACES OF EXISTING AREAS WHICH HAVE BEEN ALTERED BY DEMOLITION NOT SPECIFICALLY CALLED FOR SHALL BE REPAIRED & FINISHED TO MATCH THE ADJACENT SURFACES. PROTECT ALL EXISTING OR REMAINING SURFACES, FLOORS, WALLS & CEILINGS FROM AREAS OF DEMOLITION & NEW CONSTRUCTION.
5. CONTRACTOR SHALL FIELD VERIFY THE EXISTING CONDITIONS & NOTIFY THE ARCHITECT OF DISSIMILARITIES BEFORE PROCEEDING WITH DEMOLITION WORK. ALL WORK SHALL BE COORDINATED WITH DRAWINGS FOR NEW CONSTRUCTION.
6. CONTRACTOR SHALL FIELD VERIFY ALL DIMENSIONS PRIOR TO DEMOLITION OR CONSTRUCTION IN EXISTING AREAS
7. ERECT & MAINTAIN DUSTPROOF PARTITIONS AS REQUIRED TO PREVENT THE SPREAD OF DUST, FUMES & SMOKE TO OTHER PARTS OF THE BUILDING. MAINTAIN EXIT REQUIREMENTS, COORDINATE EXACT LOCATION WITH OWNER. ON COMPLETION OF WORK, REMOVE TEMPORARY PARTITIONS & REPAIR DAMAGED SURFACES TO MATCH ADJACENT SURFACES.
8. ARRANGE & PAY FOR DISCONNECTING, REMOVING & CAPPING UTILITY SERVICES WITHIN AREAS OF DEMOLITION. DISCONNECT & STUB-OFF, NOTIFY THE AFFECTED UTILITY COMPANY & OBTAIN APPROVAL BEFORE STARTING THIS WORK.
9. REMOVE FROM THE SITE ALL CONTAMINATED, VERMIN INFESTED OR DANGEROUS MATERIALS ENCOUNTERED & DISPOSE OF BY LEGAL MEANS. DO NOT ENDANGER THE HEALTH OF WORKERS OF THE PUBLIC.
10. THE OWNER RESERVES THE RIGHT TO SALVAGE ANY MATERIAL OR PORTION THEREOF FROM DEMOLISHED MATERIAL BEFORE IT IS DISPOSED OF.
11. ALL PLUMBING (POTABLE, SANITARY, ETC.) REQUIRED FOR OTHER FLOORS ABOVE OR BELOW SHALL NOT BE DISRUPTED OR REMOVED. CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY ALL SERVICES PRIOR TO REMOVAL OR DISCONNECTION OF ALL SERVICES (ELECTRICAL, HVAC, PLUMBING, SECURITY & TEL./DATA) FOR ADJOINING OCCUPIED SPACES SHALL BE MAINTAINED. CONTRACTOR TO MAKE MINOR ADJUSTMENTS AS REQUIRED TO MAINTAIN PLUMBING, ELECTRICAL & TEL./DATA TO OCCUPIED AREAS.
12. THE CONTRACTOR SHALL REMOVE & DISCARD ALL PLUMBING FIXTURES. THEY SHALL CAP ALL PIPES TEMPORALLY FOR RE-USE.
13. WHERE EXISTING WALLS ARE INDICATED TO BE REMOVED, PATCH INTERSECTING WALLS TO REMAIN FLUSH TO ACCEPT NEW FINISH.
14. CONTRACTOR SHALL REMOVE & DISCARD ALL DOORS, FRAMES, HARDWARE, ETC. UNLESS SPECIFICALLY NOTED OTHERWISE.
15. CONTRACTOR SHALL USE SELF LEVELING COMPOUND IN AREAS WHERE WALLS OR TILES HAVE BEEN REMOVED OR ALTERED IN ORDER TO MAINTAIN A LEVEL SURFACE THROUGHOUT. THEY MUST ALSO USE IT IN AREAS WHERE A TRANSITION IS NEEDED BETWEEN SURFACES THAT ARE DIFFERENT IN MATERIAL OR HEIGHT.
16. THE CONTRACTOR SHALL SURVEY THE EXISTING EQUIPMENT & FURNISHINGS & DISCARD ALL EQUIPMENT & FURNISHINGS NOT DESIGNATED BY OWNER OR OWNER'S REPRESENTATIVE TO BE SALVAGED.
17. EXISTING BASEBOARDS TO BE REMOVED AND DISCARDED.
18. CONTRACTOR SHALL REMOVE ALL EXISTING FLOOR TILES DOWN TO EXISTING FLOOR SLAB. THEY SHALL REPAIR AND PATCH EXISTING FLOOR AS REQUIRED FOR THE INSTALLATION OF NEW FLOOR TILES.

NEW CONSTRUCTION NOTES

1. THE INTENT OF THIS CONTRACT IS TO PROVIDE FOR THE CONSTRUCTION AND COMPLETION OF THE WORK DESCRIBED IN THE DRAWINGS. THE PRECISE DETAILS OF PERFORMING THE WORK ARE NOT STIPULATED EXCEPT AS CONSIDERED ESSENTIAL FOR THE SUCCESSFUL COMPLETION OF THE WORK. FURNISH ALL LABOR, MATERIAL, EQUIPMENT, TOOLS, TRANSPORTATION, AND SUPPLIES NECESSARY TO COMPLETE THE WORK ACCORDING TO THE CONTRACT.
2. THE CONTRACTOR SHALL NOTIFY THE ARCHITECT OF ALL PROPOSED SOURCES OF MATERIAL BEFORE DELIVERY TO THE PROJECT SITE TO EXPEDITE MATERIAL INSPECTION AND TESTING IF REQUIRED BY THE ARCHITECT. STORED MATERIAL PREVIOUSLY APPROVED FOR PAYMENT MAY AGAIN BE INSPECTED BY THE ARCHITECT FOR COMPLIANCE WITH APPLICABLE CONTRACT SPECIFICATION PRIOR TO BEING INCORPORATED INTO THE WORK.
3. THE CONTRACTOR IS RESPONSIBLE FOR THE SECURITY OF ALL STORED MATERIAL.
4. THE CONTRACTOR SHALL PROVIDE ALL SPACE OUTSIDE THE CONSTRUCTION LIMITS NEEDED FOR STORAGE OF MATERIAL. DO NOT USE PRIVATE PROPERTY FOR STORAGE WITHOUT WRITTEN PERMISSION OF THE OWNER OR LESSEE. FURNISH COPIES OF ALL SUCH AGREEMENTS TO THE ARCHITECT. ANY AREA PROVIDED BY THE OWNER FOR THE STORAGE OF MATERIAL SHALL BE RESTORED TO THEIR ORIGINAL CONDITION AT THE END OF THE CONTRACT.
5. THE EXISTING UTILITIES SHOWN ARE FOR INFORMATION ONLY. THERE MAY BE OTHER UTILITIES NOT SHOWN ON THESE PLANS, AND THE ARCHITECT ASSUMES NO RESPONSIBILITY FOR THEIR LOCATION. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE LOCATIONS IN THE FIELD WITHIN THE LIMITS OF THE WORK. ALL DAMAGE MADE TO EXISTING UTILITIES, WHETHER SHOWN OR NOT ON THE PLANS SHALL BE REPAIRED AT THE CONTRACTOR'S EXPENSE.
6. ALL EXCESS MATERIAL SHALL BE REMOVED FROM THE CONSTRUCTION SITE AND DISPOSED OF BY THE CONTRACTOR AT HIS OWN EXPENSE.
7. WORK THAT DOES NOT CONFORM TO THE CONTRACT, OR TO PREVAILING INDUSTRY STANDARDS WHERE NO SPECIFIC CONTRACT REQUIREMENTS ARE NOTED SHALL BE REMOVED AND REPLACED AT NO COST TO THE OWNER.
8. DO NOT DISTURB AREAS OUTSIDE OF THE LIMITS OF THE AREAS SHOWN ON THE PLANS. DISTURBANCES OUTSIDE THESE LIMITS SHALL BE REPAIRED AND/OR REPLACED AT CONTRACTOR'S EXPENSE.
9. THE CONTRACTOR SHALL PROVIDE DUST CONTROL AND STREET PROTECTION AT ALL TIMES. STREET SHALL NOT BE LEFT IN AN UNSAFE OR MUDDY CONDITION.
10. ALL WORK SHALL BE PERFORMED IN A FINISHED AND WORKMANLIKE MANNER TO THE SATISFACTION OF THE OWNER, AND IN ACCORDANCE WITH THE BEST RECOGNIZED TRADE PRACTICES.
11. THE ARCHITECT SHALL BE NOTIFIED A MINIMUM OF 48 HOURS PRIOR TO STARTUP OF CONSTRUCTION BY THE CONTRACTOR.
12. DEVIATION OF THESE PLANS AND SPECIFICATIONS WITHOUT THE PRIOR WRITTEN CONSENT OF THE ARCHITECT MAY BE CAUSE THE WORK TO BE REJECTED BY THE ARCHITECT.
13. CONTRACTOR WARRANTS THAT HIS EMPLOYEES, AGENTS, AND SUBCONTRACTORS POSSESS THE EXPERIENCE, KNOWLEDGE AND CHARACTER NECESSARY TO QUALIFY THEM INDIVIDUALLY FOR THE PARTICULAR CONSTRUCTION TECHNIQUES THEY PERFORM IN CONNECTION WITH THE TASKS TO BE PERFORMED UNDER THE CONTRACT. THE CONTRACTOR SHALL ENSURE THAT ALL CONSTRUCTION BE IN STRICT COMPLIANCE WITH ALL OSHA (OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION), AND LOCAL HEALTH AND SAFETY CODES.
14. THE CONTRACTOR SHALL CHECK, REVIEW, AND VERIFY ALL PLANS, DIMENSIONS, AND SITE CONDITIONS PRIOR TO CONSTRUCTION. ANY DISCREPANCIES OR OMISSIONS NOTED ON THE DRAWINGS OR IN THE SPECIFICATIONS OR ANY VARIATIONS NEEDED IN ORDER TO CONFORM TO CODE, RULES, AND REGULATIONS SHALL BE REPORTED IN WRITING TO THE ARCHITECT.
15. ALL MATERIALS AND CONSTRUCTION SHALL COMPLY WITH THE LATEST EDITIONS OF THE U.S. VIRGIN ISLANDS BUILDING CODE, THE ACI, AND THE ASTM.
16. THE PREMISES SHALL BE KEPT FROM ACCUMULATION OF WASTE MATERIAL AND DEBRIS, AND AT THE END OF THE JOB THE CONTRACTOR SHALL REMOVE ALL RUBBISH, SURPLUS MATERIAL, AND TOOLS AND LEAVE THE WORK AREA BROOM CLEAN.
17. ALL WORK SHALL BE DONE IN ACCORDANCE WITH THE INTERNATIONAL BUILDING CODE AND THE VIRGIN ISLANDS CODE.

REVISIONS

NO.	DATE	REMARKS

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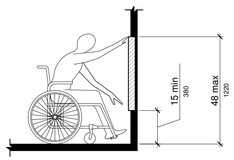
APPROVED BY:	J.P.W.

PROJECT NO.: 21-003.C
CONTRACT NO.: -

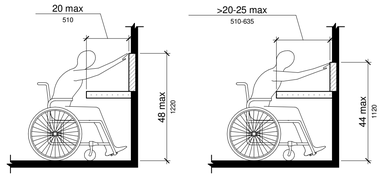
SHEET NO.: G0.2

GENERAL AND DEMOLITION NOTES

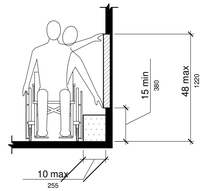
LICENSED ARCHITECT USA VIRGIN ISLANDS



UNOBSTRUCTED FORWARD REACH



OBSTRUCTED HIGH FORWARD REACH



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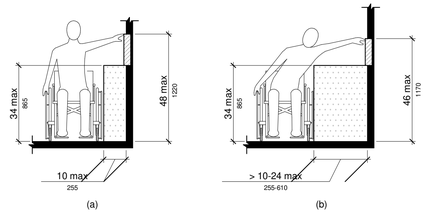
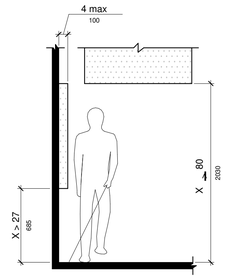
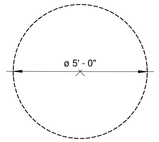


OBSTRUCTED HIGH FORWARD REACH

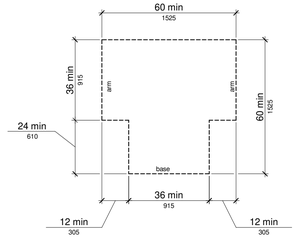
1 BUILDING BLOCKS
3/8" = 1'-0"



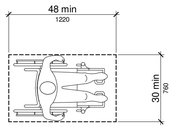
LIMIT OF PROTRUDING OBJECTS



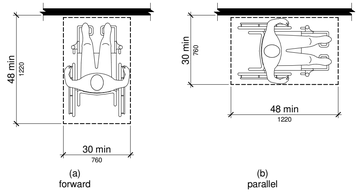
CIRCULAR TURNING SPACE



T-SHAPED TURNING SPACE



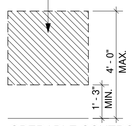
CLEAR FLOOR OR GROUND SPACE



POSITION OF CLEAR FLOOR OR GROUND SPACE

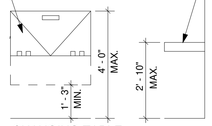
2 FLOOR CLEARANCES
3/8" = 1'-0"

FOR ADA ACCESS:
AREA OF UNOBSTRUCTED REACH FOR ALL OPERABLE CONTROLS AND AREAS SUBJECT TO REACH CONSTRAINTS OF WALL MOUNTED DEVICES TO BE LOCATED WITHIN THIS AREA



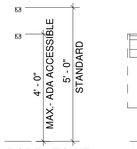
OPERABLE CONTROLS ZONE

MOUNT CHANGING TABLE SO THAT OPERABLE CONTROLS ARE BETWEEN 15" AND 48" MAX, AND SO THAT WHEN CHANGING TABLE IS IN OPEN POSITION THAT TABLE SURFACE DOES NOT EXCEED 2'-10" A.F.F. MAX

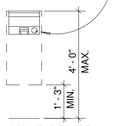


CHANGING TABLE

MOUNT DRYER SO THAT OPERABLE CONTROLS ARE BETWEEN 15" MIN. & 48" MAX A.F.F.

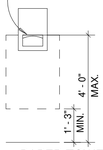


ROBE / TOWEL HOOK

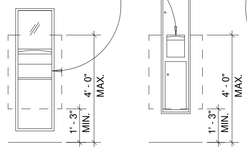


HAND DRYER

MOUNT PAPER TOWEL DISPENSER SO THAT OPERABLE CONTROLS, TOWEL DISPENSER AND WASTE RECEPTACLE OPENING ARE BETWEEN 15" MIN. & 48" MAX A.F.F.

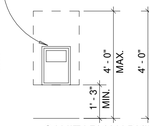


PAPER TOWEL DISPENSER

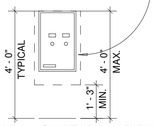


PAPER TOWEL DISPENSER & WASTE RECEPTACLE

MOUNT SANITARY NAPKIN ACCESSORIES SO THAT OPERABLE CONTROLS, DISPENSER OUTLETS AND WASTE RECEPTACLESARE BETWEEN 15" MIN & 48" MAX. A.F.F.



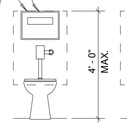
SANITARY NAPKIN DISPOSAL



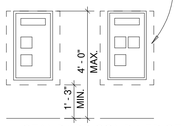
SANITARY NAPKIN DISPENSER

MOUNT SEAT COVER ACCESSORIES SO THAT OPERABLE CONTROLS, AND DISPENSER OUTLETS ARE BETWEEN 15" MIN & 48" MAX. A.F.F.

CENTER DISPENSER ABOVE TOILET

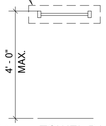


STANDARD SEAT COVER DISPENSER

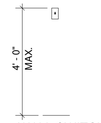


STANDARD COMBINED SEAT COVER & TOILET TISSUE DISPENSERS

BLOCKING SUPPORT (TYP)

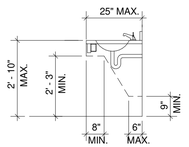


TOWEL BAR

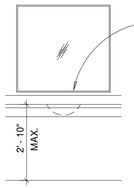


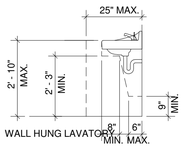
WALL SWITCH

3 MOUNTING HEIGHTS
3/8" = 1'-0"

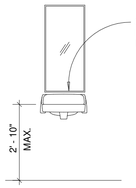


COUNTER WITH
DROP IN LAVATORY

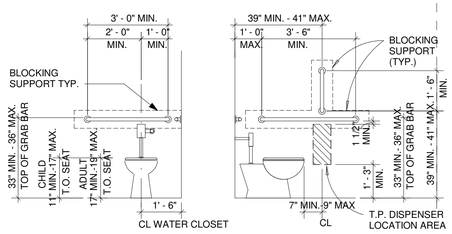




WALL HUNG LAVATORY

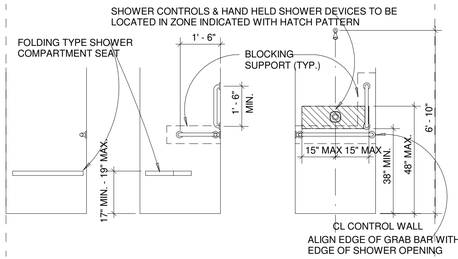


4 ADA SINK & VANITY
3/8" = 1'-0"



ADA TOILET & GRAB BARS - FLOOR MTD.

5 ADA TOILET & GRAB BAR
3/8" = 1'-0"



SHOWER / ADA SHOWER

6 SHOWER/ ADA SHOWER
3/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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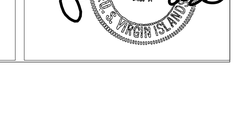
Post Office Box 6218
St. Thomas, U.S. Virgin Islands 00804-6218
telephone: 340.777.1600
e-mail: admin@jaredian.com
website: www.jaredian.com

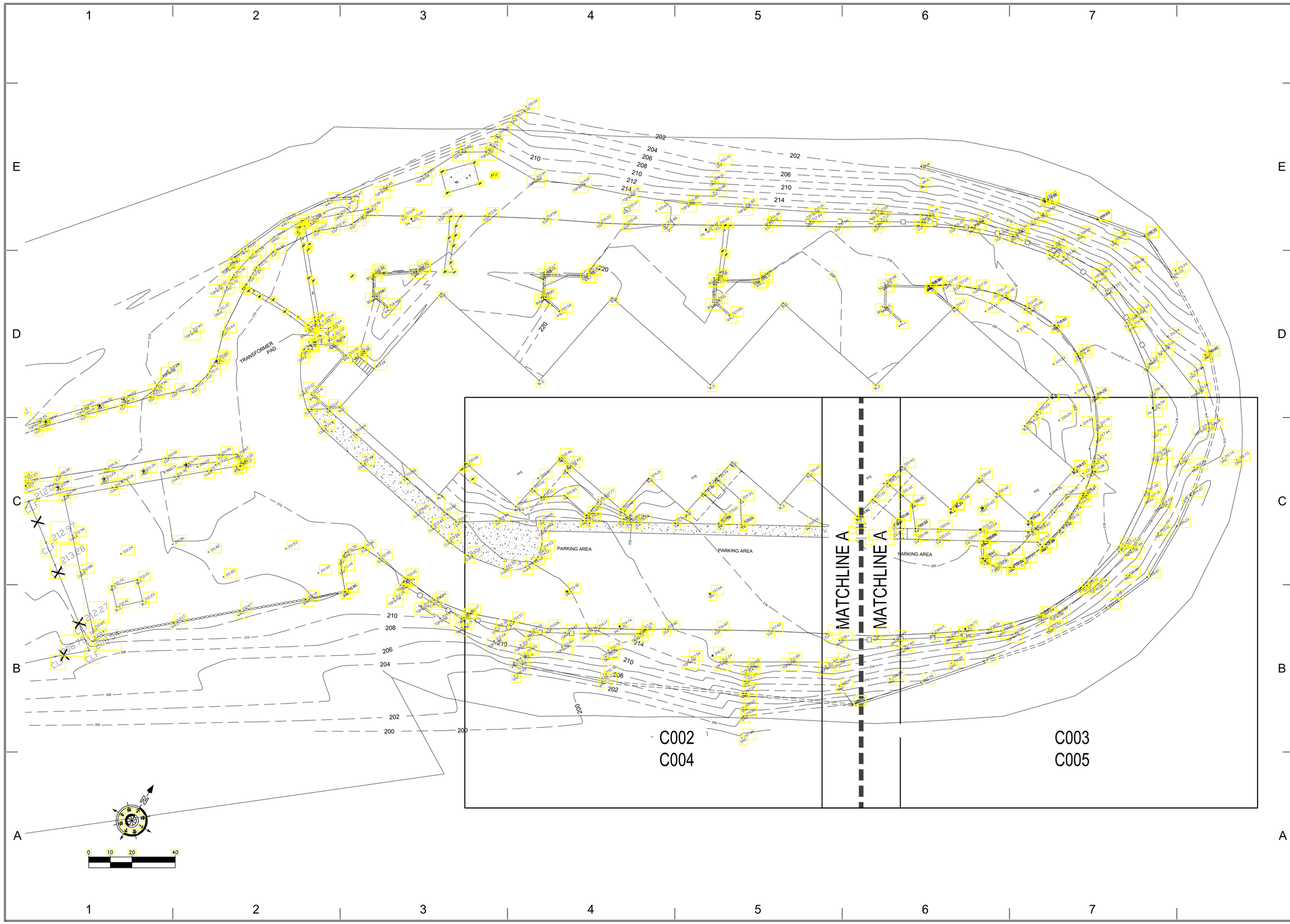
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W.
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	G0.3

TYPICAL DETAILS





REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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P.O. BOX 8269
Christiansted, USVI 00823

Jaredian Design Group
Architects, Engineers and Construction Managers

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e-mail: jarediandesigngroup@gmail.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

DATE:	10.06.2023
DRAWN BY:	DC
CHECKED BY:	DC
APPROVED BY:	DC

PROJECT NO.:

CONTRACT NO.:

SHEET NO.:

C001

EXISTING CONDITION PLAN
&
KEY PLAN

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-IB
U.S. VIRGIN ISLANDS

SAWCUT AT SIDEWALK DEMOLITION LIMITS

ALL EXISTING PARKING STRIPING IN THIS AREA TO BE REMOVED AND RE-STRIPED AS PER PROPOSED SITE IMPROVEMENTS

BEGIN GUARDRAIL DEMOLITION

EXISTING GUARDRAIL TO BE DEMOLISHED

BEGIN GUARDRAIL DEMOLITION

EXISTING GUARDRAIL TO BE DEMOLISHED

PARKING AREA

PARKING AREA

PARKING AREA

MATCHLINE A

MATCHLINE A

FFE

FFE

DEMOLITION LEGEND

- DISTURBED SITE / DEMOLITION LIMITS
- GENERAL CLEARING AND GRUBBING
- ASPHALTIC CONCRETE PAVEMENT DEMOLITION
- CONCRETE PAVEMENT / SIDEWALK DEMOLITION
- GUARDRAIL DEMOLITION

NOTES:

1. COORDINATE THIS PLAN SHEET WITH THE ARCHITECTURAL, STRUCTURAL AND M.E.P. DRAWINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH THE WORKS SHOWN ON THESE PLANS.
2. COORDINATE THIS PLAN SHEET WITH THE PROPOSED IMPROVEMENTS SHOWN IN THE CIVIL SHEET SET. NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH THE WORKS SHOWN ON THESE PLANS.
3. ALL DEMOLISHED MATERIALS SHALL BE LEGALLY DISPOSED OF.
4. INSTALL ALL STORMWATER POLLUTION PREVENTION DEVICES AND FOLLOW BEST MANAGEMENT PRACTICES (BMPs) PRIOR TO COMMENCING DEMOLITION ACTIVITIES. REFER TO DETAILS 1 & 2 ON SHEET C007.

0 5 10 20

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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e-mail: jarediandesigngroup@gmail.com
website: www.jarediandesigngroup.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

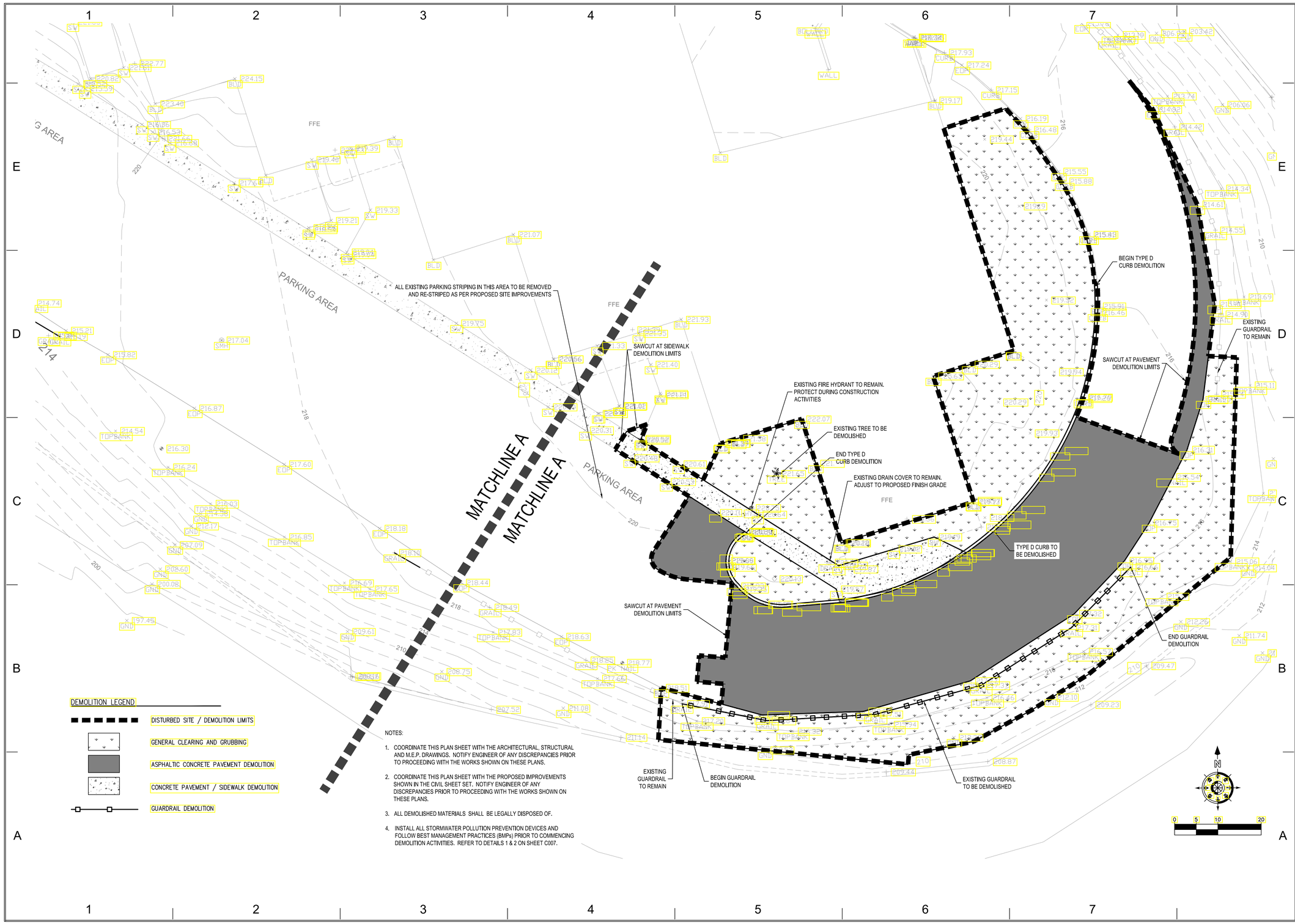
DATE: 10.06.2023
DRAWN BY: DC
CHECKED BY: DC
APPROVED BY: DC

PROJECT NO.:
CONTRACT NO.:

SHEET NO.:
C002

DEMOLITION PLAN

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-IB
U.S. VIRGIN ISLANDS



NOTES:

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3. ALL DEMOLISHED MATERIALS SHALL BE LEGALLY DISPOSED OF.
4. INSTALL ALL STORMWATER POLLUTION PREVENTION DEVICES AND FOLLOW BEST MANAGEMENT PRACTICES (BMPs) PRIOR TO COMMENCING DEMOLITION ACTIVITIES. REFER TO DETAILS 1 & 2 ON SHEET C007.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

BUILDTEC
P.O. BOX 8269
Christiansted, USVI 00823

Jaredian Design Group
Architects, Engineers and Construction Managers
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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

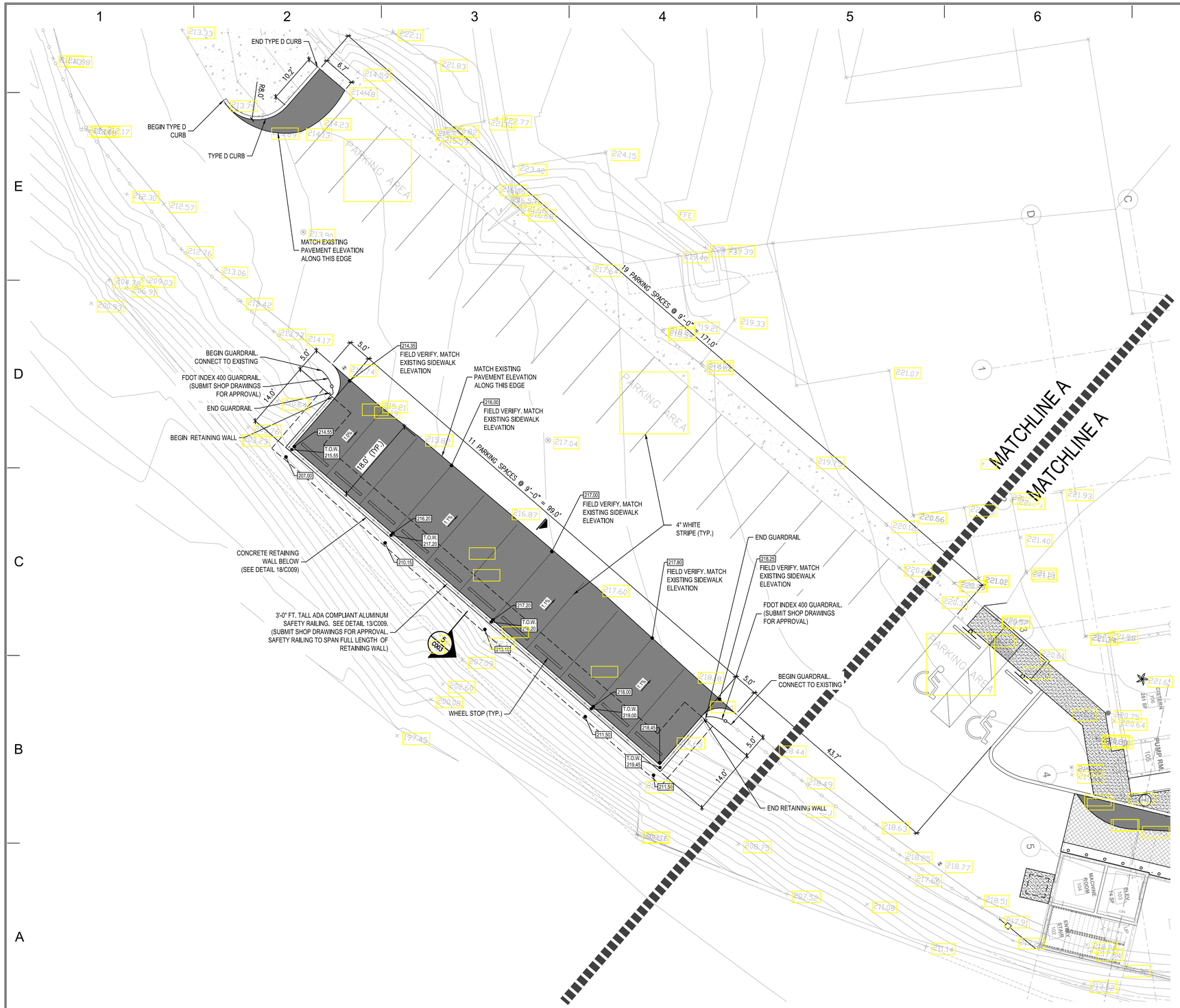
DATE: 10.06.2023
DRAWN BY: DC
CHECKED BY: DC
APPROVED BY: DC

PROJECT NO.:
CONTRACT NO.:

SHEET NO.: C003

DEMOLITION PLAN

COUNCIL OF REGISTERED ENGINEERS INTERNATIONAL
DAMIAN CARTWRIGHT
28851-1B



NOTES:

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2. COORDINATE THIS SHEET WITH DEMOLITION PLAN SHEETS C002 & C003.
3. ALL DIRT/GREEN AREAS TO REMAIN THAT WERE DISTURBED AS A RESULT OF PROPOSED IMPROVEMENTS SHALL BE HYDRO-SEEDED AT THE END OF THE PROJECT AND WATERED ON A REGULAR BASIS FOR A MINIMUM OF 1 MONTH THEREAFTER OR UNTIL THE GRASS HAS CAUGHT

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

DATE:	10.06.2023
DRAWN BY:	DC
CHECKED BY:	DC
APPROVED BY:	DC

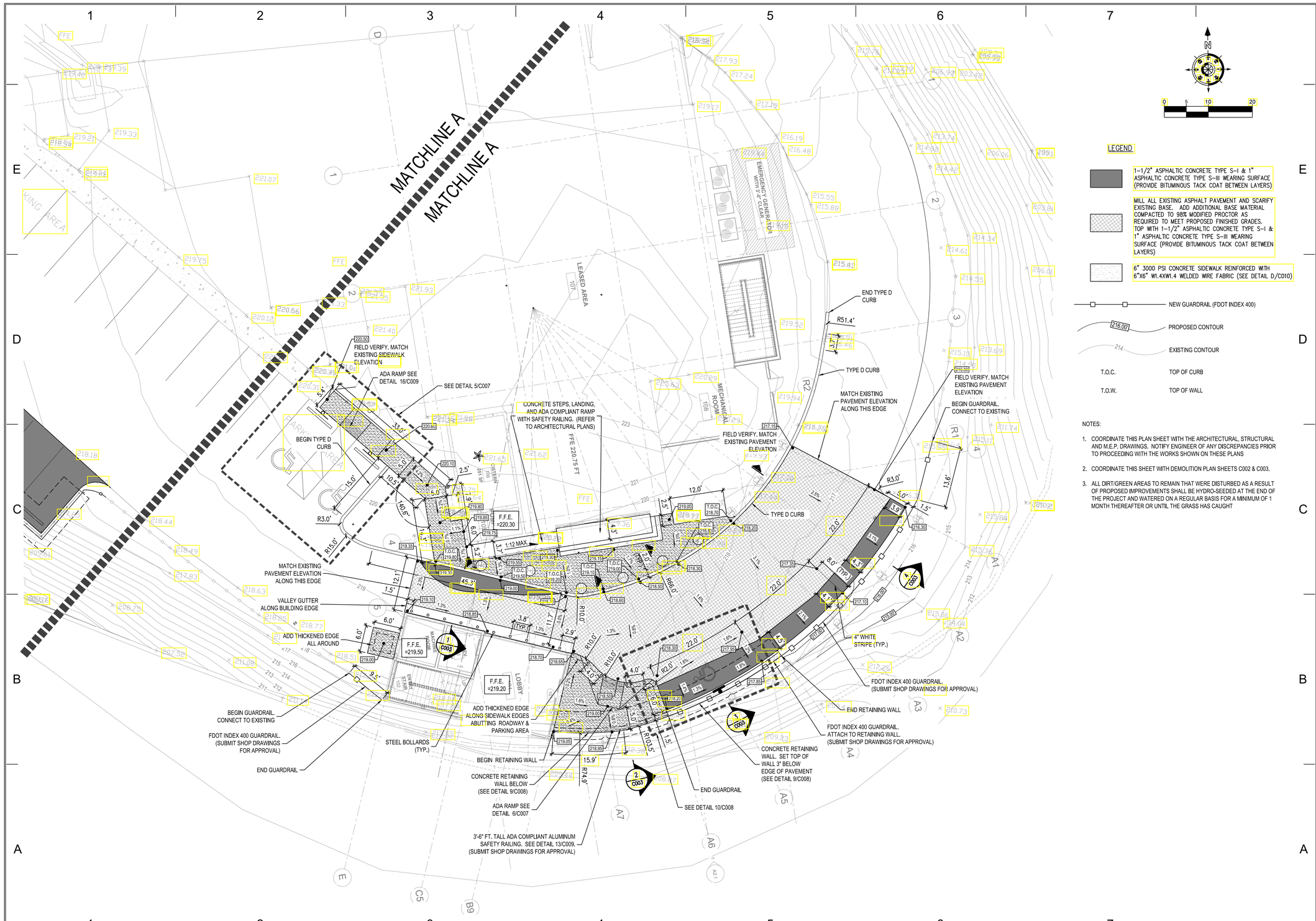
PROJECT NO.:

CONTRACT NO.:

SHEET NO.:
SITE, GEOMETRY AND PAVING, GRADING & DRAINAGE PLAN

C004

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-IB
U.S. VIRGIN ISLANDS



0 5 10 20

LEGEND

	1-1/2" ASPHALTIC CONCRETE TYPE S-I & 1" ASPHALTIC CONCRETE TYPE S-III WEARING SURFACE (PROVIDE BITUMINOUS TACK COAT BETWEEN LAYERS)
	MILL ALL EXISTING ASPHALT PAVEMENT AND SCARIFY EXISTING BASE. ADD ADDITIONAL BASE MATERIAL COMPACTED TO 98% MODIFIED PROCTOR AS REQUIRED TO MEET PROPOSED FINISHED GRADES. TOP WITH 1-1/2" ASPHALTIC CONCRETE TYPE S-I & 1" ASPHALTIC CONCRETE TYPE S-III WEARING SURFACE (PROVIDE BITUMINOUS TACK COAT BETWEEN LAYERS)
	6" 3000 PSI CONCRETE SIDEWALK REINFORCED WITH 6"X6" W1.4XW1.4 WELDED WIRE FABRIC (SEE DETAIL D/C010)
	NEW GUARDRAIL (FDOT INDEX 400)
216.00	PROPOSED CONTOUR
214	EXISTING CONTOUR
T.O.C.	TOP OF CURB
T.O.W.	TOP OF WALL

NOTES:

1. COORDINATE THIS PLAN SHEET WITH THE ARCHITECTURAL, STRUCTURAL AND M.E.P. DRAWINGS. NOTIFY ENGINEER OF ANY DISCREPANCIES PRIOR TO PROCEEDING WITH THE WORKS SHOWN ON THESE PLANS
2. COORDINATE THIS SHEET WITH DEMOLITION PLAN SHEETS C002 & C003.
3. ALL DIRT/GREEN AREAS TO REMAIN THAT WERE DISTURBED AS A RESULT OF PROPOSED IMPROVEMENTS SHALL BE HYDRO-SEEDED AT THE END OF THE PROJECT AND WATERED ON A REGULAR BASIS FOR A MINIMUM OF 1 MONTH THEREAFTER OR UNTIL THE GRASS HAS CAUGHT

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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e-mail: jarediandesigngroup@gmail.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

DATE:	10.06.2023
DRAWN BY:	DC
CHECKED BY:	DC
APPROVED BY:	DC

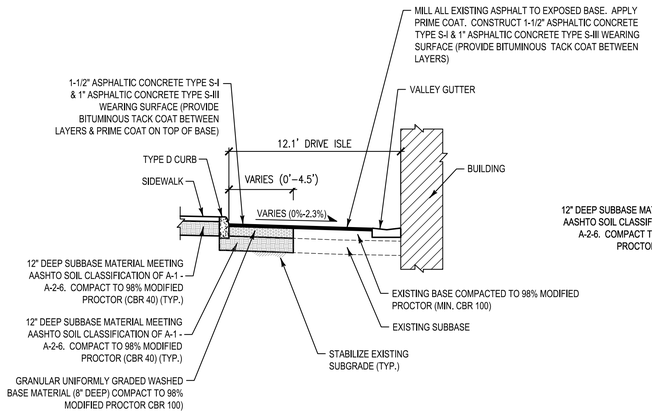
PROJECT NO.:

CONTRACT NO.:

SHEET NO.:
SITE, GEOMETRY AND PAVING, GRADING & DRAINAGE PLAN

C005

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-1B
U.S. VIRGIN ISLANDS



TYPICAL SECTION 1
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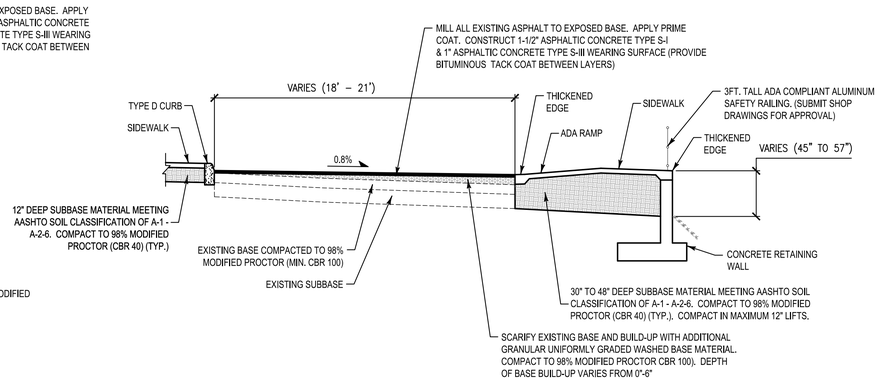

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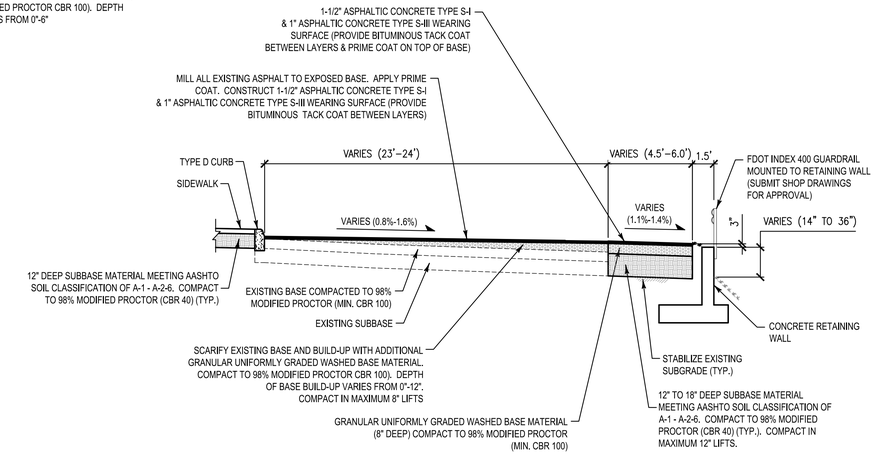

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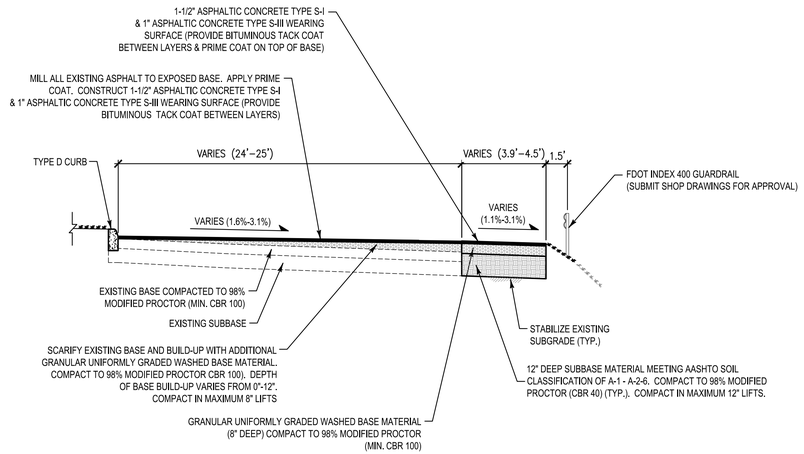

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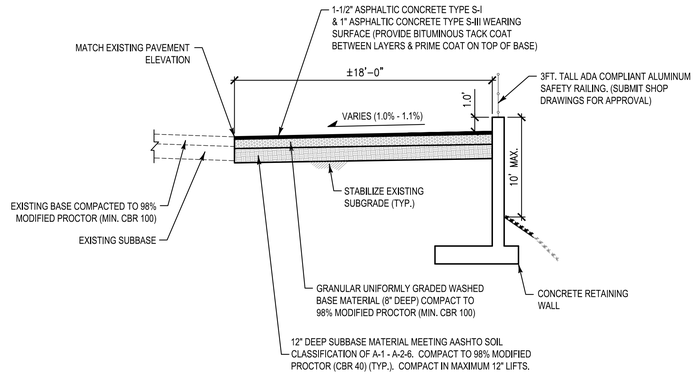


TYPICAL SECTION 5
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

BUILDTEC
P.O. BOX 8269
Christiansted, USVI 00823

Jaredian Design Group
Architects, Engineers and Construction Managers
5143 Palm Passage,
Second Floor - Suites B24-25 St. Thomas,
US Virgin Islands 00802
Post Office Box 6218
St. Thomas, U.S. Virgin Islands 00804-6218
telephone: 340.777.1600
e-mail: jarediandesigngroup@gmail.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

DATE:	10.06.2023
DRAWN BY:	DC
CHECKED BY:	DC
APPROVED BY:	DC

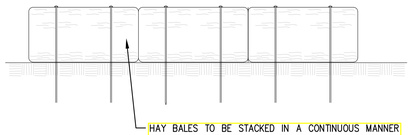
PROJECT NO.:

CONTRACT NO.:

SHEET NO.:
C006

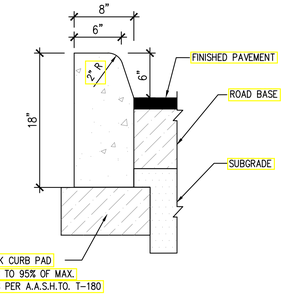
TYPICAL SECTIONS

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-1B
U.S. VIRGIN ISLANDS



NOTE:
TO BE USED AT ALL DOWNSTREAM AREAS WHERE THERE IS A LIKELIHOOD OF DEGRADATION DUE TO EROSION.

1 HAY BALE SEDIMENT BARRIER DETAIL
N.T.S.



3 TYPE "D" CURB
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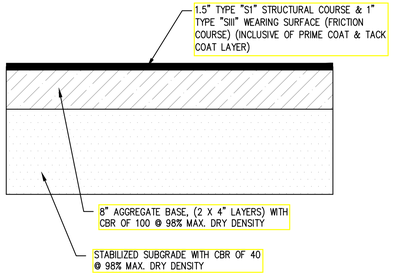
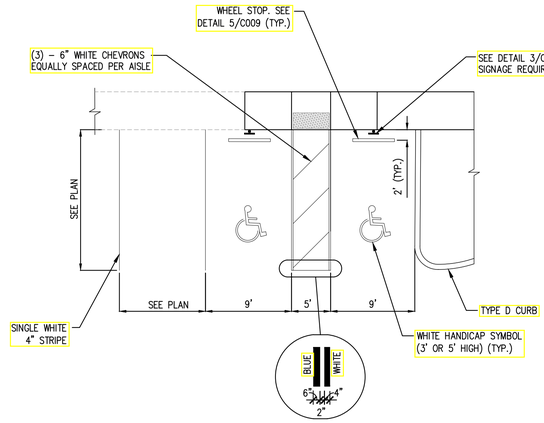
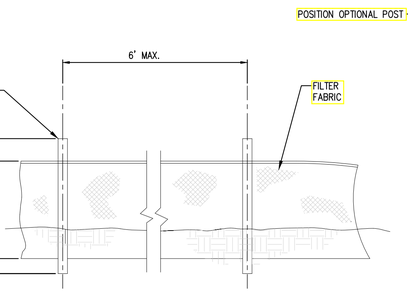
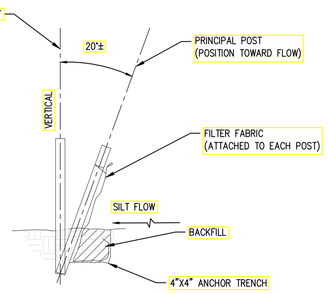


4 PAVEMENT DETAIL
N.T.S.



5 HANDICAP RAMP
N.T.S.





STEEL 1.33 LBS/FT MIN.)
POST OPTIONS: WOOD 2"X4" OR 2 1/2" DIA.

2 SILT FENCE (FDOT INDEX No. 102)
N.T.S.

NOTE:

1. CURB RAMP RUNNING SLOPES SHALL NOT BE STEEPER THAN 1:12 AND CROSS SLOPE SHALL BE 0.02 OR FLATTER.

2. DETECTABLE WARNING SURFACES SHALL BE CONSTRUCTED BY TEXTURING OR APPLYING A TRUNCATED DOME MATERIAL IN CONFORMANCE WITH THE U.S. DEPARTMENT OF JUSTICE A.D.A. STANDARDS FOR ACCESSIBLE DESIGN, A.D.A. ACCESSIBILITY GUIDELINES, SECTION 4.29.2. TRANSITION SLOPES ARE NOT TO HAVE DETECTABLE WARNINGS.

3. WHERE A CURB RAMP IS CONSTRUCTED WITHIN AN EXISTING CURB, CURB AND GUTTER AND/OR SIDEWALK, THE EXISTING CURB OR CURB AND GUTTER SHALL BE REMOVED TO THE NEAREST JOINT BEYOND THE CURB TRANSITIONS OR TO THE EXTENT THAT NO REMAINING SECTION OF CURB OR CURB AND GUTTER IS LESS THAN 5' LONG. THE EXISTING SIDEWALK SHALL BE REMOVED TO THE NEAREST JOINT BEYOND THE TRANSITION SLOPE WALK AROUND OR TO THE EXTENT THAT NO REMAINING SECTION OF SIDEWALK IS LESS THAN 5'.

4. WHERE ADJACENT WALKING SURFACES ARE COLORED OR ARE CONSTRUCTED WITH MATERIALS OTHER THAN STANDARD CLASS I PORTLAND CEMENT CONCRETE IN ACCORDANCE WITH SECTION 522 OF THE SPECIFICATIONS, THE PLAN MUST PROVIDE FOR DETECTABLE WARNING SURFACE COLORS OR MATERIALS THAT PROVIDE THE NECESSARY CONTRAST, EITHER DARK-ON-LIGHT OR LIGHT-ON-DARK. (SAFETY YELLOW OR BRICK RED)

5. RECOMMENDED MANUFACTURER: ENGINEERED PLASTICS, INC. (ARMOR TILE) OLYMPIC TOWERS, 300 PEARL STREET, SUITE 200, BUFFALO, NY 14202. PHONE: (800) 682-2525

EROSION/TURBIDITY CONTROL NOTES

1. THE INSTALLATION OF TEMPORARY EROSION CONTROL BARRIERS SHALL BE COORDINATED WITH THE CONSTRUCTION OF THE PERMANENT EROSION CONTROL FEATURES TO THE EXTENT NECESSARY TO ASSURE ECONOMICAL, EFFECTIVE AND CONTINUOUS CONTROL OF EROSION AND WATER POLLUTION THROUGHOUT THE LIFE OF THE CONSTRUCTION PHASE.

2. THE TYPE OF EROSION CONTROL BARRIERS USED SHALL BE GOVERNED BY THE NATURE OF THE CONSTRUCTION OPERATION AND SOIL TYPE THAT WILL BE EXPOSED. SILTY AND CLAYEY MATERIAL USUALLY REQUIRE SOLID SEDIMENT BARRIERS TO PREVENT TURBID WATER DISCHARGE, WHILE SANDY MATERIAL MAY NEED ONLY SILT SCREENS OR HAY BALES TO PREVENT EROSION. FLOATING TURBIDITY CURTAINS SHALL BE USED IN OPEN WATER SITUATIONS. DIVERSION DITCHES OR SWALES MAY BE REQUIRED TO PREVENT TURBID STORMWATER RUNOFF FROM BEING DISCHARGED TO WETLANDS OR OTHER WATER BODIES. IT MAY BE NECESSARY TO EMPLOY A COMBINATION OF BARRIERS, DITCHES AND OTHER EROSION/TURBIDITY CONTROL MEASURES IF CONDITIONS WARRANT.

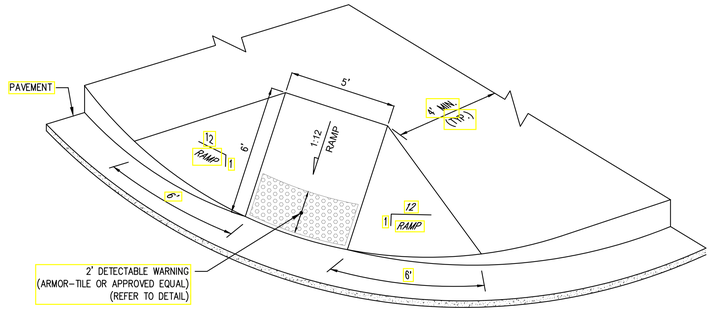
3. CONSTRUCTION OPERATIONS IN OR ADJACENT TO WETLANDS SHALL BE RESTRICTED TO THOSE AREAS IDENTIFIED IN THE PLANS AND IN THE SPECIFICATIONS.

4. EXCEPT AS NECESSARY FOR CONSTRUCTION, EXCAVATED MATERIAL SHALL NOT BE DEPOSITED IN THE WETLANDS OR IN A POSITION CLOSE ENOUGH THERETO TO BE WASHED AWAY BY HIGH WATER OR RUNOFF.

5. WHERE PUMPS ARE TO BE USED TO REMOVE TURBID WATERS FROM CONSTRUCTION AREAS, THE WATER SHALL BE TREATED PRIOR TO DISCHARGE TO THE WETLANDS. TREATMENT METHODS INCLUDE AND ARE NOT LIMITED TO, TURBID WATER BEING PUMPED INTO GRASSED SWALES OR APPROPRIATE VEGETATED AREAS, SEDIMENT BASINS, OR CONFINED BY AN APPROPRIATE ENCLOSURE SUCH AS TURBIDITY BARRIERS, AND KEPT CONFINED UNTIL ITS TURBIDITY LEVEL MEETS STATE WATER QUALITY STANDARDS.

6. THE CONTRACTOR SHALL SCHEDULE HIS OPERATIONS SUCH THAT THE AREA OF UNPROTECTED ERODIBLE EARTH EXPOSED AT ANY ONE TIME IS NOT LARGER THAN THE MINIMUM AREA NECESSARY FOR EFFICIENT CONSTRUCTION OPERATIONS, AND THE DURATION OF EXPOSED, UNCOMPLETED CONSTRUCTION TO THE ELEMENTS SHALL BE AS SHORT AS PRACTICABLE. CLEARING AND GRUBBING SHALL BE SO SCHEDULED AND PERFORMED THAT GRADING OPERATIONS CAN FOLLOW IMMEDIATELY THEREAFTER, AND GRADING OPERATIONS SHALL BE SCHEDULED AND PERFORMED THAT PERMANENT EROSION CONTROL FEATURES CAN FOLLOW IMMEDIATELY THEREAFTER IF CONDITIONS ON THE PROJECT PERMIT.

7. THE CONTRACTOR SHALL PROVIDE ROUTINE MAINTENANCE OF PERMANENT AND TEMPORARY EROSION CONTROL FEATURES UNTIL THE PROJECT IS COMPLETE AND ALL ALTERED SOILS ARE STABILIZED.



NOTE 1:
REFERENCE FDOT INDEX 304, PAGE 1, GENERAL AND DESIGN NOTES FOR RAMPS IN R.O.W.

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6 HANDICAP RAMP
N.T.S.

REVISIONS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
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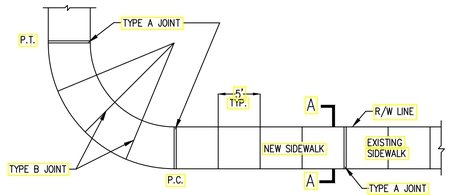
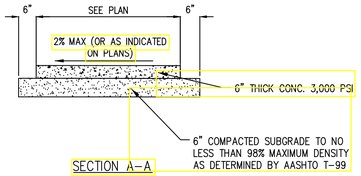
PAVING, GRADING AND DRAINAGE DETAILS

C007

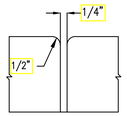
PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28895I-19
REGISTERED VIRGIN ISLANDS U.S.A.



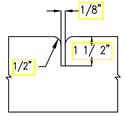
PLAN VIEW



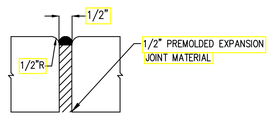
SECTION A-A



TYPE A



TYPE B



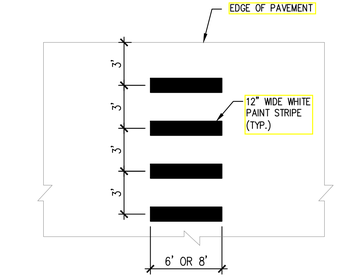
TYPE C

TABLE OF SIDEWALK THICKNESS - T	
LOCATION	
STANDARD SIDEWALK	6"

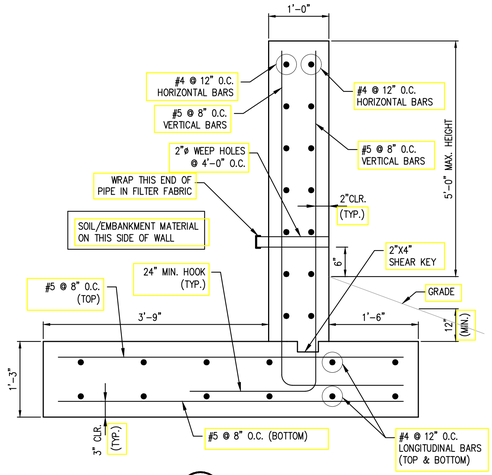
TABLE OF SIDEWALK JOINTS	
	LOCATION
A	P.C. AND P.T. OF CURVES, JUNCTION OF EXISTING AND NEW SIDEWALK.
B	5'-0" CENTER TO CENTER ON SIDEWALK.
C	WHERE SIDEWALK ABUTS CONCRETE CURBS, DRIVEWAYS, AND SIMILAR STRUCTURES

NOTES:
1. PLACE W1.4XW1.4 6"x6" WIRE MESH IN THE SIDEWALK
2. STANDARD SIDEWALK TO BE 4000 PSI @ 28 DAYS
3. CURE ALL CONCRETE WITH CLEAN SAND, PLASTIC MEMBRANE, OR OTHER APPROVED METHOD.
4. TOOLED JOINTS TO OCCUR AT INTERVALS EQUAL TO WIDTH OF SIDEWALK
5. SIDEWALK TO HAVE BROOM FINISH PERPENDICULAR TO PATH OF TRAVEL.
6. CONTRACTOR SHALL REPAIR AND RESTORE ALL AREAS WITHIN THE ABUTTING R/W DISTURBED DURING CONSTRUCTION TO A CONDITION EQUAL TO OR BETTER THAN EXISTING AT NO ADDITIONAL COST TO THE OWNER.
7. ALL DEMOLISHED SIDEWALKS AND ADDITIONS ARE TO BE REPLACED WITH CONCRETE SIDEWALK TO MATCH EXISTING AT NO ADDITIONAL COST TO THE OWNER.
8. ALL DEMOLISHED SIDEWALK RAMPS ARE TO BE REPLACED AS PER ADA STANDARDS AND INCLUDE ADA COMPLIANT DETECTABLE WARNINGS AT NO ADDITIONAL COST TO THE OWNER.

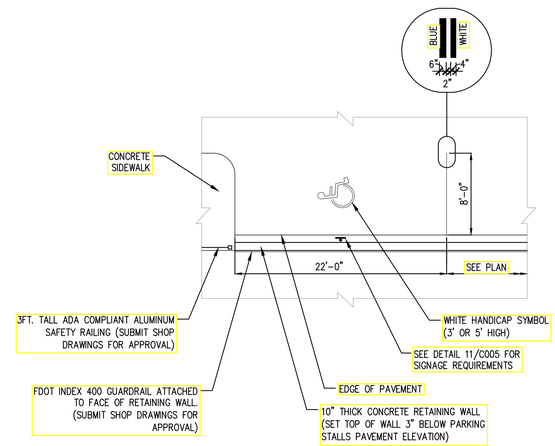
7 SIDEWALK DETAIL
N.T.S.



8 SPECIAL EMPHASIS CROSSWALK MARKING DETAIL
N.T.S.



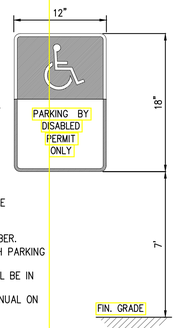
9 RETAINING WALL
N.T.S.



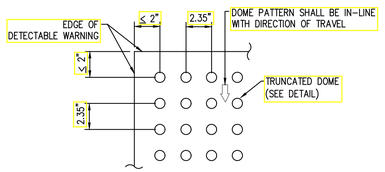
NOTE: FINISHED GRADES OF PAVEMENT IN HC PARKING AND ACCESSIBILITY AISLE SHALL NOT EXCEED 1:50 SLOPE IN ANY DIRECTION. SEE PLAN FOR STYLE OF HANDICAP RAMP.
HANDICAP STRIPING SHALL CONFORM TO FDOT INDEX 17346 (LATEST EDITION)

10 HANDICAP STRIPING
N.T.S.

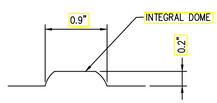
NOTES:
1. ALL LETTERS ARE 1" SERIES "C" PER MUTCD.
2. TOP PORTION OF SIGN SHALL HAVE A REFLECTORIZED (ENGINEERING GRADE) BLUE BACKGROUND WITH WHITE REFLECTORIZED LEGEND AND BORDER.
3. BOTTOM PORTION OF SIGN SHALL HAVE A REFLECTORIZED (ENGINEERING GRADE) WHITE BACKGROUND WITH BLACK OPAQUE LEGEND AND BORDER.
4. FINE NOTIFICATION SIGN SHALL HAVE A REFLECTORIZED (ENGINEERING GRADE) WHITE BACKGROUND WITH BLACK OPAQUE LEGEND & BORDER. CONTRACTOR SHALL VERIFY FINE AMOUNT & ORDINANCE NUMBER.
5. ONE(1) SIGN REQUIRED FOR EACH PARKING SPACE.
6. INSTALLED HEIGHT OF SIGN SHALL BE IN ACCORDANCE WITH SECTION 24-23 OF THE MANUAL ON UNIFORM TRAFFIC CONTROL DEVICES (MUTCD).
7. SIGN MAY BE MOUNTED ON BUILDING/WALL, AT PROPER HEIGHT, IF ALIGNED WITHIN 12" OF CENTER OF PARKING SPACE.



11 HANDICAP SIGN
N.T.S.



PLAN VIEW



TRUNCATED DOME

12 DETECTABLE WARNING
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

BUILDTEC
P.O. BOX 8269
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C008

PROFESSIONAL ENGINEER
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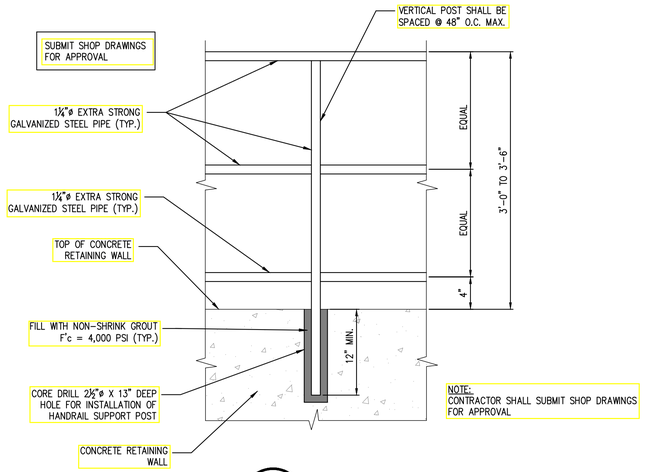
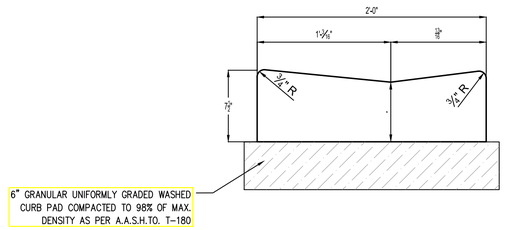
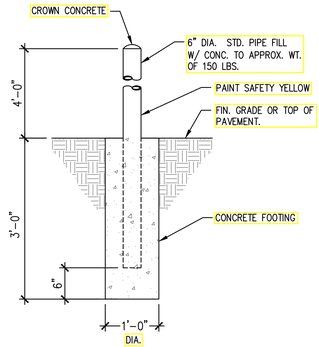


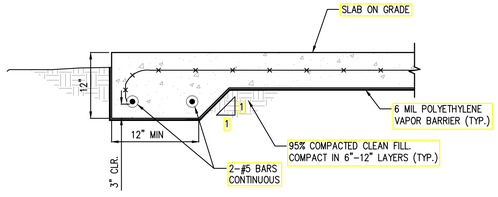
13 RAILING DETAIL
NOT TO SCALE



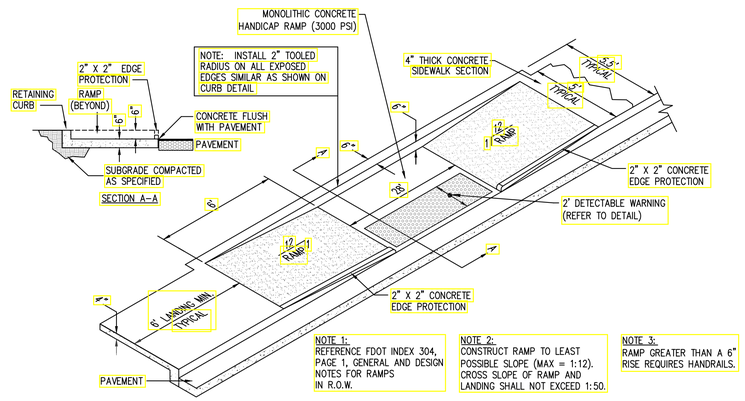
14 VALLEY GUTTER
N.T.S.



15 BOLLARD
N.T.S.



17 THICKENED EDGE DETAIL
N.T.S.



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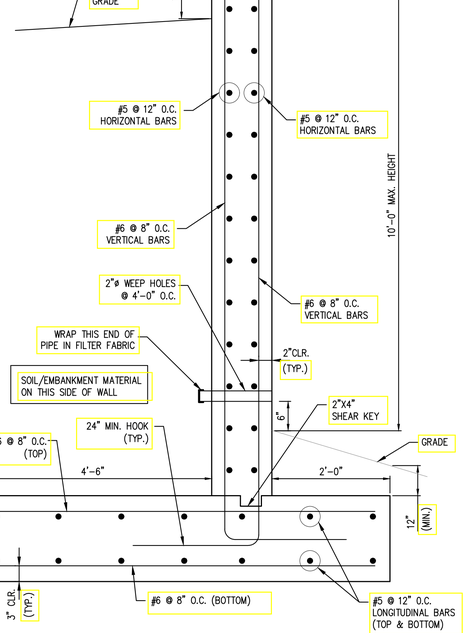
16 HANDICAP RAMP
N.T.S.



18 RETAINING WALL
N.T.S.

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUBBASE, CROWN BAY
ST. THOMAS, USVI

DATE: 10.06.2023
DRAWN BY: DC
CHECKED BY: DC
APPROVED BY: DC

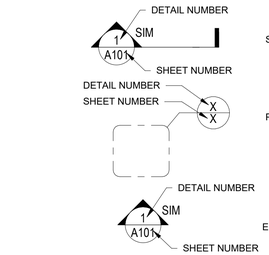
PROJECT NO.:
CONTRACT NO.:

SHEET NO.:
PAVING, GRADING AND DRAINAGE DETAILS

C009

PROFESSIONAL ENGINEER
DAMIAN CARTWRIGHT
28851-IB
U.S. VIRGIN ISLANDS

STRUCTURAL SYMBOLS AND LEGEND



SECTION / DETAIL MARK

PLAN / DETAIL MARK

ELEVATION MARK

RECESS OR STEP IN SLAB

SLOPED SURFACE

PITCHED ROOF

PLAN KEY NOTE

SIDEPLATE CONNECTION

MOMENT CONNECTION

STEPPED FOUNDATION

COLUMN AND FOUNDATION TYPE MARKS

CONCRETE REINF. (REBAR) ORIENTATION

SPOT ELEVATION, TYPICALLY TOP OF ITEM TAGGED (T/WALL, T/FOUNDATION, ETC)

PANEL TYPE, SEE SCHEDULE

WALL TYPE DESIGNATION TAG

INCREASED FLOOR LOAD AREA IN PSF

WALL TYPES

LOAD BEARING MASONRY WALL

NON-LOAD BEARING MASONRY WALL

TILT-UP/PRECAST CONCRETE WALL

CIP CONCRETE WALL

STUD WALL

NOTE: SYMBOLS AND LEGEND SHOWN ARE GENERIC AND DO NOT NECESSARILY INDICATE ACTUAL OCCURRENCES IN THESE DRAWINGS.

SPAN DIRECTION

NORTH ARROW

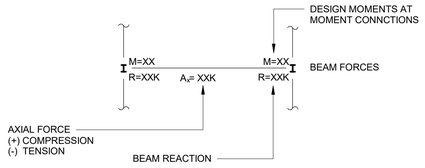
COLUMN GRID LINE

METAL DECK

CONCRETE SLAB ON METAL DECK

CAST-IN-PLACE CONCRETE SLAB

EARTH



STRUCTURAL ABBREVIATIONS

ACI	AMERICAN CONCRETE INSTITUTE
ADD	ADDITIVE
ADDL	ADDITIONAL
AFF	ABOVE FINISHED FLOOR
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION
AISI	AMERICAN IRON AND STEEL INSTITUTE
ALT	ALTERNATE/ALTERNATIVE
ALUM	ALUMINUM
ANCH	ANCHOR
ARCH	ARCHITECTURE/ARCHITECTURAL
ASD	ALLOWABLE STRESS DESIGN
ASTM	AMERICAN SOCIETY OF TESTING MATERIALS
AWS	AMERICAN WELDING SOCIETY
B/	BOTTOM OF
BCX	BOTTOM CHORD EXTENSION
BLDG	BUILDING
BLK	BLOCK
BM	BEAM
BOT	BOTTOM
BP	BASE PLATE/BEARING PLATE
BRG	BEARING
BTWN	BETWEEN
C	CHANNEL
CB	CONCRETE BEAM
CC	CONCRETE COLUMN
CF	CUBIC FEET (FOOT)
CFS	COLD FORMED STEEL
CFMF	COLD FORMED METAL FRAMING
CIP	CAST IN PLACE
CJ	CONTRACTION JOINT OR CONTROL JOINT
CL	CENTERLINE
CLR	CLEAR/CLEARANCE
CMU	CONCRETE MASONRY UNIT
CO	COMPANY
COL	COLUMN
CONC	CONCRETE
CONT	CONTINUOUS
CONN	CONNECTION
CONST	CONSTRUCTION
COORD	COORDINATE
CSJ	CONSTRUCTION JOINT
CTR	CENTER
CTRD	CENTERED
CY	CUBIC YARD
DEPT	DEPARTMENT
DIA	DIAMETER
DIAG	DIAGONAL
DIM	DIMENSION
DIST	DISTANCE
DL	DEAD LOAD
DN	DOWN
DTL	DETAIL
DWG	DRAWING
EA	EACH
EE	EACH END
EF	EACH FACE
EHPA	EMERGENCY HURRICANE PROTECTION AREA
EJ	EXPANSION JOINT
ELEC	ELECTRIC/ELECTRICAL
ELEV	ELEVATION
ENGR	ENGINEER
EOD	EDGE OF DECK
EOR	ENGINEER OF RECORD
EOS	EDGE OF SLAB
EQ SP	EQUALLY SPACED
ES	EACH SIDE
EW	EACH WAY
EX	EXISTING
EXIST	EXISTING
EXP	EXPANSION
EXT	EXTERIOR
F	FOUNDATION
FD	FLOOR DRAIN
FF	FINISHED FLOOR
FFE	FINISHED FLOOR ELEVATION
FIN	FINISH
FIN GR	FINISH GRADE
FL	FLOOR
FND	FOUNDATION
FS	FAR SIDE
FT	FEET/FOOT
FTG	FOOTING
FV	FIELD VERIFY
GA	GAGE/GAUGE
GALV	GALVANIZED
GB	GRADE BEAM
GC	GENERAL CONTRACTOR
GEN	GENERAL
GL	GRID LINE
GS	GALVANIZED STEEL
HSA	HEADED STUD ANCHOR
HT	HEIGHT
H	HIGH
HSS	HOLLOW STRUCTURAL SECTION
HORIZ	HORIZONTAL
HD	HOT DIPPED
HDG	HOT DIPPED GALVANIZED

ID	INSIDE DIAMETER
I.F.	INSIDE FACE
IN	INCH
INT	INTERIOR
JST	JOIST
JT	JOINT
K	KIP (1000 LB)
KLF	KIPS PER LINEAL FOOT
KSI	KIPS PER SQUARE INCH
KWY	KEYWAY
L	LONG
LB	POUND
Ld	DEVELOPMENT LENGTH
LGTH	LENGTH
LL	LIVE LOAD
LLH	LONG LEG HORIZONTAL
LLV	LONG LEG VERTICAL
LONG	LONGITUDINAL
LRFD	LOAD AND RESISTANCE FACTOR DESIGN
LSH	LONG SIDE HORIZONTAL
LSL	LAMINATED STRAND LUMBER
LSV	LONG SIDE VERTICAL
LT WT	LIGHT WEIGHT
LVL	LAMINATED VENEER LUMBER
LW	LONG WAY
LWIC	LIGHT WEIGHT INSULATED CONCRETE
MATL	MATERIAL
MAX	MAXIMUM
MB	MASONRY BEAM
MC	MASONRY COLUMN
MECH	MECHANICAL
MEP	MECHANICAL, ELECTRICAL, PLUMBING
MET	METAL
MFR	MANUFACTURER/MANUFACTURER
MID	MIDDLE
MIN	MINIMUM
MISC	MISCELLANEOUS
MO	MASONRY OPENING
MPH	MILES PER HOUR
NGVD	NATIONAL GEODETIC VERTICAL DATUM
NIC	NOT IN CONTRACT
NO	NUMBER
NS	NEAR SIDE
NTS	NOT TO SCALE
OC	ON CENTER
OD	OUTSIDE DIAMETER
O.F.	OUTSIDE FACE
OPNG	OPENING
OPP	OPPOSITE
OSB	ORIENTED STRAND BOARD
P/C	PRECAST CONCRETE/PILE CAP
P/T	POST TENSIONED
PAR	PARALLEL
PCB	PRECAST CONCRETE BEAM
PCC	PRECAST CONCRETE COLUMN
PCF	POUNDS PER CUBIC FEET
PCI	PRECAST/PRESTRESSED CONCRETE INSTITUTE
PEMB	PRE-ENGINEERED METAL BUILDING
PEN	PENETRATION
PERP	PERPENDICULAR
PJ	PANEL JOINT CENTERLINE
PL	PLATE
PLF	POUNDS PER LINEAR FOOT
PLMG	PLUMBING
PLY	PLYWOOD
PRE-ENG	PRE-ENGINEERED
PREFAB	PREFABRICATED
PSF	POUNDS PER SQUARE FOOT
PSI	POUNDS PER SQUARE INCH
PSL	PARALLEL STRAND LUMBER
PT	PRESSURE TREATED
PTI	POST-TENSIONING INSTITUTE
RD	ROOF DRAIN
REF	REFERENCE
REINF	REINFORCING
REQD	REQUIRED
REV	REVISION
RFI	REQUEST FOR INFORMATION
RND	ROUND
RO	ROUGH OPENING
RTU	ROOF TOP UNIT
SB	SOFFIT BEAM
SCHED	SCHEDULE
SF	SQUARE FEET or STRIP FOUNDATION
SIM	SIMILAR
SPC	SPACE/SPACES
SPECS	SPECIFICATIONS
SQ	SQUARE
SS	STAINLESS STEEL
STD	STANDARD
STIFF	STIFFENER
STL	STEEL
STRUCT	STRUCTURAL
SW	SHORT WAY
SYM	SYMMETRICAL
T/	TOP OF
TB	TIE BEAM
TC	TIE COLUMN
T&B	TOP AND BOTTOM
TCX	TOP CHORD EXTENSION
TDS	TURN DOWN SLAB
TE	THICKENED EDGE
THK	THICKNESS
TEMP	TEMPERATURE
TENS	TENSION
THD	THREAD/THREADED
THK	THICK
TOL	TOLERANCE
TRANS	TRANSVERSE
T.S.	THICKENED SLAB
TWF	THICKENED WALL FOUNDATION
TYP	TYPICAL
UNO	UNLESS NOTED OTHERWISE
VERT	VERTICAL
VIF	VERIFY IN FIELD
VOL	VOLUME
W	WIDTH
W/	WITH
W/O	WITHOUT
WD	WOOD
WF	WALL FOOTING
WP	WATERPROOF or WORKING POINT
WS	WELDED STUD
WT	WEIGHT/STRUCTURAL TEE SECTION
WWF	WELDED WIRE FABRIC
@	AT DESIGNATION
#	POUNDS / REBAR SIZE NUMBER
+/-	PLUS OR MINUS
℄	CENTER LINE
&	AND
Sx	SECTION MODULUS
Ix	MOMENT OF INERTIA

STRUCTURAL SHEET INDEX

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S0.3	STRUCTURAL GENERAL NOTES CONTINUED
S0.4	STRUCTURAL GENERAL NOTES CONTINUED
S0.5	COMPONENT WIND CRITERIA
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S2.1	FOUNDATION - SLAB PLAN
S2.2	LEVEL 2 - FRAMING PLAN
S2.3	ROOF FRAMING PLAN
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S3.1	BUILDING SECTIONS
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S7.6	SIDEPLATE BEAM DETAILS
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S7.8	SIDEPLATE FIELD ERECTION DETAILS (1 OF 2)
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S7.14	SIDEPLATE CONSTRUCTION GUIDELINES (2 OF 3)
S7.15	SIDEPLATE CONSTRUCTION GUIDELINES (3 OF 3)

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY
INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: SH

CHECKED BY: TL

APPROVED BY: JRM

PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: S0.1

ABBREVIATIONS SYMBOLS AND SHEET INDEX

1000 GENERAL NOTES:

1. STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH PROJECT SPECIFICATIONS AND ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, AND SITE DRAWINGS. CONSULT THESE DRAWINGS FOR OPENINGS, DEPRESSIONS, EQUIPMENT WEIGHTS AND LOCATIONS, EMBEDDED ITEMS AND OTHER DETAILS NOT SHOWN ON STRUCTURAL DRAWINGS.
2. DIMENSIONS AND CONDITIONS MUST BE VERIFIED IN THE FIELD. ANY DISCREPANCIES SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER OF RECORD BEFORE PROCEEDING WITH THE AFFECTED PART OF THE WORK.
3. NO STRUCTURAL MEMBER OR COMPONENT SHALL BE CUT, NOTCHED, OR OTHERWISE ALTERED UNLESS APPROVED IN WRITING BY THE ENGINEER OF RECORD. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY AND ALL COSTS INCURRED BY THE ENGINEER OF RECORD FOR REVIEW OF ANY SUCH DEVIATIONS.
4. DO NOT SCALE DRAWINGS.
5. THE STRUCTURE IS DESIGNED TO BE SELF-SUPPORTING AND STABLE AFTER THE BUILDING IS COMPLETE. IT IS THE CONTRACTOR'S RESPONSIBILITY TO DETERMINE ERECTION PROCEDURES AND SEQUENCE TO ENSURE SAFETY OF THE BUILDING AND ITS COMPONENTS DURING ERECTION. THIS INCLUDES THE ADDITION OF NECESSARY SHORING, SHEETING, TEMPORARY BRACING, GUYS OR TIEDOWNS.
6. DETAILS ON THE DRAWINGS SHALL APPLY TO ALL SITUATIONS OCCURRING ON THE PROJECT THAT ARE THE SAME OR SIMILAR TO THOSE SPECIFICALLY DETAILED. THE APPLICABILITY OF THE DETAIL TO ITS LOCATION ON THE DRAWINGS CAN BE DETERMINED BY THE TITLE OF DETAIL. SUCH DETAILS SHALL APPLY WHETHER OR NOT THEY ARE REFERENCED AT EACH LOCATION. QUESTIONS REGARDING APPLICABILITY OF TYPICAL DETAILS SHALL BE DETERMINED BY THE ENGINEER OF RECORD.
7. THE GENERAL CONTRACTOR SHALL COMPARE THE ARCHITECTURAL, MECHANICAL, ELECTRICAL, PLUMBING, CIVIL AND STRUCTURAL DRAWINGS AND REPORT ANY DISCREPANCIES BETWEEN EACH SET OF DRAWINGS AND WITHIN EACH SET OF DRAWINGS TO THE ARCHITECT AND ENGINEER OF RECORD PRIOR TO THE FABRICATION AND INSTALLATION OF ANY STRUCTURAL MEMBERS.
8. THE CONTRACT STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, AND DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, SEQUENCE AND SAFETY. THE ENGINEER DOES NOT HAVE CONTROL OR CHARGE OF, AND SHALL NOT BE RESPONSIBLE FOR, CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES, FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK, FOR THE ACTS OR OMISSION OF THE CONTRACTOR, SUBCONTRACTOR OR ANY OTHER PERSONS PERFORMING ANY OF THE WORK, OR FOR THE FAILURE OF ANY OF THEM TO CARRY OUT THE WORK IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.
9. THE STRUCTURAL ENGINEER'S OBLIGATIONS TO REVIEW SHOP DRAWINGS AND OTHER SUBMITTALS AND TO RETURN THEM IN A TIMELY MANNER ARE CONDITIONED UPON THE PRIOR REVIEW AND APPROVAL OF THE SHOP DRAWINGS OR SUBMITTALS BY THE CONTRACTOR AS REQUIRED IN THE CONSTRUCTION CONTRACT AND THE CONTRACTOR'S SUBMITTAL OF THE SHOP DRAWINGS AND OTHER SUBMITTALS IN ACCORDANCE WITH A WRITTEN SCHEDULE DISTRIBUTED IN ADVANCE TO THE ENGINEER IDENTIFYING THE DATES FOR THE SUBMITTAL OF THE VARIOUS SHOP DRAWINGS AND SUBMITTALS.
10. PERIODIC SITE OBSERVATION BY FIELD REPRESENTATIVES OF TLC ENGINEERING SOLUTIONS IS SOLELY FOR THE PURPOSE OF DETERMINING IF THE WORK OF THE CONTRACTOR IS PROCEEDING IN GENERAL ACCORDANCE WITH THE STRUCTURAL CONTRACT DOCUMENTS. THIS LIMITED SITE OBSERVATION SHALL NOT BE CONSTRUED AS EXHAUSTIVE OR CONTINUOUS TO CHECK THE QUALITY OR QUANTITY OF THE WORK.
11. ALL STRUCTURES REQUIRE PERIODIC MAINTENANCE TO EXCEED LIFE SPAN AND TO ENSURE STRUCTURAL INTEGRITY FROM EXPOSURE TO THE ENVIRONMENT. A PLANNED PROGRAM OF MAINTENANCE SHALL BE ESTABLISHED BY THE OWNER. THIS PROGRAM SHALL INCLUDE ITEMS SUCH AS, BUT NOT LIMITED TO, PAINTING OF STRUCTURAL STEEL, PROTECTIVE COATINGS FOR CONCRETE, SEALANTS, CAULKED JOINTS, EXPANSION JOINTS, CONTROL JOINTS, SPALLS AND CRACKS IN CONCRETE, AND PRESSURE WASHING OF EXPOSED STRUCTURAL ELEMENTS EXPOSED TO SALT ENVIRONMENT OR OTHER HARSH CHEMICALS.
12. STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR THE DESIGN OF STEEL STAIRS, HANDRAILS, CURTAIN WALL/WINDOW WALL SYSTEMS, COLD-FORMED STEEL FRAMING, OR OTHER SYSTEMS NOT SHOWN IN THE STRUCTURAL DOCUMENTS. SUCH SYSTEMS SHALL BE DESIGNED, FURNISHED, AND INSTALLED AS REQUIRED BY OTHER PORTIONS OF THE CONTRACT DOCUMENTS.
13. IN THE PROFESSIONAL OPINION OF TLC ENGINEERING SOLUTIONS, INC. THE STRUCTURAL CONTRACT DOCUMENTS FOR THIS PROJECT HAVE BEEN PREPARED IN ACCORDANCE WITH THE DESIGN CRITERIA AS SET FORTH IN THE 2021 INTERNATIONAL BUILDING CODE (IBC).
14. NO PROVISIONS HAVE BEEN MADE FOR VERTICAL OR HORIZONTAL EXPANSION EXCEPT AS SHOWN ON CONTRACT DOCUMENTS.
15. FINISH FLOOR ELEVATION (FIRST FLOOR) OF 0'-0" (100'-0") IS USED AS A REFERENCE ELEVATION. ACTUAL FLOOR ELEVATION IS + 220.30'. SEE CIVIL DRAWINGS FOR ACTUAL ELEVATION.
16. THE USE OF REPRODUCTIONS OF THESE CONTRACT DOCUMENTS AND USE OF CAD/REVIT FILES BY ANY CONTRACTOR, SUBCONTRACTOR, ERECTOR, FABRICATOR OR MATERIAL SUPPLIER IN LIEU OF PREPARATION OF SHOP DRAWINGS IS PROHIBITED UNLESS PRIOR WRITTEN APPROVAL IS OBTAINED FROM ENGINEER OF RECORD.
17. IN THE EVENT THAT THE STRUCTURAL CONTRACTS DRAWINGS AND SPECIFICATIONS CONFLICT ON INFORMATION, THE STRUCTURAL CONTRACT DRAWINGS SHALL SUPERSEDE THE SPECIFICATIONS.

1060 DESIGN LOADS:

1. THE STRUCTURAL SYSTEM FOR THIS BUILDING HAS BEEN DESIGNED IN ACCORDANCE WITH THE 2021 INTERNATIONAL BUILDING CODE.
2. THE FOLLOWING SUPERIMPOSED LOADINGS HAVE BEEN UTILIZED:

A. DEAD LOADS:

ROOF STRUCTURE	10 PSF
M/E/P LOADS	5 PSF
CEILINGS	5 PSF
COLLATERAL LOADS	10 PSF
STEEL STUDS W/ GYP. BOARD PARTITIONS	10 PSF

B. LIVE LOADS:

ROOF	20 PSF
FLOOR (OFFICE)	50 PSF
FLOOR (MECH ROOM)	150 PSF
BALCONIES (EXTERIOR)	(1.5*OCCUPANCY SERVED*)
STAIRS AND EXITS	100 PSF
GUARDRAILS/HANDRAILS	50 PLF (UNIFORM) 200 LBS (CONCENTRATED)

C. WIND LOADS: PER ASCE 7-16.

SEE SHEET S0.5 FOR COMPONENTS AND CLADDING PRESSURES

ULTIMATE DESIGN WIND SPEED, Vult	217 MPH (3 SEC. GUST)
NOMINAL DESIGN WIND SPEED, Vasd	168 MPH (3 SEC. GUST)
RISK CATEGORY	II
EXPOSURE	D

D. SEISMIC LOADS: PER ASCE 7-16.

RISK CATEGORY	II
SEISMIC IMPORTANCE FACTOR, Ie	1.0
SPECTRAL RESPONSE ACCELERATION, SHORT DURATION, Ss	1.222
SPECTRAL RESPONSE ACCELERATION, 1.0 SECOND DURATION, S1	0.428
DESIGN SPECTRAL RESPONSE ACCELERATION, SHORT DURATION, Sds	0.978
DESIGN SPECTRAL RESPONSE ACCELERATION, 1.0 SECOND DURATION, Sd1	0.428
SITE CLASS	C
SEISMIC DESIGN CATEGORY, SDC	D
BASIC SEISMIC FORCE-RESISTING SYSTEM(S)	SPECIAL STRUCTURAL STEEL MOMENT FRAMES (SIDEPLATE)
DESIGN BASE SHEAR(S)	0.144W KIPS
SEISMIC RESPONSE COEFFICIENT(S), SC	0.114
RESPONSE MODIFICATION COEFFICIENT(S), R	8
ANALYSIS PROCEDURE USED	ELF

1330 SHOP DRAWING REVIEW:

1. SHOP DRAWINGS SHALL ADEQUATELY DEPICT THE STRUCTURAL ELEMENTS AND CONNECTIONS SHOWN ON THE CONTRACT DOCUMENTS. SHOP DRAWINGS WILL BE REVIEWED FOR GENERAL COMPLIANCE WITH THE DESIGN INTENT OF THE CONTRACT DOCUMENTS ONLY. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY COMPLIANCE WITH THE CONTRACT DOCUMENTS AS TO QUANTITY, LENGTH, ELEVATIONS, DIMENSIONS, ETC. REVIEW OF SUBMITTALS AND SHOP DRAWINGS DOES NOT RELIEVE THE CONTRACTOR OF FULL RESPONSIBILITY FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF THE SHOP DRAWINGS.
2. SHOP DRAWINGS SHALL BE REVIEWED BY THE CONTRACTOR AND MARKED "APPROVED" PRIOR TO SUBMITTAL TO THE ARCHITECT/ENGINEER. NON-CONFORMING DRAWING SUBMITTALS WILL BE RETURNED WITHOUT REVIEW.
3. SHOP DRAWING SUBMITTALS SHALL BE SUBMITTED ELECTRONICALLY.
4. THE CONTRACT DOCUMENTS WILL GOVERN OVER THE SHOP DRAWINGS UNLESS OTHERWISE SPECIFIED IN WRITING BY THE ENGINEER OF RECORD.
5. CHANGES AND ADDITIONS MADE ON RE-SUBMITTALS SHALL BE CLEARLY FLAGGED AND NOTED. THE PURPOSE OF THE RE-SUBMITTALS SHALL BE CLEARLY NOTED ON THE LETTER OF TRANSMITTAL. ARCHITECT/ENGINEER OF RECORD REVIEW WILL BE LIMITED TO THOSE ITEMS CAUSING THE RE-SUBMITTAL. CONTRACTOR IS RESPONSIBLE FOR COSTS CAUSED BY MULTIPLE RE-SUBMITTALS (MORE THAN ONE) AT ARCHITECT/ENGINEERS' CURRENT HOURLY RATES.

1331 SHOP DRAWINGS FOR SPECIALTY ENGINEERED PRODUCTS:

1. THE FOLLOWING SYSTEMS AND COMPONENTS AS A MINIMUM REQUIRE FABRICATION AND ERECTION DRAWINGS PREPARED BY A DELEGATED ENGINEER:
   A. COLD FORMED STEEL EXTERIOR OR LOAD BEARING WALL SYSTEMS
   B. CURTAIN WALL SYSTEMS
   C. PREFABRICATED STEEL STAIRS & RAILINGS
   D. METAL ROOF DECK ASSEMBLIES
   E. TEMPORARY WALL BRACING
   F. FORMWORK AND TEMPORARY SHORING
2. SUBMITTALS SHALL CLEARLY IDENTIFY THE SPECIFIC PROJECT AND APPLICABLE CODES, LIST THE DESIGN CRITERIA, AND SHOW ALL DETAILS AND DRAWINGS NECESSARY FOR PROPER FABRICATION AND INSTALLATION. SHOP DRAWINGS AND CALCULATIONS SHALL IDENTIFY SPECIFIC PRODUCT UTILIZED. GENERIC PRODUCTS WILL NOT BE ACCEPTED.
3. SHOP DRAWINGS AND CALCULATIONS SHALL BE PREPARED UNDER THE DIRECT SUPERVISION AND CONTROL OF THE DELEGATED ENGINEER.
4. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE TERRITORY OF THE U.S. VIRGIN ISLANDS (IN WHICH THE PROJECT RESIDES). COMPUTER PRINTOUTS ARE AN ACCEPTABLE SUBSTITUTE FOR MANUAL COMPUTATIONS PROVIDED THEY ARE ACCOMPANIED BY SUFFICIENT DESCRIPTIVE INFORMATION TO PERMIT THEIR PROPER EVALUATION. SUCH DESCRIPTIVE INFORMATION SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE TERRITORY OF THE U.S. VIRGIN ISLANDS (IN WHICH THE PROJECT RESIDES) AS AN INDICATION THAT HE/SHE HAS ACCEPTED RESPONSIBILITY FOR THE RESULTS. THE STRUCTURAL ENGINEER WILL RETAIN ONE SIGNED AND SEALED SET FOR THEIR RECORDS.
5. DRAWINGS PREPARED SOLELY TO SERVE AS A GUIDE FOR FABRICATION AND INSTALLATION (SUCH AS REINFORCING STEEL SHOP DRAWINGS OR STRUCTURAL STEEL ERECTION DRAWINGS) AND REQUIRING NO ENGINEERING, DO NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
6. CATALOG INFORMATION ON STANDARD PRODUCTS DOES NOT REQUIRE THE SEAL OF A DELEGATED ENGINEER.
7. REVIEW BY THE STRUCTURAL ENGINEER OF RECORD OF SUBMITTALS IS LIMITED TO VERIFYING THE FOLLOWING:
   A. THAT THE SPECIFIED STRUCTURAL SUBMITTALS HAVE BEEN FURNISHED.
   B. THAT THE STRUCTURAL SUBMITTALS HAVE BEEN SIGNED AND SEALED BY THE DELEGATED ENGINEER.
   C. THAT THE DELEGATED ENGINEER HAS UNDERSTOOD THE DESIGN INTENT AND HAS USED THE SPECIFIED STRUCTURAL CRITERIA. NO DETAILED CHECK OF CALCULATIONS WILL BE MADE.
   D. THAT THE CONFIGURATION SET FORTH IN THE STRUCTURAL SUBMITTALS IS CONSISTENT WITH THE CONTRACT DOCUMENTS. NO DETAILED CHECK OF DIMENSIONS OR QUANTITIES WILL BE MADE.
8. SUBMITTALS NOT MEETING THE ABOVE CRITERIA WILL NOT BE REVIEWED AND WILL BE RETURNED.

1333 SUBMITTALS:

1. ALL SHOP DRAWINGS MUST BE REVIEWED AND STAMPED APPROVED BY THE GENERAL CONTRACTOR PRIOR TO SUBMITTAL.
2. THE GENERAL CONTRACTOR SHALL SUBMIT FOR ENGINEER REVIEW SHOP DRAWINGS FOR THE FOLLOWING ITEMS:
   - ITEMS MARKED (D) SHALL HAVE SHOP DRAWINGS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE TERRITORY OF THE US VIRGIN ISLANDS.
   - ITEMS MARKED (#) SHALL BE SUBMITTED FOR ENGINEER'S RECORD ONLY.

   A. STRUCTURAL STEEL (D)
   B. SIDEPLATE
   C. REINFORCING STEEL
   D. STEEL STAIRS (D)
   E. METAL ROOF OR FORM DECK
   F. COMPOSITE METAL DECK
   G. COLD-FORMED STEEL FRAMING (D)
   H. FORMWORK, SHORING, RESHORING (D,#)
   I. CONCRETE MIX DESIGNS
   J. CONSTRUCTION JOINT LOCATIONS IN STRUCTURAL FLOORS
   K. ELEVATORS (#)
   L. CURTAIN WALL SYSTEMS (D)
   M. TEMPORARY WALL BRACING (D)
   N. MECHANICAL ANCHORS (#)
   O. CHEMICAL (ADHESIVE) ANCHORS (#)
   P. FORMWORK AND TEMPORARY SHORING (D)
   Q. METAL ROOF DECK ASSEMBLIES (D)
   R. POST-INSTALLED ADHESIVE ANCHORS (#)
   S. POST-INSTALLED MECHANICAL ANCHORS (#)
3. MANUFACTURER'S LITERATURE. SUBMIT TWO COPIES OF MANUFACTURER'S LITERATURE FOR ALL MATERIALS AND PRODUCTS USED IN CONSTRUCTION ON THE PROJECT.

1334 REQUEST FOR INTERPRETATION:

1. RFI SHALL ORIGINATE WITH CONTRACTOR AND SHALL BE SUBMITTED IN THE FORM SPECIFIED WITHIN CONTRACT DOCUMENTS. RFI SHALL BE SUBMITTED IN A PROMPT MANNER AS TO AVOID DELAYS IN CONTRACTORS WORK.
2. RFI SHALL BE SUBMITTED AS SPECIFIED WITHIN THE CONTRACT DOCUMENTS AND SHALL BE FORWARDED TO THE ENGINEER VIA THE ARCHITECT OR DIRECTLY TO THE ENGINEER BY THE CONTRACTOR WHEN APPROVED BY THE ARCHITECT.
3. ENGINEER SHALL TAKE UP TO 5 BUSINESS DAYS TO REVIEW AND RETURN RFI'S. HOWEVER, THE ENGINEER WILL ATTEMPT TO EXPEDITE THE REVIEW OF ALL RFI'S WITHIN A REASONABLE TIME FRAME.
4. RFI RESPONSES ARE NOT INTENDED TO AUTHORIZE ANY INCREASE IN CONSTRUCTION COST, SCHEDULE OR TIME EXTENSIONS, OR CONSTRUCTION IN CONFLICT WITH ANY APPLICABLE CODES OR SPECIFIED DESIGN STANDARDS. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO NOTIFY THE DESIGN TEAM IMMEDIATELY OF ANY PERCEIVED SCOPE, SCHEDULE, OR COST IMPACTS OR ADJUSTMENTS. IF CONTRACTOR REQUESTS ANY ADDITIONAL COST, INCREASE IN SCHEDULE OR ADJUSTMENT IN SCOPE, THE CONTRACTOR SHALL NOT PROCEED WITH ADDITIONAL WORK UNTIL APPROVED IN WRITING BY THE CONSTRUCTION ADMINISTRATOR.

2210 DEMOLITION NOTES:

1. THE CONTRACTOR IS REQUIRED TO PROVIDE ALL TEMPORARY SCAFFOLDING, PLATFORMS, BARRICADES, RAILINGS, SCREENING, ETC. NECESSARY TO PROTECT EXISTING FACILITIES, STRUCTURES AND THE PUBLIC DURING DEMOLITION AND ERECTION OF THE NEW CONSTRUCTION, AS WELL AS FOR JOB SAFETY. JOB SAFETY, CONSTRUCTION AND DEMOLITION PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. THE CONTRACTOR IS REQUIRED TO TAKE ALL PRECAUTIONS TO MINIMIZE VIBRATION, NOISE, DUST AND DEBRIS IN ALL AREAS ADJACENT TO AREAS OF DEMOLITION.
2. THE CONTRACTOR IS REQUIRED TO COORDINATE WITH THE OWNER FOR THE TEMPORARY SUSPENSION OF USE OF ANY FACILITY OR PORTION THEREOF, AND THE ASSOCIATED BARRICADING REQUIREMENTS WITHIN A MINIMUM OF 7 DAYS PRIOR TO COMMENCING WORK.
3. THE CONTRACTOR IS REQUIRED TO PERFORM HIS WORK IN A MANNER, WHICH WILL NOT CONFLICT WITH ANY OPERATION, WHICH IS TO REMAIN FUNCTIONAL DURING THE COURSE OF THE PROJECT, UNTIL SUCH OPERATION IS SCHEDULED TO BE SHUT DOWN.
4. THE CONTRACTOR IS REQUIRED TO COORDINATE WITH OWNER FOR THE TEMPORARY SUSPENSION OF USE OF ANY UTILITY SYSTEM, A MINIMUM OF 3 DAYS PRIOR TO COMMENCING WORK.
5. AT ALL LOCATIONS WHERE NEW CONSTRUCTION WILL INTERFACE WITH EXISTING ELEMENTS, CUT THROUGH EXISTING STRUCTURE IN STRAIGHT AND TRUE LINES TO ENSURE A NEAT INTERFACE.
6. AT ALL LOCATIONS WHERE THE DEMOLITION OF A CONCRETE MEMBER LEAVES THE ENDS OF REINFORCING STEEL EXPOSED, PROVIDE THE FOLLOWING:
   A. A. CHIP CONCRETE FROM AROUND THE STEEL TO A DEPTH OF 1".
   B. B. CUT OFF REINFORCING STEEL NOT LESS THAN 3/4" BELOW THE CONCRETE SURFACE.
   C. FILL THE CAVITY FLUSH WITH A HIGH MODULUS GEL EPOXY. SEE SPECIFICATION FOR ACCEPTED MANUFACTURERS.
7. BEFORE DEMOLISHING ANY STRUCTURAL ELEMENT, INSTALL ALL REQUIRED TEMPORARY AND/OR PERMANENT BRACING AND SUPPORTS.
8. PROVIDE TEMPORARY CLOSURE OF ALL ROOF FASCIA, WALL AND OTHER OPENINGS TO PROTECT BUILDING FROM EXPOSURE TO UNDESIRABLE ELEMENTS UNTIL NEW CONSTRUCTION IS WEATHERPROOFED, AT WHICH TIME SUCH TEMPORARY CONSTRUCTION SHALL BE REMOVED. ALL TEMPORARY EXTERIOR WALLS THAT ARE SUBJECT TO WIND LOADS ARE TO BE DESIGNED BY A DELEGATED ENGINEER.
9. UPON COMPLETION OF NEW CONSTRUCTION UNDER EACH PHASE, ALL DEMOLISHED AREAS SHALL BE RESTORED TO ACCEPTABLE USAGE ACCORDING TO THE CONTRACT DOCUMENTS AS DETERMINED BY THE ARCHITECT/ENGINEER.
10. REMOVE COMPLETELY FROM THE SITE AND LEGALLY DISPOSE ALL DEBRIS GENERATED BY THE DEMOLITION WORK AS THE WORK PROGRESSES. STOCKPILING OF DEBRIS AND BURNING OF DEBRIS ON THE PREMISES IS STRICTLY PROHIBITED.

2220 EXISTING STRUCTURE:

1. INFORMATION SHOWN FOR THE EXISTING STRUCTURE ON THESE DRAWINGS WAS TAKEN FROM THE DRAWINGS THAT WERE PREPARED FOR:

PREPARED BY:	PAUL FERRERAS, PE, STRUCTURAL ENGINEER
ENTITLED:	VISUAL STRUCTURAL CONDITION SURVEY AND RECOMMENDATIONS, I.D.C. LIGHT MANUFACTURING COMPLEX, ST. THOMAS, USVI, ECONOMIC DEVELOPMENT AUTHORITY
DATED:	JUNE 10, 2021

2. WORK SHOWN ON THESE DRAWINGS ASSUMES THAT THE ORIGINAL CONSTRUCTION WAS PERFORMED IN ACCORDANCE WITH THE ABOVE INDICATED ORIGINAL DRAWINGS INCLUDING (BUT NOT LIMITED TO) DIMENSIONS, ELEVATIONS, MEMBER SIZES, MATERIALS, DETAILS, ETC. IT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR TO VERIFY THE CONDITIONS RELATING TO THE EXISTING STRUCTURE AND TO NOTIFY THE ENGINEER IMMEDIATELY OF ANY DISCREPANCIES OR CONFLICTS.
3. WORK SHOWN ON THESE DRAWINGS IS INCIDENTAL TO THE STRUCTURAL SYSTEM OF THE BUILDING AND IS THEREFORE LIMITED TO SPECIFIC ITEMS SHOW HEREIN. THE RESPONSIBILITY FOR THE OVERALL STRUCTURAL SYSTEM REMAINS WITH THE ORIGINAL ENGINEER AND DESIGNERS OF RECORD.

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	SH
CHECKED BY:	TL
APPROVED BY:	JRM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S0.2

STRUCTURAL GENERAL NOTES

2310 FOUNDATIONS:

1. SEE THE FOLLOWING GEOTECHNICAL REPORT FOR COMPLETE GEOTECHNICAL RECOMMENDATIONS AND INSTALLATION PROCEDURES. SITE PREPARATION AND FOUNDATION INSTALLATION SHALL COMPLY WITH:

   REPORT No. VTE24-01.1148
   PREPARED BY: VITEST ENGINEERS, LLC
   TITLED: GEOTECHNICAL INVESTIGATION REPORT - ECONOMIC DEVELOPMENT AUTHORITY, PHASE II - ESTATE NISKY, ST THOMAS, USVI
   DATED: NOVEMBER 12, 2024

2. FOLLOW THE RECOMMENDATIONS LISTED IN THE GEOTECHNICAL REPORT FOR SITE PREPARATION WORK. AT A MINIMUM, SITE PREPARATION WORK SHALL INCLUDE:

   A. STRIPPING AND GRUBBING OF THE BUILDING FOOTPRINT PLUS A MARGIN OF 5 FEET AROUND THE BUILDING, REMOVING ALL ORGANIC MATERIALS THE DEPTH OF GRUBBING AND STRIPPING SHOULD BE DETERMINED IN THE FIELD BASED ON VISUAL OBSERVATIONS AND PROPER JUDGEMENT.

   B. PROOF ROLLING THE BUILDING SITE TO LOCATE ANY UNFORESEEN SOFT AREAS. ANY SOFT AREAS SHALL BE EXCAVATED AND REPLACED WITH CLEAN FILL. A DENSITY OF AT LEAST 95% FOR A DEPTH OF 2 FEET IS REQUIRED UNDER THE BUILDING FOOTPRINT.

   C. ALL FILL SHALL BE CLEAN SAND AND FREE OF ORGANIC MATERIALS. COMPACT FILL IN 12 INCH (UNCOMPACTED THICKNESS) LIFTS TO A MINIMUM OF 95% OF THE MODIFIED PROCTOR MAXIMUM DRY DENSITY VALUE.

   D. EXCAVATIONS FOR FOUNDATIONS SHALL BE COMPACTED TO 95% FOR A DEPTH OF AT LEAST 2 FEET BELOW THE BOTTOM OF THE FOUNDATION AND SLABS.

   E. DEWATERING MAY BE REQUIRED TO ACHIEVE THE REQUIRED COMPACTION VALUES, AND IF USED, SHOULD DRAW DOWN THE WATER LEVEL TO AT LEAST 2 FEET BELOW THE BOTTOM OF THE EXCAVATION.

3. SLABS ON GRADE SHALL BE PLACED OVER A 15 MIL, CLASS "A" VAPOR RETARDER. VAPOR RETARDER SHALL BE LAPPED A MINIMUM OF 6", OR AS RECOMMENDED BY THE MANUFACTURER (WHICHEVER IS GREATER) AND TAPED AT ALL JOINTS. ALL PUNCTURES IN THE VAPOR RETARDER SHALL BE REPAIRED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. ALL PENETRATIONS THROUGH THE VAPOR RETARDER (COLUMNS, PLUMBING, CONDUITS, ETC) SHALL BE SEALED PER MANUFACTURER'S WRITTEN INSTRUCTIONS. VAPOR RETARDER SHALL BE CONTINUOUS UNDER WALL FOUNDATIONS OR SEALED TO EXTERIOR WALLS PER MANUFACTURER'S WRITTEN INSTRUCTIONS.

4. FOUNDATION DESIGN IS BASED ON AN ALLOWABLE BEARING PRESSURE OF 3,500 PSF.

5. CANTILEVERED RETAINING WALL AND RESTRAINED FOUNDATION WALL DESIGN IS BASED ON THE FOLLOWING SOIL PROPERTIES:

ACTIVE SOIL PRESSURE COEFFICIENT (Ka)	.038
AT-REST SOIL PRESSURE COEFFICIENT (Ko)	.058
PASSIVE SOIL PRESSURE COEFFICIENT (Kp)	3.25
SATURATED SOIL UNIT WEIGHT	125 PCF
COEFFICIENT OF FRICTION	0.40

3302 CONCRETE:

1. SHALL BE PER AN APPROVED MIX DESIGN PROPORTIONED TO ACHIEVE A STRENGTH AT 28 DAYS AS LISTED BELOW WITH A PLASTIC AND WORKABLE MIX:

CONCRETE STRUCTURE TYPE	COMPRESSIVE STRENGTH	SLUMP	COARSE AGGREGATE	MAXIMUM W/C RATIO
FOUNDATIONS	3000 PSI	4-6"	1"	0.50
SLABS-ON-GRADE	3000 PSI	4-6"	3/4"	0.50
CONCRETE ON METAL DECK	3000 PSI	4-6"	3/4"	0.50
BEAMS AND COLUMNS	4000 PSI	4-6"	3/8"	0.48
BALCONIES	5000 PSI	4-6"	3/4"	0.40

2. CONCRETE SHALL BE PLACED AND CURED ACCORDING TO ACI STANDARDS AND SPECIFICATIONS.

3. SUBMIT PROPOSED MIX DESIGN WITH RECENT FIELD CYLINDER OR LAB TESTS FOR REVIEW PRIOR TO USE. MIX SHALL BE UNIQUELY IDENTIFIED BY MIX NUMBER OR OTHER POSITIVE IDENTIFICATION. MIX SHALL MEET THE REQUIREMENTS OF ASTM C33 FOR COARSE AGGREGATE.

4. CONCRETE SHALL COMPLY WITH THE REQUIREMENTS OF ASTM STANDARD C94 FOR MEASURING, MIXING, TRANSPORTING, ETC. CONCRETE TICKETS SHALL BE TIME STAMPED WHEN CONCRETE IS BATCHED.

5. THE MAXIMUM TIME ALLOWED FROM THE TIME THE MIXING WATER IS ADDED UNTIL IT IS DEPOSITED IN ITS FINAL POSITION SHALL NOT EXCEED ONE AND ONE HALF (1-1/2) HOURS. IF FOR ANY REASON THERE IS A LONGER DELAY THAN THAT STATED ABOVE, THE CONCRETE SHALL BE DISCARDED. IT SHALL BE THE RESPONSIBILITY OF THE TESTING LAB TO NOTIFY THE OWNER'S REPRESENTATIVE AND THE CONTRACTOR OF ANY NONCOMPLIANCE WITH THE ABOVE.

6. SLABS SHALL BE CURED USING A DISSIPATING CURING COMPOUND MEETING ASTM STANDARD C309 TYPE 1-CLASS D AND SHALL HAVE A FUGITIVE DYE. THE COMPOUND SHALL BE PLACED AS SOON AS THE FINISHING IS COMPLETED OR AS SOON AS THE WATER HAS LEFT THE UNFINISHED CONCRETE. SCUFFED OR BROKEN AREAS IN THE CURING MEMBRANE SHALL BE RECOATED DAILY.

7. CALCIUM CHLORIDES SHALL NOT BE UTILIZED; OTHER ADMIXTURES MAY BE USED ONLY WITH THE APPROVAL OF THE ENGINEER.

8. CONCRETE MIX DESIGNS SHALL INCLUDE A WRITTEN DESCRIPTION INDICATING WHERE EACH PARTICULAR MIX IS TO BE PLACED WITHIN THE STRUCTURE.

9. CONDUITS, PIPES AND SLEEVES SHALL BE PLACED AND SPACED IN ACCORDANCE WITH ACI 318, 6.3.

10. CONCRETE DESIGN MIX SUBMITTALS SHALL INCLUDE TESTED, STATISTICAL BACK-UP DATA AS PER CHAPTER 5 OF ACI 318.

11. ALL COLUMNS AND BEAMS INTEGRATED IN CMU WALLS ARE 8" AND 12" NOMINAL AND 7-5/8" AND 11-5/8" ACTUAL DIMENSIONS, RESPECTIVELY.

12. WATER/CEMENT RATIO FOR CONCRETE AT EXTERIOR BALCONIES SHALL NOT EXCEED 0.40 BY WEIGHT.

13. CONCRETE SLABS ON GRADE SHALL BE REINFORCED WITH SYNTHETIC FIBERS AT A MINIMUM RATE OF 1.5 LBS/CY, OR AS RECOMMENDED BY THE FIBER MANUFACTURER FOR CONTROL OF TEMPERATURE AND SHINKAGE/CRACKING, WHICHEVER IS GREATER.

14. WHEN TOTAL WIDTH OF PIPES OR DUCTS CAST INTO A SLAB EXCEED 12" IN A 24" WIDTH THEN THE CONTRACTOR SHALL ADD A LAYER OF #4 @ 12" ABOVE AND PERPENDICULAR TO THE DUCT/PIPE RUNS EXTENDING 12" BEYOND THE LAST DUCT/PIPE ON EACH SIDE.

15. WHEN ADHESIVES OR WATER VAPOR IMPERMEABLE FLOOR COVERINGS ARE BEING USED ON CONCRETE SURFACES, THE CONTRACTOR SHALL VERIFY THROUGH APPROPRIATE TESTING THAT THE WATER CONTENT OF THE CONCRETE, AND VAPOR TRANSMISSION RATE THROUGH THE CONCRETE, IS WITHIN THE ALLOWABLE RANGE BEFORE INSTALLATION. THE MANUFACTURER OF THE ADHESIVE OR FLOOR COVERING SHALL REVIEW AND APPROVE THE RESULTS OF THE TESTING PRIOR TO INSTALLATION OF PRODUCTS.

16. WHERE SPECIFIED CONCRETE STRENGTH OF COLUMN IS GREATER THAN THE SPECIFIED SLAB CONCRETE STRENGTH, HIGHER STRENGTH CONCRETE SHALL BE PUDDLED AT THE COLUMN. THE STRENGTH OF PUDDLED CONCRETE SHALL BE AT LEAST 0.72 TIMES THE STRENGTH OF COLUMN CONCRETE PER ACI 318, 10.15.

3310 REINFORCING STEEL:

1. SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS.

2. PROVIDE CONCRETE COVER OVER PRIMARY REINFORCEMENT, TIES, AND STIRRUPS, AS FOLLOWS, UNLESS OTHERWISE NOTED:

LOCATION AND CONDITION	MINIMUM COVER
A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH.	ALL BARS 3"
B. CONCRETE EXPOSED TO EARTH OR WEATHER	#6 OR GREATER 2" #5 OR SMALLER 1.5"
C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND	
SLABS, WALLS, AND JOISTS	#11 OR SMALLER 3/4"
BEAMS AND COLUMNS	ALL BARS 1.5"

3. SECURE APPROVAL OF SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.

4. PROVIDE STANDARD HOOKS AT DISCONTINUOUS ENDS OF ALL TOP BARS.

5. WHERE REINFORCING IS SHOWN CONTINUOUS, SPLICE BOTTOM BARS OVER SUPPORTS AND TOP BARS AT CENTER OF SPAN. ALL OTHER LAP SPLICES SHALL BE IN ACCORDANCE WITH SPLICE TABLES AND DETAILS SHOWN ON DRAWINGS.

6. PROVIDE DOWELS INTO FOOTINGS, PILE CAPS, SUPPORT BEAMS, ETC. TO MATCH VERTICAL BARS WITH CLASS B TENSION LAP SPLICES, U.N.O.

7. AT CHANGES IN DIRECTION OF CONCRETE WALLS AND TIE BEAMS, PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL STEEL.

8. WHERE HOOKS ARE SHOWN ON THE PLANS OR DETAILS, HOOKS SHALL BE DETAILED TO EXTEND DEEP ENOUGH INTO SUPPORTING STRUCTURE TO DEVELOP THE FULL STRENGTH OF THE HOOKED BAR. PROVIDE ADDITIONAL TIES OR STIRRUPS IN SUPPORTING STRUCTURE AS REQUIRED TO SATISFY ACI 318 HOOK DEVELOPMENT, CONFINEMENT, AND ANCHORAGE CRITERIA.

9. AT CANTILEVER SLABS AND BEAMS, REINFORCING BARS IN DIRECTION OF CANTILEVER SHALL BE DETAILED TO FULLY DEVELOP THE BAR STRENGTH INTO THE SUPPORTING STRUCTURE, EITHER BY PROVIDING FULL CLASS B LAP SPLICE OR STANDARD ACI HOOKS EMBEDDED DEEP ENOUGH BEYOND SUPPORT TO DEVELOP STRENGTH OF BAR.

3400 CONCRETE TESTING:

1. AN INDEPENDENT TESTING LABORATORY SHALL PERFORM THE FOLLOWING TESTS ON CAST IN PLACE CONCRETE:

   A. A. ASTM C143 - "STANDARD TEST METHOD FOR SLUMP OF PORTLAND CEMENT CONCRETE."

   B. B. ASTM C39 - "STANDARD TEST METHOD FOR COMPRESSIVE STRENGTH OF CYLINDRICAL CONCRETE SPECIMENS." A SEPARATE TEST SHALL BE CONDUCTED FOR EACH CLASS, FOR EVERY 50 CUBIC YARDS (OR FRACTION THEREOF), PLACED PER DAY. REQUIRED LAB CURED CYLINDER QUANTITIES AND TEST AGE AS FOLLOWS:

      (2) AT 7 DAYS
      (3) AT 28 DAYS

   ONE ADDITIONAL RESERVE CYLINDER TO BE TESTED UNDER THE DIRECTION OF THE ENGINEER, IF REQUIRED. IF 28-DAY STRENGTH IS ACHIEVED, THE ADDITIONAL CYLINDER(S) MAY BE DISCARDED.

3601 CHEMICAL (ADHESIVE) ANCHORS:

1. SHALL BE A TWO PART EPOXY POLYMER INJECTION SYSTEM, SUCH AS HILTI HIT HY200, HILTI RE500 SD, DEWALT PURE 100+, DEWALT AC200+, OR SIMPSON SET ADHESIVE SYSTEM, OR ENGINEER APPROVED SUBSTITUTION.

2. EPOXY TYPES AND BRANDS VARY IN THEIR BOND STRENGTH AND SUITABILITY OF USE, DEPENDING ON TYPE OF LOADING, ANCHOR SPACING, ETC. WHEN A PARTICULAR TYPE OF EPOXY IS SPECIFIED IN THESE DRAWINGS, A UNIQUE CALCULATION HAS BEEN MADE BASED ON THE PROPERTIES OF THAT SPECIFIC TYPE OF EPOXY FOR THE SPECIFIC CONDITION SHOWN IN THE DETAIL. SUBSTITUTION OF EPOXY TYPE IS NOT ALLOWED WHERE DETAIL SPECIFIES ONLY ONE TYPE OF EPOXY, WITHOUT PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD. NOT ALL EPOXY BRANDS OR TYPES WILL BE ALLOWED AS SUBSTITUTES. ICC-ES REPORTS FOR PROPOSED ANCHOR SUBSTITUTIONS MUST BE SUBMITTED TO EOR FOR REVIEW. EOR MAY REQUIRE ENGINEERED CALCULATIONS FOR REVIEW AND APPROVAL.

3. SUBSTITUTION OF EPOXIES IN ONE CONDITION SHALL NOT BE CONSTRUED AS APPROVAL TO MAKE SIMILAR SUBSTITUTION OF EPOXIES IN OTHER DIFFERING CONDITIONS. EACH SUBSTITUTION MUST RECEIVE PRIOR WRITTEN APPROVAL BY THE ENGINEER OF RECORD.

4. INSTALL ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT SPECIFIED ON DRAWINGS.

5. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL TO VERTICALLY OVERHEAD ORIENTATION TO SUPPORT SUSTAINED TENSION LOADS SHALL BE DONE BY A CERTIFIED ADHESIVE ANCHOR INSTALLER (AAI) AS CERTIFIED THROUGH ACI/CRSI (ACI 318-14 D.9.2.2). PROOF OF CURRENT CERTIFICATION SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF INSTALLATION.

6. THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS FOR ALL PRODUCTS TO BE USED PRIOR TO COMMENCEMENT OF WORK. ONLY TRAINED INSTALLERS SHALL PERFORM POST INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE MADE AVAILABLE TO THE EOR AS REQUESTED.

7. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT ALL HOLE CLEAN-OUT REQUIREMENTS ARE FULLY COMPLETED BY THE INSTALLERS PRIOR TO INJECTING EPOXY INTO THE HOLES IN ACCORDANCE WITH THE MANUFACTURER'S MPII.

8. NO LOAD SHALL BE APPLIED TO THE EPOXY ANCHORS UNTIL THE EPOXY HAS FULLY CURED AND HAS ACHIEVED IT'S SPECIFIED STRENGTH. CURE TIME SHALL BE PER MANUFACTURERS PUBLISHED VALUES FOR SPECIFIC PRODUCT BEING USED.

9. IF DETAIL SHOWS EPOXY ANCHORS IN SLOTTED HOLES, IT IS IMPERATIVE THAT ANY EXCESS EPOXY IS CLEANED UP FROM AROUND THE ANCHOR ROD, SO THAT IT DOES NOT INTERFERE WITH ADJUSTABILITY OF ANCHOR ROD IN SLOTTED HOLE.

10. ADHESIVE ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC193 FOR CRACKED, UNCRACKED, AND SEISMIC CONCRETE RECOGNITION.

11. ADHESIVE ANCHORS IN MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED IN ACCORDANCE WITH ICC-ES AC70.

12. EXISTING REINFORCING IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE CUT UNLESS APPROVED BY THE EOR.

13. ADHESIVE ANCHORS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE INSTALLED UNTIL CONCRETE AND/OR MASONRY HAS CURED FOR AT LEAST 21-DAYS.

14. PROVIDE SPECIAL INSPECTION FOR ALL ADHESIVE ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND THE CURRENT ICC-ES REPORT (IBC-2015 TABLE 1705.3 NOTE B).

15. ADHESIVE ANCHORS INSTALLED IN HORIZONTAL OR UPWARDLY INCLINED ORIENTATIONS TO RESIST SUSTAINED TENSION LOADS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION BY AN INSPECTOR SPECIALLY APPROVED FOR THAT PURPOSE BY THE BUILDING OFFICIAL (ACI 318-14 17.8.2.4)

3602 MECHANICAL ANCHORS:

1. SHALL BE EITHER HEAVY DUTY CONCRETE SCREW ANCHOR (SUCH AS DEWALT SCREW-BOLT +, SIMPSON TITEN HD, OR HILTI HUS-H) OR WEDGE TYPE EXPANSION ANCHOR (SUCH AS DEWALT POWER-STUD+SD1, SIMPSON WEDGE-ALL, OR HILTI KWIK BOLT 3).

2. TYPE OF ANCHOR SHALL BE AS SPECIFIED ON THE DRAWINGS, WHILE BRAND AND MODEL OF ANCHOR MAY BE SELECTED FROM THE ABOVE LISTED ANCHORS. SUBSTITUTION ANCHORS MUST BE SUBMITTED TO THE ENGINEER FOR REVIEW AND APPROVED IN WRITING BY THE ENGINEER OF RECORD PRIOR TO USE. ICC-ES REPORTS FOR PROPOSED ANCHOR SUBSTITUTES MUST BE SUBMITTED TO EOR FOR REVIEW. EOR MAY REQUEST ENGINEERED CALCULATIONS FOR REVIEW AND APPROVAL.

3. IN SOME CASES OF CRITICAL LOADING OR GEOMETRIC CONDITIONS, ONLY SPECIFIC ANCHORS WILL BE ALLOWED, AS NOTED ON THE DRAWINGS. IN THESE CASES, THE SPECIFIED BRAND AND MODEL OF ANCHOR MUST BE USED.

4. INSTALL ANCHORS IN STRICT ACCORDANCE WITH MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPII) IN CONJUNCTION WITH EDGE DISTANCE, SPACING, AND EMBEDMENT SPECIFIED ON DRAWINGS.

5. THE MANUFACTURER'S REPRESENTATIVE SHALL TRAIN INSTALLERS FOR ALL PRODUCTS TO BE USED PRIOR TO COMMENCEMENT OF WORK. ONLY TRAINED INSTALLERS SHALL PERFORM POST INSTALLED ANCHOR INSTALLATION. A RECORD OF TRAINING SHALL BE KEPT ON SITE AND MADE AVAILABLE TO THE EOR AS REQUESTED.

6. MINIMUM EMBEDMENT DEPTH OF 1/4" TAPCONS OR POWERS TAPPER + INSTALLED IN CONCRETE SHALL BE 1.25" AND INSTALLED INTO MASONRY SHALL BE 1.5". SELECT ANCHOR LENGTH AS REQUIRED TO ACHIEVE THE SPECIFIED MINIMUM EMBEDMENT DEPTH.

7. TAPCON SCREWS, OR DEWALT TAPPER +, MAY BE REPLACED W/ 0.157" SHANK DIAMETER PAF ANCHORS (HILTI X-U, POWERS CSI, OR APPROVED EQUAL) ON A 1:1 SUBSTITUTION BASIS. MINIMUM EMBEDMENT DEPTH SHALL BE 1.25" WHEN INSTALLED INTO CONCRETE OR GROUTED MASONRY. FOLLOW MANUFACTURER'S INSTALLATION RECOMMENDATIONS, MINIMUM EDGE DISTANCES, AND PLACEMENT LIMITATIONS (RELATIVE TO MORTAR JOINTS IN MASONRY).

8. MECHANICAL ANCHORS IN CONCRETE SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ACI 355.2 AND ICC-ES AC 193 FOR CRACKED, UNCRACKED AND SEISMIC CONCRETE RECOGNITION.

9. MECHANICAL ANCHORS IN MASONRY SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC-ES AC01 OR AC106.

10. POWER ACTUATED FASTENERS SHALL HAVE BEEN TESTED AND QUALIFIED FOR USE IN ACCORDANCE WITH ICC0ES AC70.

11. EXISTING REINFORCING BARS IN CONCRETE AND/OR MASONRY CONSTRUCTION SHALL NOT BE CUT UNLESS APPROVED BY THE EOR.

12. ANCHORS SHALL NOT BE INSTALLED IN CONCRETE AND/OR MASONRY CONSTRUCTION UNTIL THE CONCRETE AND/OR MASONRY HAS CURED FOR AT LEAST 21-DAYS.

13. PROVIDE SPECIAL INSPECTION FOR ALL MECHANICAL POST INSTALLED ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE AND THE CURRENT ICC-ES REPORT (IBC2015 TABLE 1705.3 NOTE B).

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	SH
CHECKED BY:	TL
APPROVED BY:	JRM

PROJECT NO.: 21-003.C

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STRUCTURAL GENERAL NOTES CONTINUED

4810 MASONRY WALLS:

1. ALL MASONRY CONSTRUCTION SHALL CONFORM TO TMS 402 "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" AND TMS 602 "SPECIFICATION FOR MASONRY STRUCTURES", LATEST EDITION.
2. MASONRY UNITS SHALL MEET ASTM C-90 FOR HOLLOW LOAD BEARING TYPE MASONRY WITH UNIT STRENGTH OF 2000 PSI ON THE NET AREA (f'm = 2000 PSI). MORTAR SHALL BE TYPE "M" OR "S" AND MEET ASTM C-270.
3. GROUT SHALL BE 3000 PSI MINIMUM COMPRESSIVE STRENGTH AND MEET ASTM C-476 AND HAVE A SLUMP BETWEEN 8" AND 11" WITH WATER CM RATIO OF 0.55 MAXIMUM AND WITH 3/8" MAXIMUM AGGREGATE.
4. BLOCK CELLS SHALL BE GROUT FILLED WITH VERTICAL REINFORCING BARS AT CORNERS, INTERSECTIONS, EACH SIDE OF OPENINGS AND AS SHOWN ON THE DRAWINGS.
5. DOWELS SHALL BE USED TO PROVIDE CONTINUITY INTO THE STRUCTURE ABOVE AND/OR BELOW, UNLESS NOTED OTHERWISE.
6. USE METAL LATH, MORTAR OR SPECIAL UNITS TO CONFINE CONCRETE AND GROUT TO AREA AS REQUIRED.
7. MASONRY SHALL BE LAID IN RUNNING BOND PATTERN UNLESS NOTED OTHERWISE. AT FILLED CELLS LAY UNITS WITH FULL BED JOINTS AROUND CELLS.
8. PROVIDE 9 GAGE GALVANIZED HORIZONTAL JOINT REINFORCING (DUR-O-WALL OR ENGINEER APPROVED SUBSTITUTION) AT ALTERNATE BLOCK COURSES. LADDER TYPE IS RECOMMENDED WITH REINFORCED FILLED CELLS. PROVIDE PREFABRICATED "TEE" OR CORNER SECTIONS AT WALL INTERSECTIONS.
9. CONTROL JOINTS SHALL BE CONSTRUCTED IN CONCRETE MASONRY CONSTRUCTION AT A MAXIMUM HORIZONTAL SPACING BETWEEN JOINTS OF 25'-0" AND NOT MORE THAN 12'-6" FROM CORNERS. SEE ARCHITECTURAL DRAWINGS FOR EXACT LOCATIONS. CONSTRUCT INTERIOR CONTROL JOINTS AT A MAXIMUM HORIZONTAL SPACING OF 32'-0" OR 16'-0" FROM CORNERS. NO JOINTS SHALL BE LOCATED WITHIN 2'-0" OF STEEL BEAM BEARINGS. HORIZONTAL WALL REINFORCING SHALL BE STOPPED EACH SIDE OF CONTROL JOINTS. SEE ARCHITECTURAL DRAWINGS FOR SEALANT REQUIREMENTS AT CONTROL JOINTS.
10. SUBMIT PROPOSED GROUT MIX DESIGNS FOR REVIEW PRIOR TO USE. MIX NUMBER OR OTHER POSITIVE IDENTIFICATION SHALL UNIQUELY IDENTIFY MIX.
11. USE OF SUPERPLASTICIZER IS PROHIBITED.
12. CELLS TO BE GROUT FILLED SHALL HAVE VERTICAL ALIGNMENT SUFFICIENT TO MAINTAIN A CLEAR, UNOBSTRUCTED, CONTINUOUS VERTICAL GROUT SPACE.
13. CLEANOUT OPENINGS SHALL BE PROVIDED AT THE BOTTOM OF CELLS TO BE GROUT FILLED IN EACH POUR IN EXCESS OF 5 FEET IN HEIGHT. AFTER INSPECTION AND BEFORE GROUTING, THE REBAR SHALL BE TIED AT THE CLEANOUTS AND THE CLEANOUTS SHALL BE SEALED.
14. ANY OVERHANGING MORTAR OR OTHER OBSTRUCTION OR DEBRIS SHALL BE REMOVED FROM THE INSIDES OF SUCH CELL WALLS.
15. VERTICAL REINFORCEMENT SHALL BE HELD IN POSITION AT TOP AND BOTTOM AND AT INTERVALS NOT EXCEEDING 192 BAR DIAMETERS.
16. CELLS CONTAINING REINFORCEMENT SHALL BE FILLED SOLIDLY WITH GROUT. SAMPLE AND TEST GROUT PER ASTM C1019.
17. GROUT SHALL BE POURED IN LIFTS OF 4 FEET MAXIMUM HEIGHT. GROUT SHALL BE CONSOLIDATED AT TIME OF PLACING BY VIBRATING AND RECONSOLIDATED LATER BY VIBRATING BEFORE PLASTICITY IS LOST.
18. WHEN TOTAL GROUT POUR EXCEEDS 5'-4" FEET IN HEIGHT, (HIGH LIFT GROUTING), THE GROUT SHALL BE PLACED IN 4-FOOT LIFTS WITH A MINIMUM OF A 30 MINUTE DELAY BETWEEN LIFTS. MINIMUM CELL DIMENSION SHALL BE IN ACCORDANCE WITH TABLE 5 OF TMS 402 (3" X 3" FOR COARSE GROUT, 12 FT. MAXIMUM POUR HEIGHT).
19. WHEN THE GROUTING IS STOPPED FOR ONE HOUR OR LONGER, HORIZONTAL CONSTRUCTION JOINTS SHALL BE MADE BY STOPPING THE POUR OF GROUT NOT LESS THAN 1-1/2 INCH BELOW THE TOP OF THE UPPERMOST UNIT GROUTED.
20. WHERE CONCRETE BEAMS ARE INSTALLED IN CONCRETE MASONRY WALL, SUPPORT CONCRETE WITH 6" SIDE CONTINUOUS STRIPS OF 1/8 SQUARE MESH SOFFIT SCREENING OR PUR-O-STOP OF EQUAL CENTERED OVER BLOCK WORK. USE OF ROOFING FELT STRIPS WILL NOT BE PERMITTED.
21. SUBMIT WRITTEN CONSTRUCTION SEQUENCES AND PROCEDURES PRIOR TO THE START OF MASONRY CONSTRUCTION.
22. REINFORCING SHALL BE ASTM A615 GRADE 60 DEFORMED BARS, FREE FROM OIL, SCALE AND RUST AND PLACED IN ACCORDANCE WITH THE TYPICAL BENDING DIAGRAM AND PLACING DETAILS OF ACI STANDARDS AND SPECIFICATIONS.
23. SECURE APPROVAL OF REINFORCING SHOP DRAWINGS PRIOR TO COMMENCING FABRICATION.
24. PROVIDE STANDARD HOOKS AT ENDS OF ALL BARS WHICH TERMINATE IN TIE BEAMS OR BOND BEAMS.
25. WHERE REINFORCING IS SHOWN CONTINUOUS, LAP SPLICE BARS IN ACCORDANCE WITH SPLICE INDICATED BELOW.
26. PROVIDE HOOKED DOWELS INTO FOOTINGS, PILE CAPS, SUPPORT BEAMS, ETC. TO MATCH VERTICAL BARS WITH LAP SPLICES AS FOLLOWS:

#4	24"
#5	30"
#6	48"
#7	60"

27. MECHANICAL BAR COUPLERS MAY BE USED TO SPLICE CONTINUOUS BARS, IN LIEU OF LAP SPLICES. BAR COUPLERS MUST ACHIEVE 125% OF BAR STRENGTH MINIMUM. COUPLERS MAY BE BOLTED TYPE (DAYTON D-250 BAR-LOCK S-SERIES COUPLER OR EQUAL) OR THREADED TYPE (DAYTON D310 TAPER-LOCK COUPLER OR EQUAL). COUPLERS SHALL BE INSTALLED PER MANUFACTURER'S WRITTEN INSTALLATION RECOMMENDATIONS.
28. AT CHANGES IN DIRECTION OF BOND BEAMS, PROVIDE CORNER BARS OF SAME SIZE AND SPACING AS HORIZONTAL STEEL.

4811 MASONRY WALLS BRACING:

1. MASONRY WALLS HAVE BEEN DESIGNED TO WITHSTAND GRAVITY LOADS AND WIND LOADS AS INDICATED ON THE DRAWINGS IN THE FINAL, COMPLETED STRUCTURAL CONFIGURATION. HOWEVER, NO ALLOWANCE HAS BEEN MADE FOR TEMPORARILY BRACED STRESSES OR CONDITIONS.
2. SHOP DRAWINGS AND CALCULATIONS FOR TEMPORARY BRACING SHALL BE SIGNED AND SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA (IN WHICH THE PROJECT RESIDES).
3. THE DESIGNERS OF THE TEMPORARY BRACING MUST CONSIDER THE STRENGTH OF GROUT AND MORTAR AT THE TIME OF BRACING AS WELL AS THE REINFORCING ALREADY PRESENT IN THE WALLS AS INDICATED IN THIS SET OF CONSTRUCTION DRAWINGS. IF ADDITIONAL WALL REINFORCING IS REQUIRED FOR THE TEMPORARY BRACED CONDITION, THE BRACING DESIGN ENGINEER MUST DESIGN THIS ADDITIONAL REINFORCING, AND THIS ADDITIONAL REINFORCING MUST BE CLEARLY DETAILED IN THE BRACING SUBMITTAL. THE GENERAL CONTRACTOR SHALL COORDINATE ANY ADDITIONAL REINFORCING FOR THE BRACED CONDITION WITH THE MASONRY SUB-CONTRACTOR.
4. TEMPORARY BRACING OF MASONRY WALLS DURING CONSTRUDICTION:
   - 4.1 MASONRY WALL DESIGN AS DEPICTED ON DESIGN DRAWINGS IS BASED ON THE FINAL CONSTRUCTION CONFIGURATION INCLUDING INSTALLATION OF PERMANENT BRACING INCLUDING, BUT NOT LIMITED TO FLOOR, ROOF FRAMING, METAL DECK AND SLAB SYSTEMS. THE DESIGN DOES NOT ADDRESS MEANS AND METHODS OF CONSTRUCTION OR TEMPORARY CONSTRUCTION CONDITIONS.
   - 4.2 IT IS THE FULL RESPONSIBILITY OF THE CONTRACTOR TO ERECT MASONRY WALLS IN A SAFE MANNER, AND TO ADEQUATELY BRACE, OR OTHERWISE PROTECT THE WALLS AGAINST WIND OR OTHER FORCES/EFFECTS DURING CONSTRUCTION UNTIL THE FINAL PERMANENT STRUCTURAL SYSTEM IS COMPLETED.
   - 4.3 CONTRACTOR TO SUBMIT COMPLETE PROPOSED BRACING PLANS AND CALCULATIONS, PREPARED SPECIFICALLY FOR THIS PROJECT BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF FLORIDA (IN WHICH THE PROJECT RESIDES) OUTLINING PROPOSED TEMPORARY BRACING SYSTEM, SEQUENCE OF CONSTRUCTION OF BRACING, WITH CONSIDERATION OF CONFLICT WITH OTHER BUILDING SYSTEMS AND COMPONENTS, AND EVALUATION OF WALLS OR FLOOR SLABS TO ACCOMMODATE TEMPORARY LOADS AND UNBRACED CONDITIONS, AS WELL AS BRACING FORCE RESOLUTION.
   - 4.4 ANY DESIGN OR MODIFIED CONSTRUCTION REQUIRED TO ACCOMMODATE PROPOSED BRACING SCHEME IS THE FULL RESPONSIBILITY OF THE CONTRACTOR, INCLUDING ANY ADDITIONAL REINFORCING OF MASONRY WALLS OR INSTALLATION OF DEADMEN OR ADDITIONAL SLAB REINFORCEMENT. GENERAL BRACING SCHEMES DEVELOPED BY BRACING MANUFACTURERS ARE NOT SUFFICIENT OR ACCEPTABLE.
   - 4.5 TEMPORARY BRACING LOADING SHALL BE IN COMPLIANCE WITH DESIGN DOCUMENTS, OSHA 1926.706 REQUIREMENTS, THE LATEST EDITION OF ASCE 7, AND THE LATEST EDITION OF ASCE 37. ACTUAL WIND PRESSURE IS TO BE CALCULATED, BUT IN NO CASE SHOULD IT BE BASED ON LESS THAN 70 MPH (3 SECOND GUST) NOMINAL WIND SPEED. CONSTRUCTION FIELD OFFICE TO MONITOR NATIONAL WEATHER SERVICE FORECASTS AND BE PREPARED TO SHUT DOWN AND CLEAR PROJECT SITE IF DESIGN WIND SPEED IS ANTICIPATED TO BE EXCEEDED.
   - 4.6 CONTRACTOR'S SPECIALTY BRACING ENGINEER OR DESIGNATED REPRESENTATIVE (MINIMUM 5 YEARS EXPERIENCE WITH MASONRY WALL BRACING SYSTEMS) SHALL VISIT SITE DURING WALL BRACING INSTALLATION TO CONFIRM THAT BRACING IS IN COMPLIANCE WITH DESIGN INTENTIONS, AND SUBMIT REPORT OF FINDINGS TO CONTRACTOR AND STRUCTURAL ENGINEER OF RECORD.

5120 STRUCTURAL STEEL:

1. STEEL WORK SHALL BE NEW AND CONFORM TO THE ANSI/AISC 360-16 SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS.
2. MATERIAL SHALL CONFORM TO THE FOLLOWING, EXCEPT AS NOTED:

WIDE FLANGE SHAPES	ASTM A992 (Fy=50 KSI)
ANGLES, CHANNELS AND PLATES	ASTM A36 (Fy=36 KSI)
RECTANGULAR HSS	ASTM A500, GRADE B (Fy=46 KSI)
HIGH STRENGTH BOLTS	ASTM A325 OR A490
THREADED RODS	ASTM A36 (Fy=36 KSI)
HEAVY HEX NUTS	ASTM A563
HARDENED STEEL WASHERS	ASTM F436
ANCHOR RODS	ASTM F1554 GR. 36 (Fy=36 KSI)

3. CONNECTIONS:
   - A. BOLTS SHALL BE HIGH-STRENGTH, BEARING TYPE IN SNUG TIGHT CONDITION, U.N.O. TIGHTEN BY AN AISC APPROVED METHOD.
   - B. WELDING ELECTRODES SHALL BE PER AWS D1.1. RETURN FILLET WELDS FOR FRAMED CONNECTIONS 1/2" AT EACH END.
   - C. FIELD CONNECTIONS SHALL BE BOLTED, EXCEPT AS NOTED OTHERWISE.
   - D. DETAIL FLOOR AND ROOF FRAMING CONNECTIONS FOLLOWING THE REQUIREMENTS SHOWN IN THE TYPICAL CONNECTION SCHEDULES SHOWN IN THESE DRAWINGS, BASED ON THE BEAM OR GIRDER SIZE.
   - E. FOR THE PURPOSE OF CORRECTLY INTERPRETING THE CONNECTION SCHEDULES, GIRDERS SHALL BE CONSIDERED AS ANY FLOOR OR ROOF BEAM WHICH CARRIES OTHER FLOOR OR ROOF BEAMS, OR ANY FLOOR OR ROOF BEAM WHICH CARRIES STEEL COLUMNS.
   - F. DETAIL DIAGONAL BRACING CONNECTIONS AS SHOWN IN THE DETAILS. IF NO DETAIL IS PROVIDED, DETAIL CONNECTION TO DEVELOP THE FULL TENSION CAPACITY OF THE DIAGONAL BRACING MEMBER.
   - G. DETAIL MOMENT CONNECTIONS AS SHOWN IN THE DETAILS. IF NO DETAIL IS PROVIDED, DETAIL MOMENT CONNECTION USING FULL PENETRATION WELDS AT BEAM FLANGES.
4. HIGH STRENGTH BOLTS IN BEARING CONDITION SUPPORTING SIMPLE SPAN BEAMS NOT SUBJECT TO AXIAL LOADS MAY BE INSTALLED TO "SNUG TIGHT" CONDITION IF NORMAL, OR SHORT SLOTTED HOLES ARE USED. THE ENGINEER OF RECORD WILL BE THE ULTIMATE AUTHORITY IN THE USE OF "SNUG TIGHT" BOLTS. IF LONG SLOTTED OR OVERSIZED HOLES ARE USED, BOLTS MUST BE FULLY PRETENSIONED AND SLIP CRITICAL. PROPER SURFACE PREPARATION IS REQUIRED FOR SLIP CRITICAL BOLTS, INCLUDING OMISSION OF PRIMER OR FIRE PROOFING, AS APPROPRIATE.
5. BOLTS SHARING LOAD WITH WELDS IN A CONNECTION SHALL BE FULLY PRETENSIONED AND SLIP CRITICAL.
6. WHERE FULLY PRETENSIONED OR SLIP CRITICAL BOLTS ARE REQUIRED, TIGHTENING SHALL BE ACHIEVED USING EITHER TWIST-OFF TENSION CONTROL BOLTS OR DIRECT TENSION INDICATING WASHERS.
7. ALL STRUCTURAL STEEL EXPOSED TO EXTERIOR CONDITIONS SHALL BE HOT DIPPED GALVANIZED PER ASTM A123 AND ALL FASTENERS AND HARDWARE SHALL BE HOT DIPPED GALVANIZED PER ASTM A153.
8. GROUT UNDER BEARING PLATES SHALL BE NON-METALLIC, NON-SHRINK TYPE WITH A COMPRESSIVE STRENGTH OF AT LEAST 5,000 PSI IN 28 DAYS.
9. COMPOSITE FLOOR MEMBERS ARE DESIGNED TO BE UNSHORED UNLESS OTHERWISE NOTED. THE WEIGHT OF THE WET CONCRETE WILL RESULT IN DEFLECTIONS OF THE SUPPORTING STEEL DECK, BEAMS, AND GIRDERS. ALL OVERRUNS OF CONCRETE QUANTITIES ARE TO BE ANTICIPATED AND INCLUDED IN THE CONTRACTOR'S BID. THE CONTRACTOR SHALL COORDINATE EMBEDDED ITEMS REQUIRED FOR ARCHITECTURAL, STRUCTURAL, AND MECHANICAL ELEMENTS. CONCRETE FLOORS UTILIZING UNSHORED CONSTRUCTION SHALL BE SCREEDED LEVEL.
10. SIZE AND SPACING OF CONDUITS IN COMPOSITE SLABS SHALL COMPLY WITH THE REQUIREMENTS OF ASCE 3-91 UNLESS NOTED OTHERWISE ON DRAWINGS.
11. LENGTH OF SHEAR STUD CONNECTIONS IN COMPOSITE SLABS SHALL EQUAL THE DEPTH OF THE COMPOSITE DECK PLUS 2" (U.N.O.).
12. THE CAMBER OF STEEL MEMBERS SHALL BE VERIFIED IN THE SHOP AND THE FIELD. WHEN NO CAMBER IS INDICATED, TURN THE MEMBER NATURAL CAMBER UP.
13. WHERE TRUSSES ARE INDICATED WITH CONTINUOUS MEMBERS, THEY SHALL BE FULL LENGTH WITHOUT SPLICES OR WELDED WITH FULL PENETRATION SHOP WELDS GROUND FLUSH WITH GRINDING IN THE DIRECTION OF APPLIED STRESS AND WITH WELD SOUNDNESS ESTABLISHED BY RADIOGRAPHIC OR ULTRASONIC INSPECTION IN ACCORDANCE WITH THE REQUIREMENTS OF 4.2.5.2 OR 4.2.5.3 OF AWS D1.1. SPLICES WILL NOT BE PERMITTED AT POINTS OF MAXIMUM STRESS. FIELD SPLICES OF TENSION MEMBERS SHALL BE DESIGNED TO DEVELOP 110 PERCENT OF THE SPLICED SECTIONS.
14. APPLY FIREPROOFING TO STEEL STRUCTURE CALCULATING THE THICKNESS OF FIREPROOFING BY COMPARING THE ACTUAL MEMBER SIZE TO THE MEMBER SIZE USED IN THE DESIGNATED UL RATING AND ADJUSTING APPROPRIATELY.
15. TRUSS STEEL IS ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS) AND SHALL CONFORM TO SECTION 10 OF THE AISC CODE OF STANDARD PRACTICE. AESS SHALL BE SANDBLASTED (SSPC-SP6-82) PRIOR TO PRIMER COAT APPLICATION. PRIMER SHALL BE COMPATIBLE WITH FINAL PAINT COAT AND SHALL BE APPROVED BY FINISH PAINT CONTRACTOR.
16. USE INTUMESCENT PAINT ON STRUCTURAL STEEL EXPOSED TO VIEW WHICH REQUIRES FIRE RATING. SEE ARCHITECTURAL DRAWINGS FOR FIRE RATING AND INTUMESCENT PAINT REQUIREMENTS.

5122 WELDING:

1. WELDING SHALL BE DONE BY WELDERS WITH CURRENT CERTIFICATION IN ACCORDANCE WITH AWS D1.1.
2. WELDS SHOWN ON STRUCTURAL DRAWINGS ARE MINIMUM DESIGN REQUIREMENTS. THE FABRICATOR'S SHOP DRAWINGS SHALL REFLECT WELDS IN ACCORDANCE WITH AWS REQUIREMENTS.
3. FULL PENETRATION GROOVE WELDS SHALL BE INSPECTED BY ULTRASONIC TESTING. TWENTY-FIVE PERCENT OF THE WELDS SHALL BE INSPECTED AT RANDOM UNLESS NOTED OTHERWISE. SEE SPECIFICATIONS FOR ADDITIONAL REQUIREMENTS.
4. UNLESS NOTED OTHERWISE ON THE DRAWINGS, GROOVE WELDS SHALL BE FULL PENETRATION.
5. PROVIDE FILLET WELDS AT CONTACT POINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE STRENGTH OF THE SMALLER MEMBER AT THE JOINT UNLESS DETAILED OTHERWISE ON THE DRAWINGS. THE MINIMUM FILLET WELD SIZE IS 3/16" UNLESS OTHERWISE NOTED.
6. MINIMUM TENSILE STRENGTH OF ELECTRODE MATERIAL F_{EXX} = 70ksi UNLESS NOTED OTHERWISE.

5230 COMPOSITE STEEL DECK:

1. SHALL BE GALVANIZED, CORRUGATED STEEL COMPOSITE DECK OF GAGE AND DEPTH INDICATED ON DRAWINGS, CONFORMING TO STEEL DECK INSTITUTE SPECIFICATION FOR COMPOSITE FLOOR DECK.
2. DECK SHALL HAVE DEFORMATIONS TO PROVIDE ADEQUATE MECHANICAL INTERLOCKING BETWEEN DECK AND CONCRETE FOR COMPOSITE ACTION.
3. PROVIDE 5/8" DIAMETER PUDDLE WELDS AT 12" C/C AT EACH SUPPORT.
4. SIDE LAPS BETWEEN DECKS SHALL BE WELDED OR FASTENED USING #10 TEK SCREWS AT A MAXIMUM SPACING OF 36 INCHES.
5. BUTTON PUNCHING OF SIDE LAPS IS NOT PERMITTED.

5312 STEEL ROOF DECK:

1. SHALL BE GALVANIZED (G30/G60/G90), TYPE "B" STEEL ROOF DECK OF GAGE AND DEPTH INDICATED ON DRAWINGS, AND SHALL CONFORM TO THE PROVISIONS OF THE STEEL DECK INSTITUTE (SDI) SPECIFICATIONS FOR STEEL ROOF DECK.
2. DECK CENTERING SHALL BE PLACED IN CONFORMANCE WITH MANUFACTURER'S RECOMMENDATIONS AND SHALL BE CONTINUOUS OVER AT LEAST 3 SPANS.
3. WELD PATTERN AT MAIN DECK SUPPORTS, AND SIDELAP CONNECTIONS, SHALL BE AS INDICATED ON ROOF DECK FASTENING DIAGRAM. PROVIDE 5/8" PUDDLE WELDS AT 12" O.C. ALONG EDGE SUPPORTS, UNLESS NOTED OTHERWISE.
4. DECK SHALL BE TOPPED WITH TYPE II INSULATING FILL CONSISTING OF RIGID INSULATING BOARD COVERED WITH TWO OR MORE INCHES OF LIGHT WEIGHT INSULATING CONCRETE OVER THE TOP (MAX. 30 P.C.F. OVEN DRY DENSITY).
5. METAL DECK MANUFACTURER SHALL BE A MEMBER OF THE STEEL DECK INSTITUTE AND ALL DESIGN SHALL BE IN ACCORDANCE WITH APPLICABLE STANDARDS.
6. DO NOT HANG MEP SYSTEMS (DUCTWORK, ROOF DRAIN OR FIRE PROTECTION PIPING, ETC) FROM ROOF DECK. ALL EQUIPMENT IS TO BE HUNG FROM ROOF BEAMS.

5400 STRUCTURAL COLD-FORMED STEEL (CFS) FRAMING

1. ALL COLD FORMED STEEL FRAMING SHALL CONFORM TO THE AISI/COFS/NASPEC 2016 NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS WITH SUPPLEMENT NO. 2, THE AISI MANUAL COLD-FORMED STEEL DESIGN (2013 EDITION), AISI CODE OF STANDARD PRACTICE FOR COLD-FORMED STEEL STRUCTURAL FRAMING (2015 EDITION), AISI/COFS/GP-2015 STANDARD FOR COLD-FORMED STEEL FRAMING - GENERAL PROVISIONS (2015 EDITION) AND COLD-FORMED STEEL ENGINEERS INSTITUTE (CFSEI) PUBLICATIONS, AS MODIFIED OR CLARIFIED HEREIN.
2. STEEL STUDS, JOISTS, LINTELS, AND RUNNER TRACK MEMBERS SHALL BE OF TYPE SHOWN ON THE DRAWINGS AND SPECIFICATIONS CONFORMING TO ASTM A-446 GRADE C WITH HOT DIPPED GALVANIZED COATING CONFORMING TO ASTM A525 (CLASS G90).
3. MINIMUM STEEL GRADES (FY): 12 GA (97 MILS), 14 GA (68 MILS); 16 GA (54 MILS) STUDS AND TRACK: 50 KSI; 18 GA (43 MILS), 20 GA (33 MILS) STUDS AND TRACK: 33 KSI.
4. STRUCTURAL LIGHT GAGE CFS FRAMING AND THEIR CONNECTIONS SHALL BE AS DEPICTED ON THE STRUCTURAL PLANS AND DETAILS.
5. ATTACHMENTS, CLOSURES, HARDWARE, ETC., SHALL BE AS SHOWN AND/OR IN ACCORDANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
6. SUBMIT PROPOSED LIGHT GAGE CFS MANUFACTURERS DATA AND LOAD TABLES FOR REVIEW.
7. TOLERANCES TO COMPLY WITH ASTM C955.
8. WELDING TO COMPLY WITH COLD-FORMED STEEL ENGINEERS INSTITUTE TECH NOTE 5606-1 - WELDING COLD-FORMED STEEL.
9. REPAIR DAMAGED OR UNCOATED GALVANIZED COATINGS PER ASTM A780.
10. FRAMING MEMBERS SHALL BE CUT SQUARELY OR AT AN ANGLE AS REQUIRED TO FIT SQUARELY AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN PLACE UNTIL PROPERLY JOINED.
11. JOINING OF STRUCTURAL MEMBERS SHALL BE MADE WITH SELF-DRILLING SCREWS OR WELDING.
12. WIRE TYING OF FRAMING MEMBERS IN STRUCTURAL APPLICATIONS SHALL NOT BE PERMITTED.
13. ATTACHMENT OF COLLATERAL MATERIALS TO STEEL MEMBERS SHALL BE MADE WITH SELF-DRILLING SCREWS OR HARDENED SCREW SHANK NAILS.
14. STUDS SHALL SIT SQUARELY IN THE TOP AND BOTTOM RUNNER TRACK WITH FIRM ABUTMENT AGAINST TRACK WEBS. STUDS SHALL BE ALIGNED OR PLUMBED AND SECURELY FASTENED TO THE FLANGES OF BOTH TOP AND BOTTOM RUNNER TRACK.
15. BRIDGING TO BE SUPPLIED AND INSTALLED PER CFS STUD MANUFACTURER RECOMMENDATIONS (5'-0" O.C. MAX AND WITHIN 1'-0" OF DEFLECTION TRACKS).
16. LATERAL BRACING SHALL BE PROVIDED BY USE OF PLYWOOD SHEATHING, GYPSUM SHEATHING, OR BY HORIZONTAL STRAPS OR COLD-ROLLED CHANNELS. BRACING SHALL CONFORM TO SECTION D3 OF THE AISI SPECIFICATIONS.
17. COLD-FORMED STEEL FRAMING CONNECTORS SPECIFIED BY PART NUMBER OR MODEL NAME ARE STANDARD CONNECTORS FABRICATED BY THE STEEL NETWORK (TSN), RALEIGH, N.C., 888-474-4876. CONNECTORS SHALL BE INSTALLED PER THE REQUIREMENTS OF THE LIGHT STEEL FRAMING CONNECTIONS CATALOG (LATEST EDITION) BY THE STEEL NETWORK, USING THE NUMBER/SIZE OF FASTENERS SHOWN IN THESE DRAWINGS. WHERE NUMBER/SIZE IS NOT SHOWN IN THESE DRAWINGS, USE THE FASTENERS SPECIFIED IN THE CATALOG TO OBTAIN THE MAXIMUM CAPACITY OF THE SPECIFIED CONNECTOR. SUBSTITUTION OF GENERIC BENT PLATE LIGHT GAGE CONNECTORS IS NOT ALLOWED WITHOUT DESIGN CALCULATIONS SHOWING EQUAL OR BETTER CAPACITY TO THE SPECIFIED TSN CONNECTOR, PREPARED AND SIGNED AND SEALED BY A FLORIDA REGISTERED ENGINEER EXPERIENCED IN LIGHT GAGE STEEL CONNECTION DESIGN.

5714 STEEL STAIRS:

1. ENGINEERED STEEL STAIR SYSTEM AND CONNECTIONS OF SAME TO THIS STRUCTURE SHALL BE DESIGNED BY AN ENGINEER REGISTERED IN THE TERRITORY OF THE US VIRGIN ISLANDS.
2. THE CONFIGURATION OF THE STEEL STAIR SYSTEM SHALL BE AS SHOWN ON THE ARCHITECTURAL DRAWINGS.
3. THE ENGINEERED STAIR SYSTEM SHALL INCLUDE THE STAIRS, LANDINGS, SUPPORT FRAMING, POSTS, HANGERS, AND CONNECTIONS TO THE BUILDING STRUCTURE, UNLESS NOTED OTHERWISE. CONNECTIONS TO THE BUILDING STRUCTURE SHALL BE COMPATIBLE WITH THE STRUCTURE SHOWN ON THE CONTRACT DRAWINGS.
4. STEEL STAIR SYSTEM AND CONNECTIONS SHALL BE DESIGNED FOR APPLICABLE LOADS AS INDICATED ON THE DRAWINGS AND IN THE 2021 INTERNATIONAL BUILDING CODE.
5. THE LOADS SHALL BE CLEARLY INDICATED ON SHOP DRAWINGS.
6. SHOP DRAWINGS SHALL SHOW AND SPECIFY CONNECTIONS UTILIZED WITHIN THE STEEL STAIR SYSTEM AS WELL AS CONNECTIONS TO AND LOADS IMPOSED UPON THE STRUCTURAL SYSTEM SHOWN ON THESE DRAWINGS.
7. SHOP DRAWINGS AND CALCULATIONS SHALL BE SIGNED AND SEALED BY AN ENGINEER REGISTERED IN THE TERRITORY OF THE US VIRGIN ISLANDS.

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
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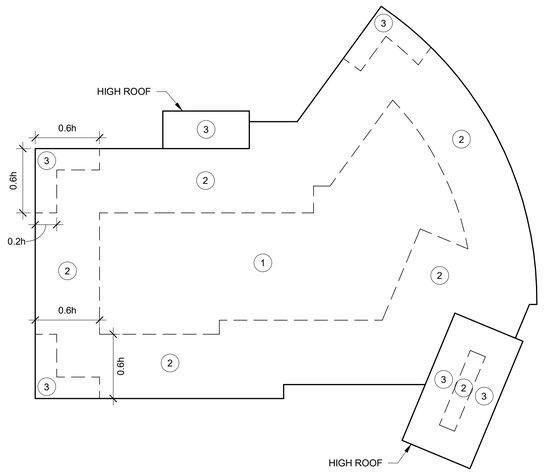
STRUCTURAL GENERAL NOTES CONTINUED



WIND PLAN

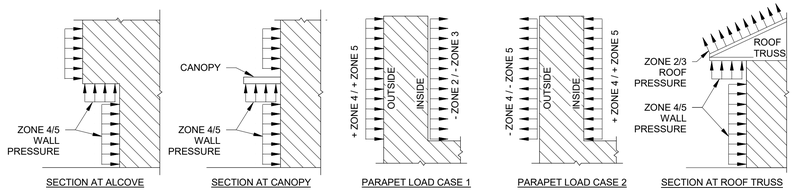
ULTIMATE C&C WIND PRESSURES (ASCE 7-16)												
BUILDING	a (FT)	Vult (MPH)	Vasd (MPH)	A (SF)	ZONE 1 (PSF)	ZONE 1' (PSF)	ZONE 2 (PSF)	ZONE 3 (PSF)	A (SF)	ZONE 4 (PSF)	ZONE 5 (PSF)
MAIN	24	217	168	<10	+60.1 -235.4	+60.1 -135.2	+60.1 -310.5	+60.1 -310.5	10	+147.7 -160.3	+147.7 -197.8
				20	+48.5 -187.4	+48.5 -135.2	+48.5 -248.9	+48.5 -248.9	100	+125.6 -138.1	+125.6 -153.6
				50	+51.3 -199.3	+51.3 -135.2	+51.3 -264.1	+51.3 -264.1	200	+119.0 -131.5	+119.0 -140.3
				100+	+47.6 -183.8	+47.6 -135.2	+47.6 -244.2	+47.6 -244.2	500	+110.2 -122.7	+110.2 -122.7

ULTIMATE C&C WIND PRESSURE PLAN NOTES:

1. WIND PRESSURE TABLE IS BASED ON IBC 2021/ASCE 7-16 ULTIMATE WIND SPEED, PRESSURES SHOWN ABOVE ARE ULTIMATE COMPONENTS AND CLADDING PRESSURES. NOMINAL PRESSURE (ASD) CAN BE OBTAINED BY MULTIPLYING BY 0.6.

   A - INDICATES TRIBUTARY AREA IN S.F.
   a - INDICATES END ZONE WIDTH IN FT.
   Vult - INDICATES ULTIMATE DESIGN WIND SPEED IN MPH
   Vasd - INDICATES NOMINAL DESIGN WIND SPEED IN MPH

2. GROSS PRESSURES ARE FOR JOISTS, WINDOWS, DOORS, VENEER, LIGHT GAGE METAL FRAMING, METAL DECK ATTACHMENTS, ROOFING, ROOFING ACCESSORIES AND OTHER BUILDING COMPONENTS AND CLADDING.
3. GROSS PRESSURES SHALL BE LINEARLY INTERPOLATED FOR (A) NOT SHOWN IN TABLE.
4. POSITIVE PRESSURES INDICATE PRESSURES ACTING TOWARD A PROJECTED SURFACE. NEGATIVE PRESSURES INDICATE PRESSURES ACTING AWAY FROM A PROJECTED SURFACE.
5. ROOF AND ZONES (1) THRU (3)
6. WALL ZONES (4) AND (5)
7. OVERHANG ZONES (2H) AND (3H) APPLY ONLY TO ROOF OVERHANGS WHERE THE COMPONENT OR CLADDING RECEIVES PRESSURE SIMULTANEOUSLY ON BOTH SIDES (UPWARD SUCTION ON TOP AND UPWARD PRESSURE ON BOTTOM, SUCH AS AT OPEN SOFFITS), AND IS CONTINUOUS WITH FIELD OF ROOF.
8. NET DESIGN ROOF PRESSURES SHALL BE CALCULATED USING THE SELFWEIGHT (DEAD LOAD) OF THE MATERIALS. HOWEVER, THE MAXIMUM REDUCTION OF WIND UPLIFT PRESSURES SHALL BE LIMITED TO THE SELF WEIGHT OF THE ROOF SYSTEM PLUS 5 PSF FOR SUPERIMPOSED DEAD LOADS.
9. INTERNAL PRESSURE COEFFICIENT FOR ENCLOSED BUILDING EQUALS +0.18 AND -0.18
   INTERNAL PRESSURE COEFFICIENT FOR OPEN STRUCTURE EQUALS +/- 0.00
   INTERNAL PRESSURE COEFFICIENT FOR PARTIALLY ENCLOSED STRUCTURE EQUALS +/- 0.55
10. ROOF TOP EQUIPMENT SHALL BE DESIGNED FOR A LATERAL PRESSURE OF 237 PSF AND A SIMULTANEOUS UPLIFT PRESSURE OF 188 PSF.
11. AT ALCOVES AND CANOPIES, THE TOTAL UPLIFT PRESSURE ON THE ALCOVE SOFFIT OR CANOPY SHALL EQUAL THE WALL PRESSURE IN THAT AREA.



SECTION AT ALCOVE | SECTION AT CANOPY | PARAPET LOAD CASE 1 | PARAPET LOAD CASE 2 | SECTION AT ROOF TRUSS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY
INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: SH
CHECKED BY: TL
APPROVED BY: JRM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S0.5

COMPONENT WIND CRITERIA

DEFORMED BAR TENSION DEVELOPMENT AND LAP SPLICE LENGTH SCHEDULE (INCHES)									
$f'c$ (PSI)	3000								
BAR TYPE	BAR SIZE								
	#3	#4	#5	#6	#7	#8	#9	#10	#11
FOOTINGS	22	29	36	43	63	72	81	91	101
COLUMN	22	29	36	43	63	72	81	91	101
WALLS	22	29	36	65	94	107	121	136	151
SLABS	22	29	36	65	94	107	121	136	151
BEAMS (TOP)	28	38	47	56	81	93	105	118	131
BEAMS (MID & BOT)	22	29	36	43	63	72	81	91	101
FILLED CELLS	SEE MASONRY NOTES								

TENSION DEVELOPMENT AND LAP SPLICE LENGTH SCHEDULE (INCHES)									
$f'c$ (PSI)	4000								
BAR TYPE	BAR SIZE								
	#3	#4	#5	#6	#7	#8	#9	#10	#11
FOOTINGS	19	25	31	37	54	62	70	79	87
COLUMN	19	25	31	37	54	62	70	79	87
WALLS	19	25	31	56	81	93	105	118	131
SLABS	19	25	31	56	81	93	105	118	131
BEAMS (TOP)	25	33	41	49	71	81	91	102	114
BEAMS (MID & BOT)	19	25	31	37	54	62	70	79	87
FILLED CELLS	SEE MASONRY NOTES								

TENSION DEVELOPMENT AND LAP SPLICE LENGTH SCHEDULE (INCHES)									
$f'c$ (PSI)	5000								
BAR TYPE	BAR SIZE								
	#3	#4	#5	#6	#7	#8	#9	#10	#11
FOOTINGS	17	23	28	34	49	56	63	71	78
COLUMN	17	23	28	34	49	56	63	71	78
WALLS	17	23	28	50	73	83	94	106	117
SLABS	17	23	28	50	73	83	94	106	117
BEAMS (TOP)	22	29	36	44	63	72	81	92	102
BEAMS (MID & BOT)	17	23	28	34	49	56	63	71	78
FILLED CELLS	SEE MASONRY NOTES								

NOTES:

1. VALUES SHALL BE INCREASED BY 1.5 IN CASES OF COVER LESS THAN d_b AND/OR SPACING LESS THAN $2d_b$.

CONTINUOUS FOUNDATIONS SCHEDULE					
MARK	SIZE		REINFORCEMENT		REMARKS
	WIDTH	THICKNESS	LONGITUDINAL	TRANSVERSE	
CF3.0	3' - 0"	1' - 8"	(4) #6 BARS T&B	#6 @ 12" O.C. T&B	
CF8.0	8' - 0"	2' - 0"	#8 BARS @ 12" O.C. T&B	#8 BARS @ 12" O.C. T&B	CURVED FTG., MANTAIN TRANSVERSE. REINF. PERPENDICULAR TO CURVATURE.
CF15.0					

CONCRETE WALL SCHEDULE			
MARK	WIDTH	REINFORCEMENT	REMARKS
CW-1	8"	#7 @ 10" OC VERT, #5 @ 12" OC HORIZ	
CW-2	1' - 4"	#6 @ 8" OC EF VERT, #5 @ 12" OC EF HORIZ	
CW-3	1' - 0"	#6 @ 12" OC EF VERT, #5 @ 12" OC EF HORIZ	
CW-4	8"	#5 @ 12" OC VERT, #5 @ 12" OC HORIZ	

COLUMN FOUNDATION SCHEDULE					
MARK	SIZE		THINKNESS	REINFORCEMENT	REMARKS
	LENGTH	WIDTH			
F5.0	5' - 0"	5' - 0"	1' - 4"	(6) #5 BARS E.W. T&B	
F13.0	13' - 0"	13' - 0"	2' - 0"	(14) #7 BARS E.W. T&B	

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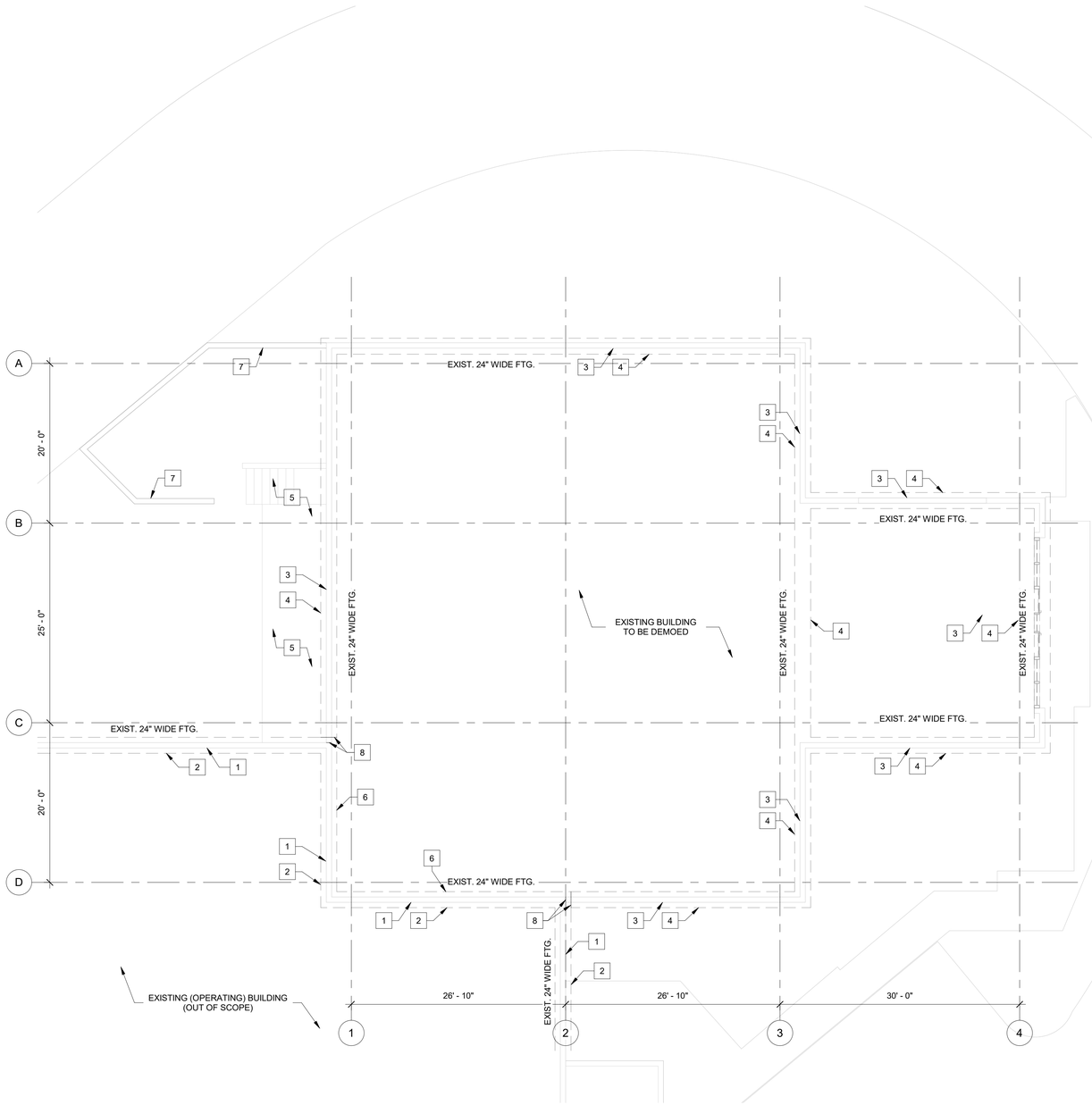
SHEET NO.: S0.6

SCHEDULES



FOUNDATION PLAN DEMO NOTES

1. REFER TO GENERAL STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITION OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.

FOUNDATION DEMO KEY NOTES

1. EXISTING 8" MASONRY WALL TO REMAIN
2. EXISTNG CONCRETE FOUNDATION TO REMAIN
3. EXISTING 8" MASONRY WALL TO BE DEMOLISHED
4. EXISTING CONCRETE FOUNDATION TO BE DEMOLISHED
5. EXISTING CONCRETE LOADING DOCK PLATFORM (INCLUDING SLAB, STAIR, WALL, AND FOUNDATIONS) TO BE DEMOLISHED
6. GC TO PROVIDE PROTECTION/UNDERPINNING OF EXISTING FOUNDATIONS AND WALLS TO ALLOW FOR CONSTRUCTION OF NEW ADJACENT STRUCTURE.
7. EXISTING SCREENWALL TO REMAIN
8. BOUNDARY FOR END OF DEMO

1 FOUNDATION / SLAB DEMO PLAN

1/8" = 1'-0"

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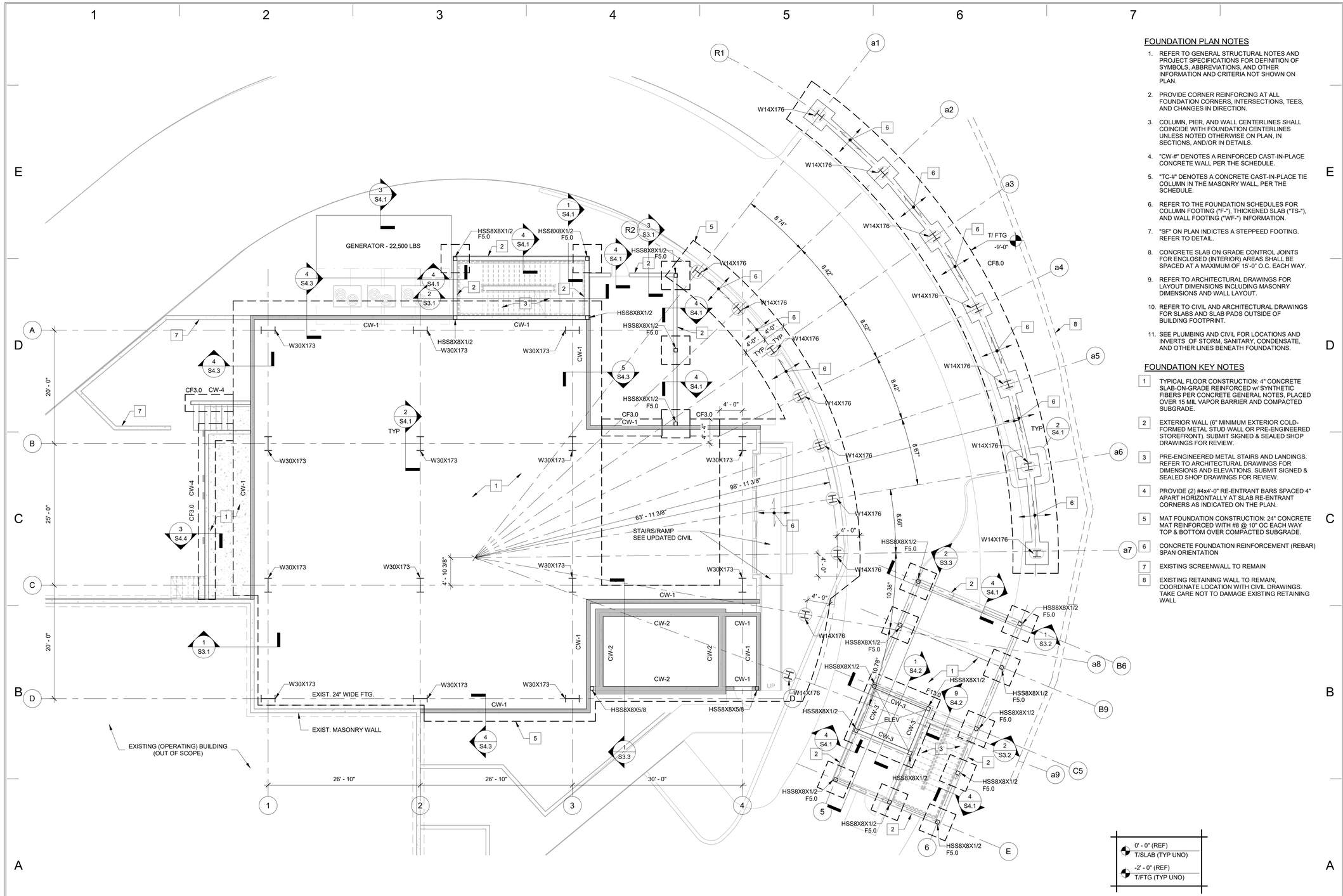
SHEET NO.:	S1.1

FOUNDATION DEMO PLAN



FOUNDATION PLAN NOTES

1. REFER TO GENERAL STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITION OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
2. PROVIDE CORNER REINFORCING AT ALL FOUNDATION CORNERS, INTERSECTIONS, TEES, AND CHANGES IN DIRECTION.
3. COLUMN, PIER, AND WALL CENTERLINES SHALL COINCIDE WITH FOUNDATION CENTERLINES UNLESS NOTED OTHERWISE ON PLAN, IN SECTIONS, AND/OR IN DETAILS.
4. "CW-#" DENOTES A REINFORCED CAST-IN-PLACE CONCRETE WALL PER THE SCHEDULE.
5. "TC-#" DENOTES A CONCRETE CAST-IN-PLACE TIE COLUMN IN THE MASONRY WALL, PER THE SCHEDULE.
6. REFER TO THE FOUNDATION SCHEDULES FOR COLUMN FOOTING ("F-"), THICKENED SLAB ("TS-"), AND WALL FOOTING ("WF-") INFORMATION.
7. "SF" ON PLAN INDICTES A STEPPEED FOOTING. REFER TO DETAIL.
8. CONCRETE SLAB ON GRADE CONTROL JOINTS FOR ENCLOSED (INTERIOR) AREAS SHALL BE SPACED AT A MAXIMUM OF 15'-0" O.C. EACH WAY.
9. REFER TO ARCHITECTURAL DRAWINGS FOR LAYOUT DIMENSIONS INCLUDING MASONRY DIMENSIONS AND WALL LAYOUT.
10. REFER TO CIVIL AND ARCHITECTURAL DRAWINGS FOR SLABS AND SLAB PADS OUTSIDE OF BUILDING FOOTPRINT.
11. SEE PLUMBING AND CIVIL FOR LOCATIONS AND INVERTS OF STORM, SANITARY, CONDENSATE, AND OTHER LINES BENEATH FOUNDATIONS.

FOUNDATION KEY NOTES

1. TYPICAL FLOOR CONSTRUCTION: 4" CONCRETE SLAB-ON-GRADE REINFORCED w/ SYNTHETIC FIBERS PER CONCRETE GENERAL NOTES, PLACED OVER 15 MIL VAPOR BARRIER AND COMPACTED SUBGRADE.
2. EXTERIOR WALL (6" MINIMUM EXTERIOR COLD-FORMED METAL STUD WALL OR PRE-ENGINEERED STOREFRONT). SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
3. PRE-ENGINEERED METAL STAIRS AND LANDINGS. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS. SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
4. PROVIDE (2) #4x4'-0" RE-ENTRANT BARS SPACED 4" APART HORIZONTALLY AT SLAB RE-ENTRANT CORNERS AS INDICATED ON THE PLAN.
5. MAT FOUNDATION CONSTRUCTION: 24" CONCRETE MAT REINFORCED WITH #8 @ 10" OC EACH WAY TOP & BOTTOM OVER COMPACTED SUBGRADE.
6. CONCRETE FOUNDATION REINFORCEMENT (REBAR) SPAN ORIENTATION
7. EXISTING SCREENWALL TO REMAIN
8. EXISTING RETAINING WALL TO REMAIN, COORDINATE LOCATION WITH CIVIL DRAWINGS. TAKE CARE NOT TO DAMAGE EXISTING RETAINING WALL

1 S2.1 FOUNDATION / SLAB PLAN

1/8" = 1'-0"

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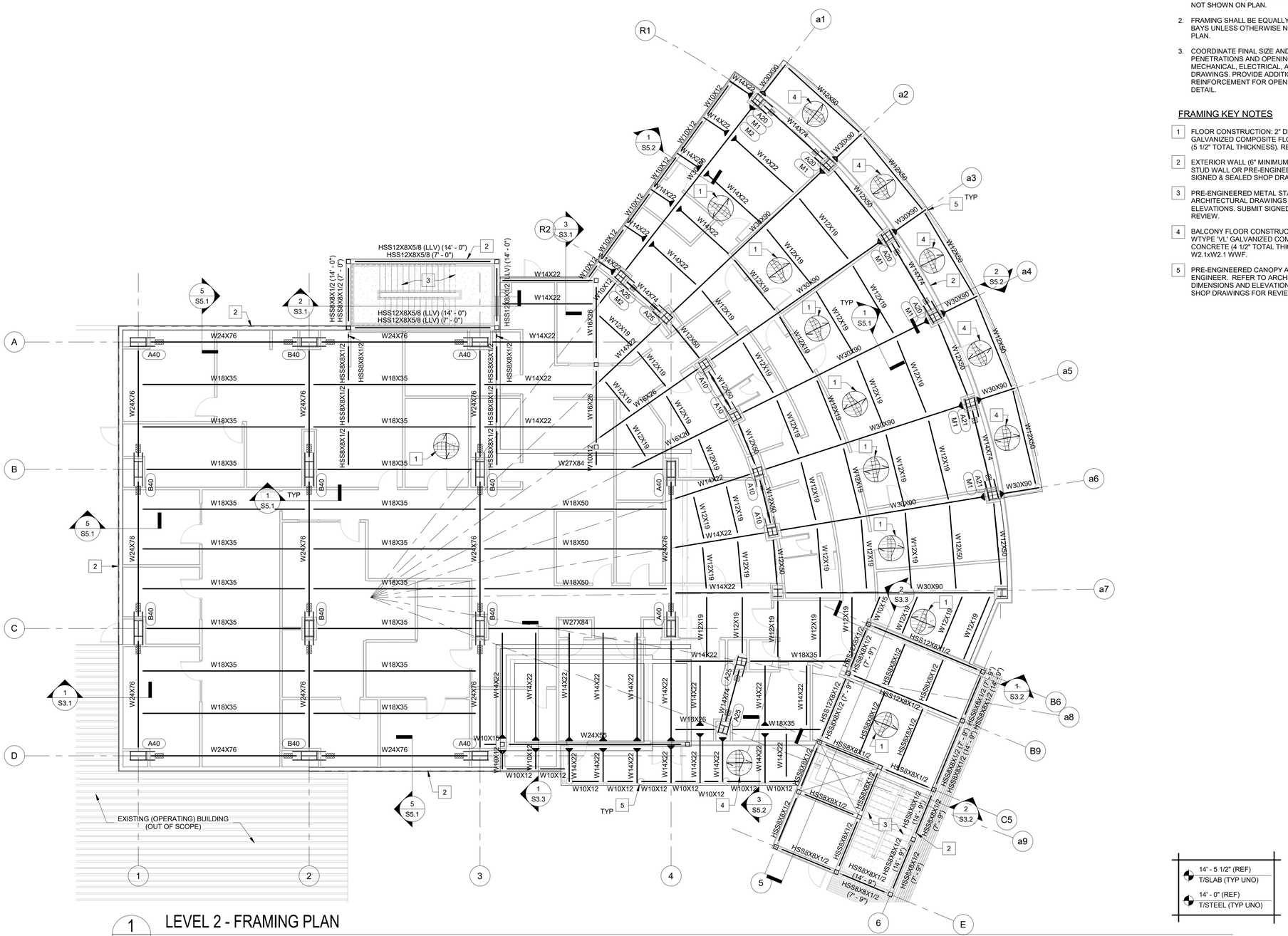
FOUNDATION - SLAB PLAN



1 S2.2 LEVEL 2 - FRAMING PLAN

1/8" = 1'-0"

14' - 5 1/2" (REF)
T/SLAB (TYP UNO)

14' - 0" (REF)
T/STEEL (TYP UNO)

FRAMING PLAN NOTES

1. REFER TO GENERAL STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITION OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
2. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS OTHERWISE NOTED OR DIMENSIONED ON PLAN.
3. COORDINATE FINAL SIZE AND LOCATION OF FLOOR PENETRATIONS AND OPENINGS WITH THE APPROPRIATE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. PROVIDE ADDITIONAL FRAMING / STRUCTURAL REINFORCEMENT FOR OPENINGS IN FLOOR DECK PER DETAIL.

FRAMING KEY NOTES

1. FLOOR CONSTRUCTION: 2" DEEP, 20 GA, VULCRAFT TYPE 'VL' GALVANIZED COMPOSITE FLOOR DECK WITH 3 1/2" CONCRETE (5 1/2" TOTAL THICKNESS). REINFORCE w/ 6x6-W2.1xW2.1 WWF.
2. EXTERIOR WALL (6" MINIMUM EXTERIOR COLD-FORMED METAL STUD WALL OR PRE-ENGINEERED STOREFRONT). SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
3. PRE-ENGINEERED METAL STAIRS AND LANDINGS. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS. SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
4. BALCONY FLOOR CONSTRUCTION: 2" DEEP, 20 GA, VULCRAFT WTYPE 'VL' GALVANIZED COMPOSITE FLOOR DECK WITH 2 1/2" CONCRETE (4 1/2" TOTAL THICKNESS). REINFORCE w/ 6x6-W2.1xW2.1 WWF.
5. PRE-ENGINEERED CANOPY AND COLUMNS BY DELEGATED ENGINEER. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS. SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.

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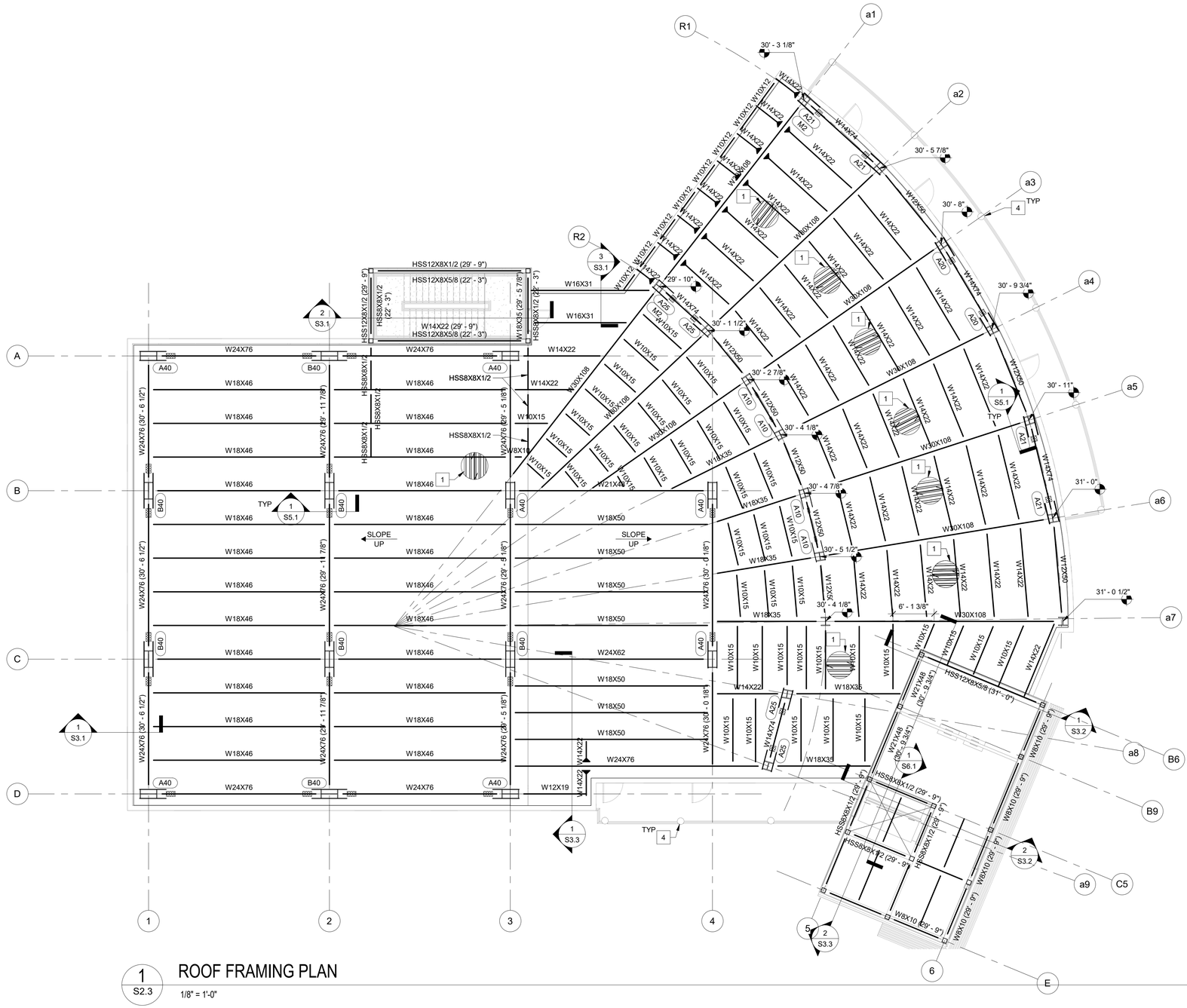
SHEET NO.:	S2.2

LEVEL 2 - FRAMING PLAN



1 S2.3 ROOF FRAMING PLAN

1/8" = 1'-0"

FRAMING PLAN NOTES

1. REFER TO GENERAL STRUCTURAL NOTES AND PROJECT SPECIFICATIONS FOR DEFINITION OF SYMBOLS, ABBREVIATIONS, AND OTHER INFORMATION AND CRITERIA NOT SHOWN ON PLAN.
2. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS OTHERWISE NOTED OR DIMENSIONED ON PLAN.
3. COORDINATE FINAL SIZE AND LOCATION OF ROOF PENETRATIONS AND OPENINGS WITH THE APPROPRIATE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. PROVIDE ADDITIONAL FRAMING / STRUCTURAL REINFORCEMENT FOR OPENINGS IN METAL ROOF DECK PER DETAIL.
4. MECHANICAL EQUIPMENT WEIGHT SHOWN ON PLANS IS PRESUMED OPERATING WEIGHT. NOTIFY ENGINEER IMMEDIATELY IF WEIGHT OF FINAL EQUIPMENT SELECTIONS EXCEED THOSE INDICATED ON PLAN.

FRAMING KEY NOTES

1. TYPICAL ROOF CONSTRUCTION: 1 1/2" 20 GAGE WIDE-RIB (TYPE B) GALVANIZED STEEL ROOF DECK. REFER TO PLAN FOR INTENDED DECK SPAN DIRECTIONS. REFER TO DETAIL X/SX.X FOR DECK ATTACHMENT.
2. EXTERIOR WALL (6" MINIMUM EXTERIOR COLD-FORMED METAL STUD WALL OR PRE-ENGINEERED STOREFRONT). SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
3. PRE-ENGINEERED METAL STAIRS AND LANDINGS. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS. SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.
4. PRE-ENGINEERED CANOPY AND COLUMNS BY DELEGATED ENGINEER. REFER TO ARCHITECTURAL DRAWINGS FOR DIMENSIONS AND ELEVATIONS. SUBMIT SIGNED AND SEALED SHOP DRAWINGS FOR REVIEW.

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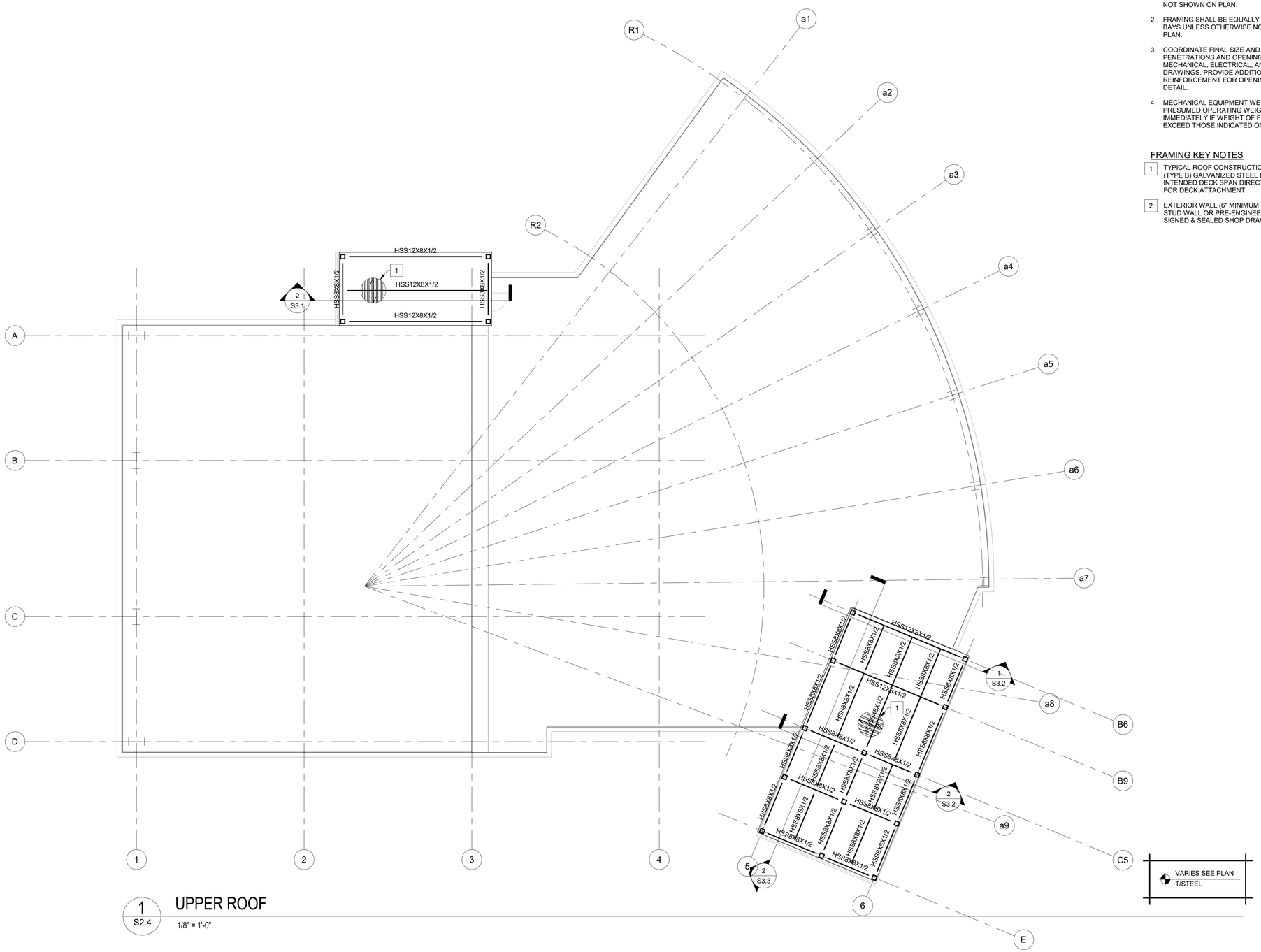
ROOF FRAMING PLAN



1 UPPER ROOF
S2.4
1/8" = 1'-0"

FRAMING PLAN NOTES

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2. FRAMING SHALL BE EQUALLY SPACED BETWEEN COLUMN BAYS UNLESS OTHERWISE NOTED OR DIMENSIONED ON PLAN.
3. COORDINATE FINAL SIZE AND LOCATION OF ROOF PENETRATIONS AND OPENINGS WITH THE APPROPRIATE MECHANICAL, ELECTRICAL, AND ARCHITECTURAL DRAWINGS. PROVIDE ADDITIONAL FRAMING / STRUCTURAL REINFORCEMENT FOR OPENINGS IN METAL ROOF DECK PER DETAIL.
4. MECHANICAL EQUIPMENT WEIGHT SHOWN ON PLANS IS PRESUMED OPERATING WEIGHT. NOTIFY ENGINEER IMMEDIATELY IF WEIGHT OF FINAL EQUIPMENT SELECTIONS EXCEED THOSE INDICATED ON PLAN.

FRAMING KEY NOTES

1 TYPICAL ROOF CONSTRUCTION: 1 1/2" 20 GAGE WIDE-RIB (TYPE B) GALVANIZED STEEL ROOF DECK. REFER TO PLAN FOR INTENDED DECK SPAN DIRECTIONS. REFER TO DETAIL X/SX.X FOR DECK ATTACHMENT.

2 EXTERIOR WALL (6" MINIMUM EXTERIOR COLD-FORMED METAL STUD WALL OR PRE-ENGINEERED STOREFRONT). SUBMIT SIGNED & SEALED SHOP DRAWINGS FOR REVIEW.

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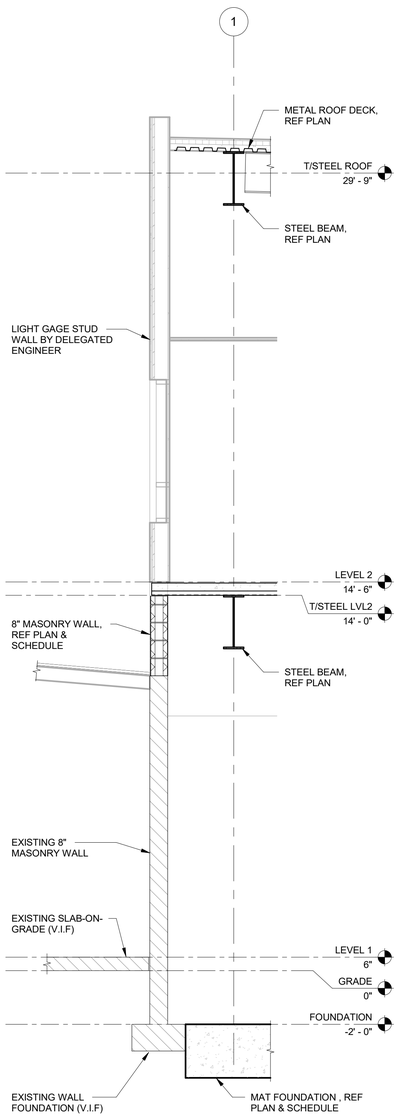
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

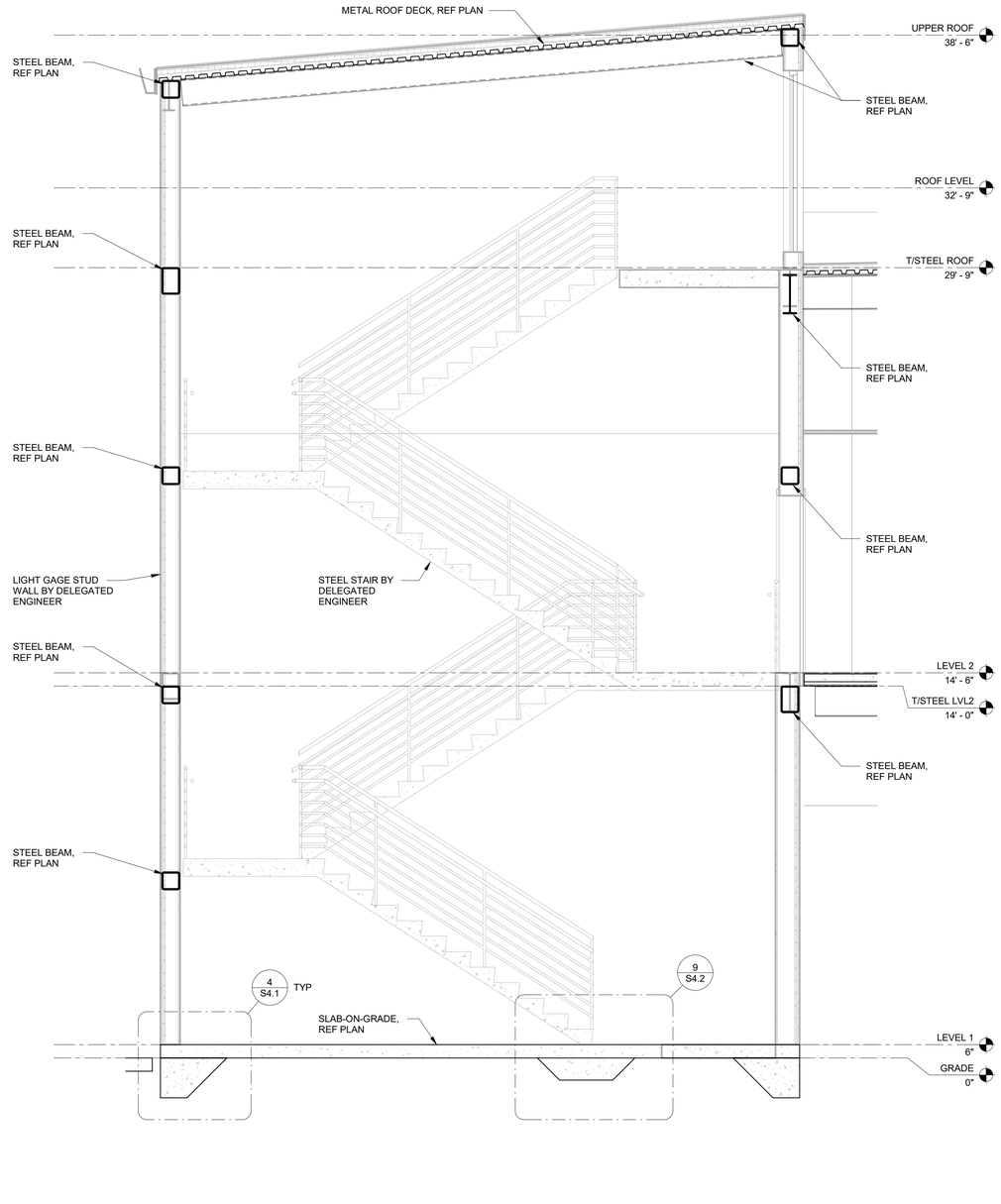
DATE:	02.14.2025
DRAWN BY:	SH
CHECKED BY:	TL
APPROVED BY:	JRM
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	S2.4

HIGH ROOF FRAMING PLAN



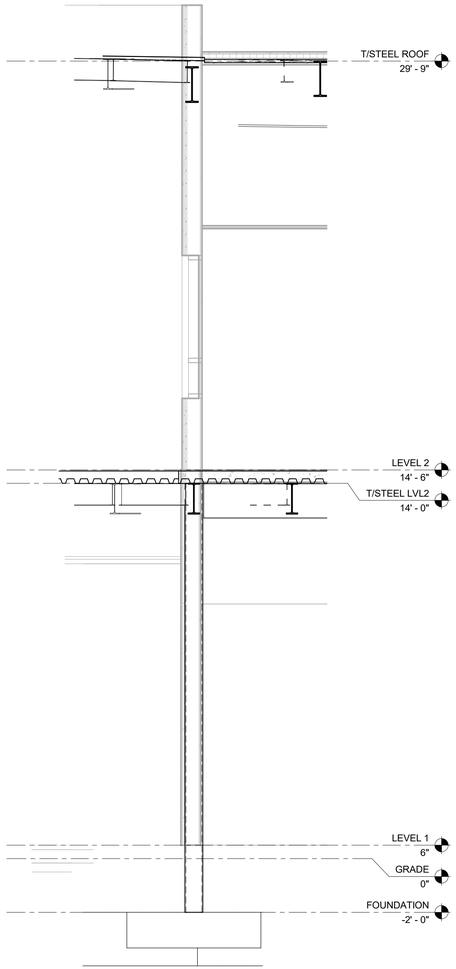
1 S3.1 SECTION

3/8" = 1'-0"



2 S3.1 SECTION

3/8" = 1'-0"



3 S3.1 SECTION

3/8" = 1'-0"

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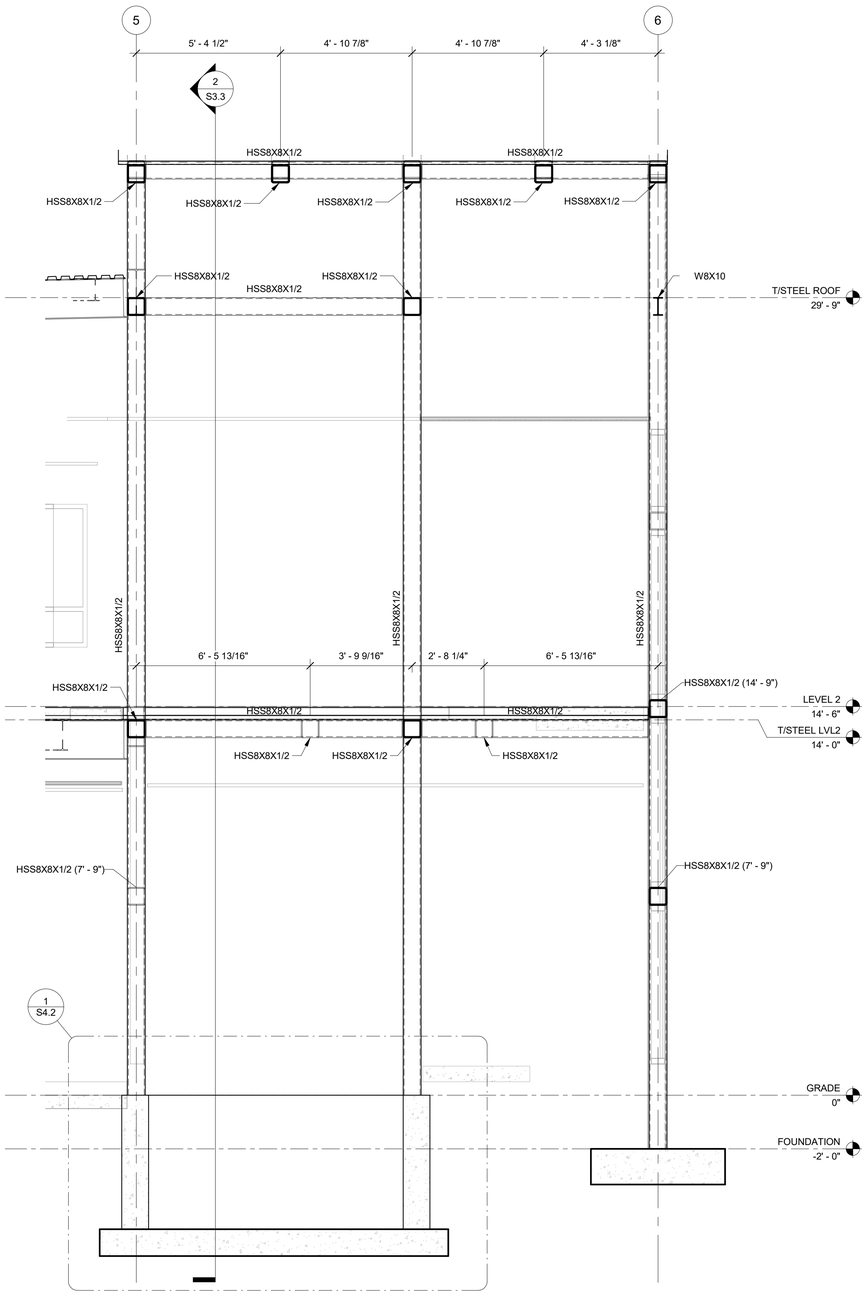
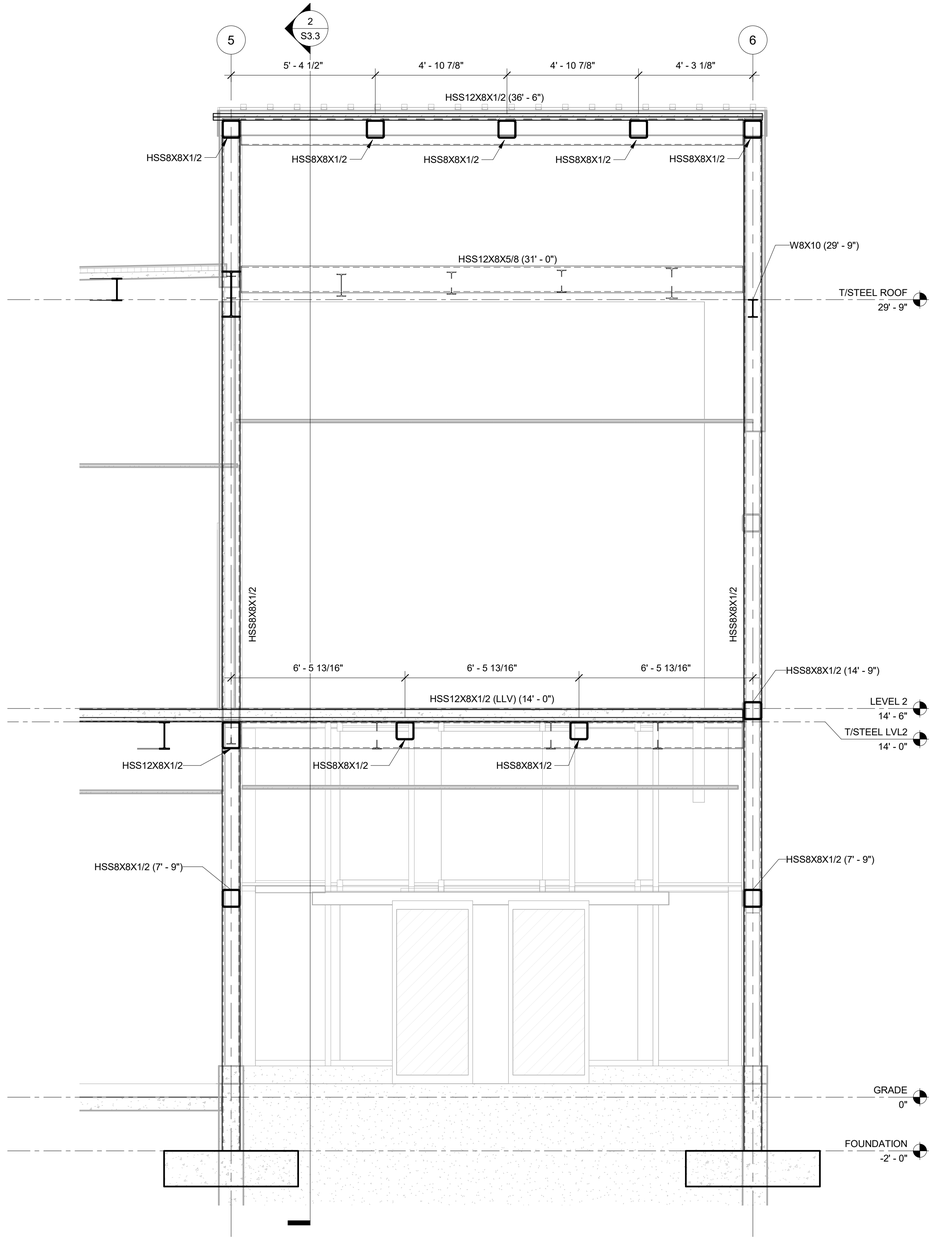
BUILDING SECTIONS



2 SECTION
S3.2
3/8" = 1'-0"



1 SECTION
S3.2
3/8" = 1'-0"

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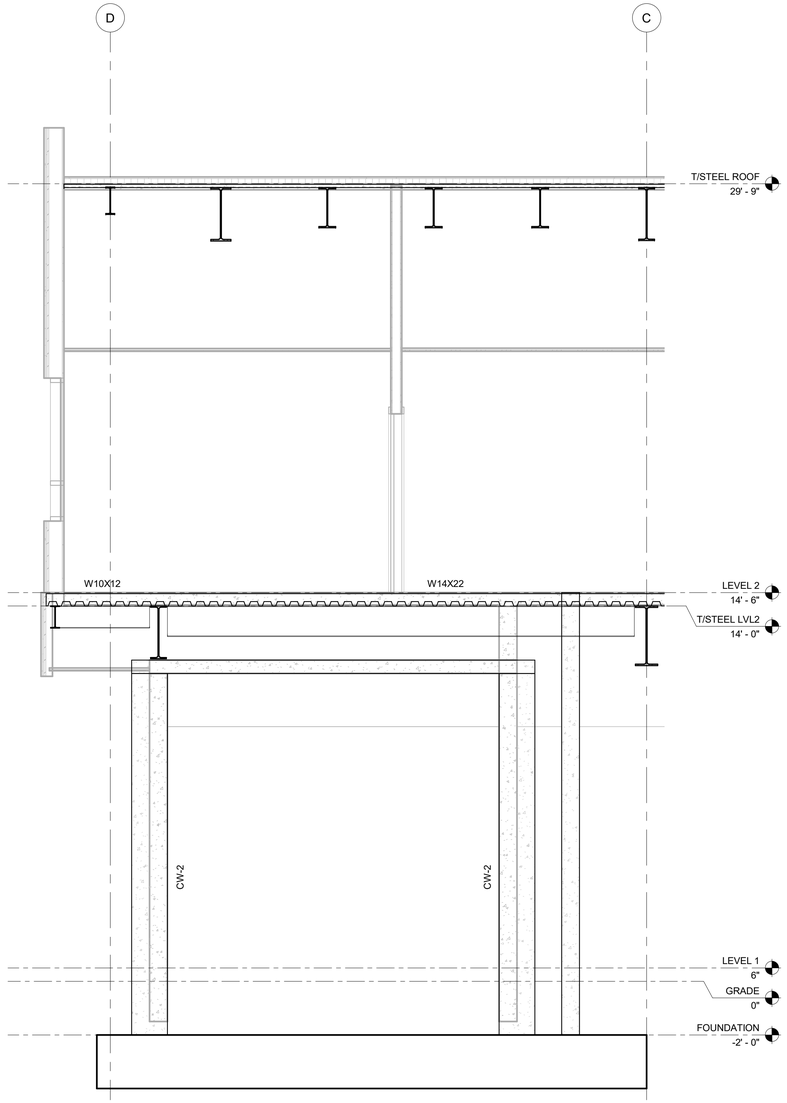
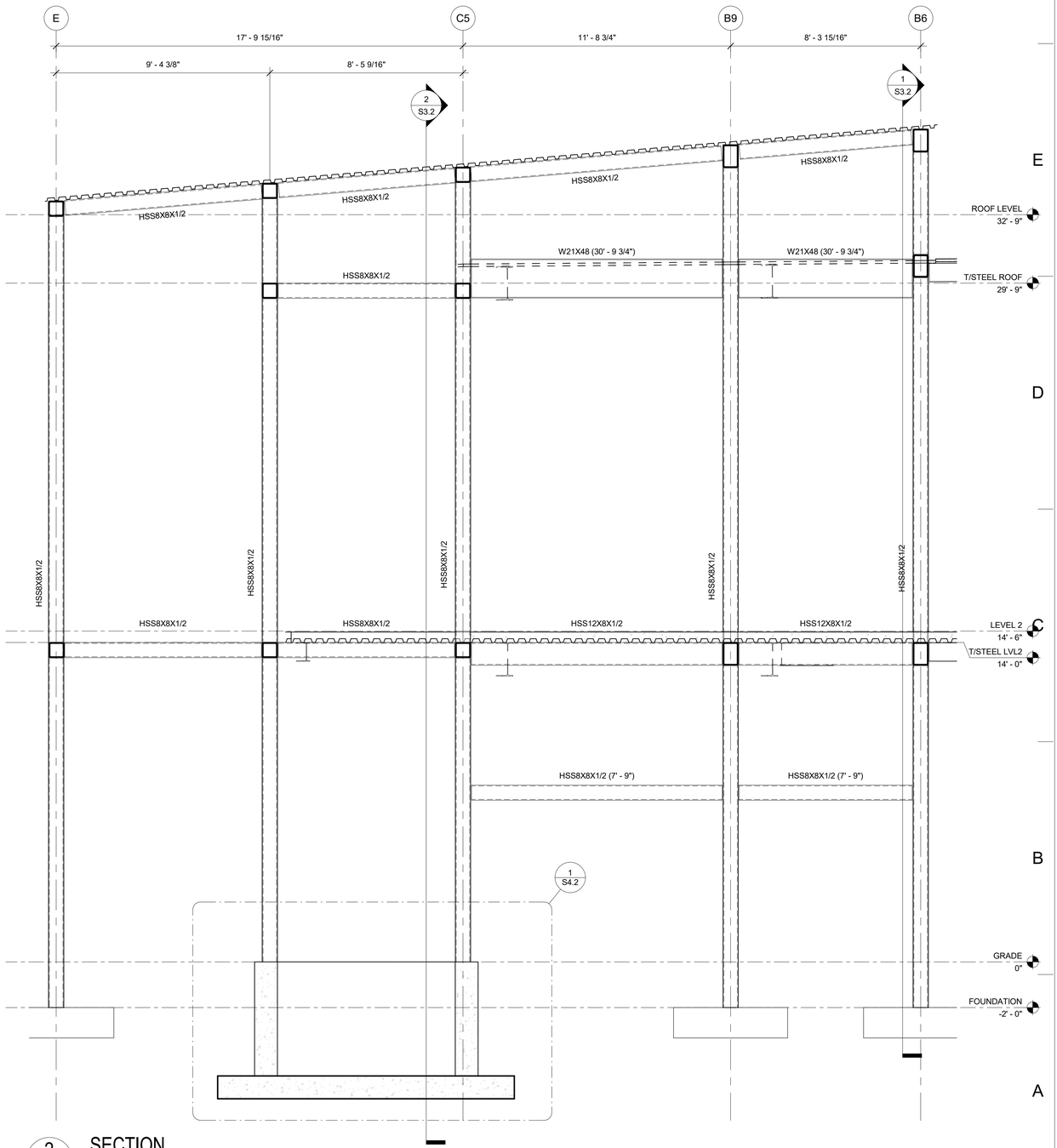
BUILDING SECTIONS



1 S3.3 SECTION

3/8" = 1'-0"



2 S3.3 SECTION

3/8" = 1'-0"

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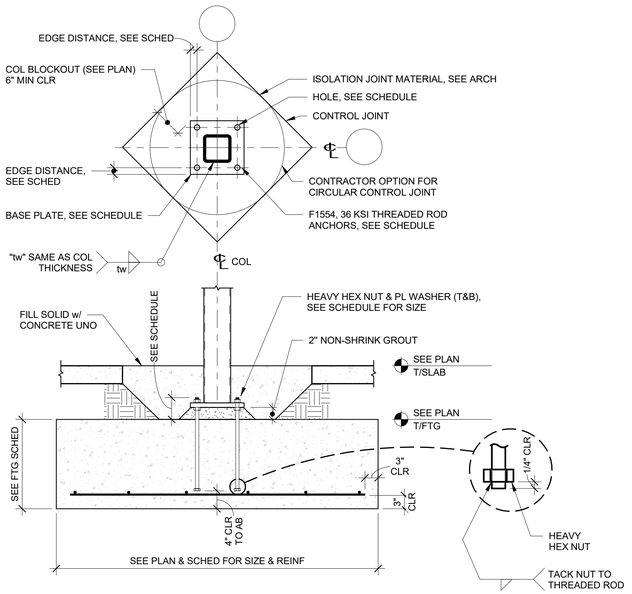
SHEET NO.: S3.3

BUILDING SECTIONS

BASE PLATE SCHEDULE

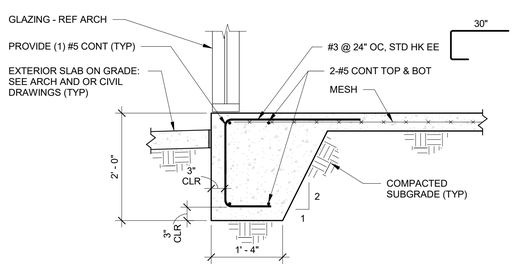
COLUMN	BASE PLATE	ANCHORS	ANCHOR LENGTH	HOLE DIAMETER	MIN WASHER PL SIZE	ANCHOR ROD EDGE DISTANCE
W14	PL 3"x48"x48"	(6) 2"Ø	SEE DETAIL	3 1/4"	3/4" x 5"	3"
W30	PL 4"x54"x54"	(6) 2"Ø	SEE DETAIL	3 1/4"	3/4" x 5"	3"
HSS-8x8	PL 2"x20"x20"	(4) 1"Ø	SEE DETAIL	1 7/8"	3/8" x 3"	2"

NOTES:
1. ANCHOR ROD PROJECTION: MINIMUM LENGTH AS REQUIRED TO ACCOUNT FOR NON-SHRINK GROUT, BASE PLATE THICKNESS, WASHERS/NUT, PLUS 1 1/2" MIN (THREADED LENGTH ABOVE NUT).
2. PLATE WASHER SHALL HAVE STANDARD HOLE, UNO.
3. ALL ANCHOR ROD NUTS SHALL BE "SNUG TIGHTENED" AS DEFINED BY AISC AFTER THE FOOTING CONCRETE HAS A MINIMUM OF 14 (FOURTEEN) DAYS TO CURE. (TYP UNLESS NOTED OTHERWISE).



1 S4.1 HSS COLUMN/FTG DETAIL

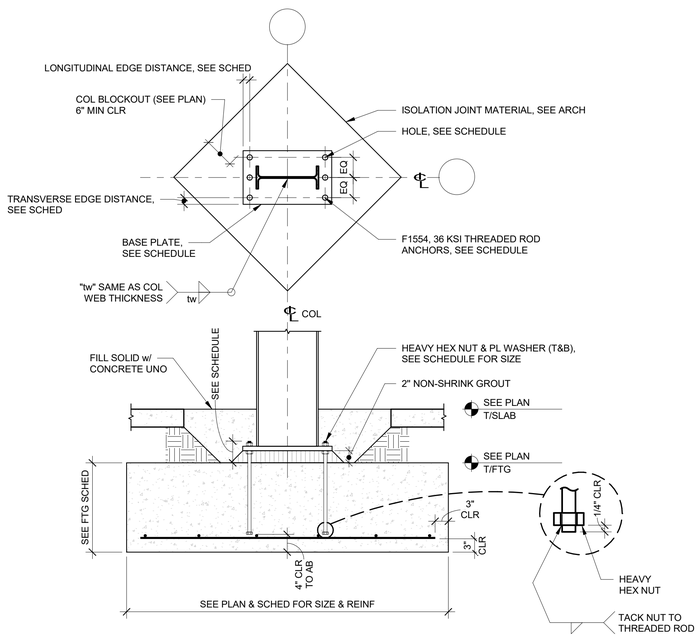
3/4" = 1'-0"



NOTE:
DEPTH OF TURNED-DOWN SLAB MAY BE INCREASED AS NEEDED TO REST ON TOP OF COLUMN OR WALL FOOTINGS TYP

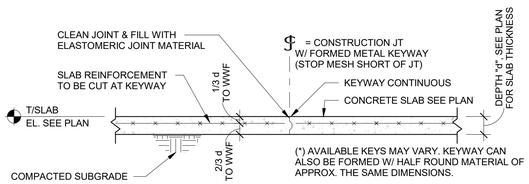
4 S4.1 TYPICAL SLAB EDGE @ GLAZING DETAIL

3/4" = 1'-0"

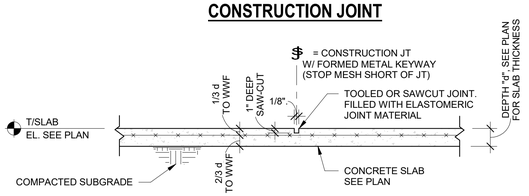


2 S4.1 W-COLUMN/FTG DETAIL

3/4" = 1'-0"



TYPICAL CONSTRUCTION JOINT

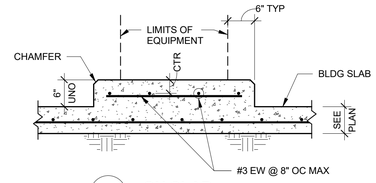


TYPICAL SAWCUT CONTROL JOINT

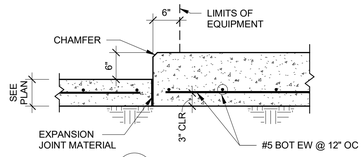
NOTE:
1. CJ - INDICATES CONSTRUCTION JOINT. SJ - INDICATES SAW-CUT CONTROL JOINT.
2. CONTROL JOINTS SHALL BE SAWCUT AS SOON AS POSSIBLE WITHOUT RAVELING CONCRETE (4 TO 16 HOURS AFTER PLACEMENT MAX.)
3. JOINT PLACEMENT REQUIREMENTS:
   FOR ENCLOSED/INTERIOR AREAS. 15'-0" O.C. (MAX.) EACH WAY
   FOR OUTSIDE/EXTERIOR AREAS. 12'-0" O.C. (MAX.) EACH WAY
4. GENERAL CONTRACTOR IS TO COORDINATE JOINT LOCATIONS WITH FLOOR FINISHES.

5 S4.1 TYPICAL SLAB CONSTRUCTION/CONTROL JOINTS

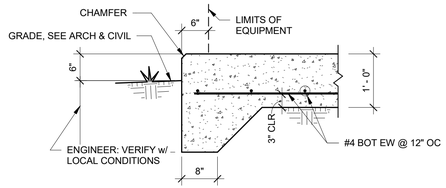
3/4" = 1'-0"



A ON SLAB



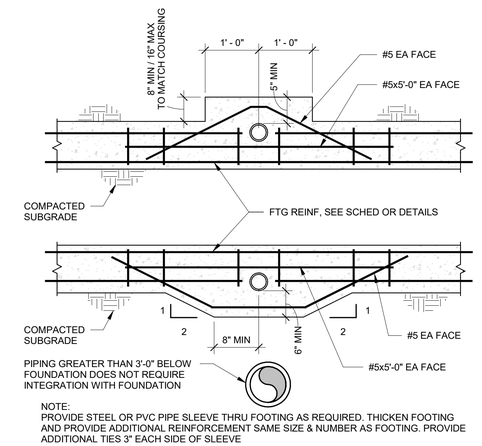
B ISOLATED PAD



C AT GRADE

3 S4.1 TYP EQUIPMENT SUPPORT PADS

3/4" = 1'-0"



NOTE:
PROVIDE STEEL OR PVC PIPE SLEEVE THRU FOOTING AS REQUIRED. THICKEN FOOTING AND PROVIDE ADDITIONAL REINFORCEMENT SAME SIZE & NUMBER AS FOOTING. PROVIDE ADDITIONAL TIES 3" EACH SIDE OF SLEEVE

6 S4.1 SLEEVED FTG DETAIL

3/4" = 1'-0"

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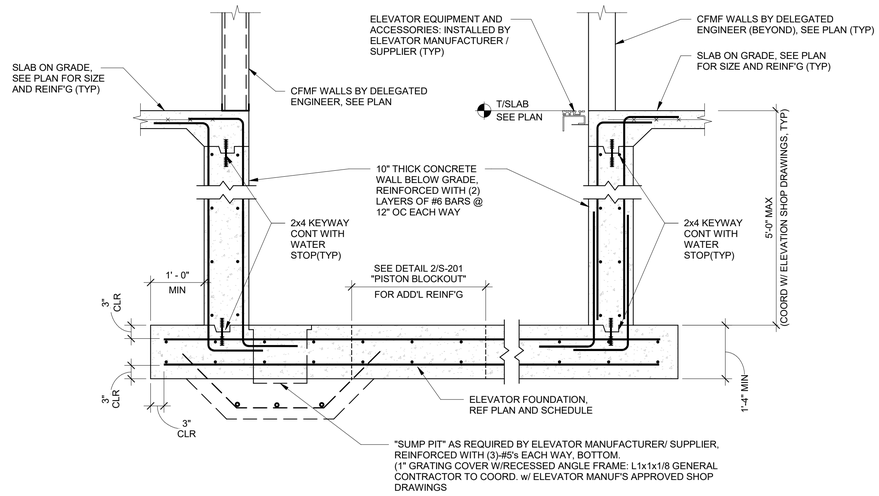
APPROVED BY: JRM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

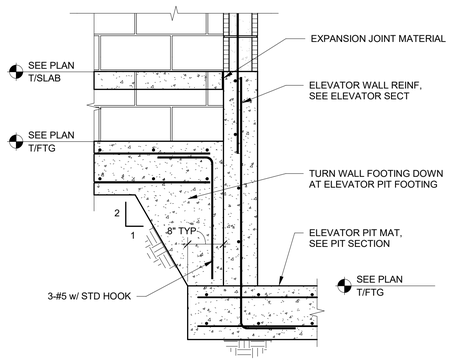
SHEET NO.: S4.1

FOUNDATION DETAILS AND SECTIONS



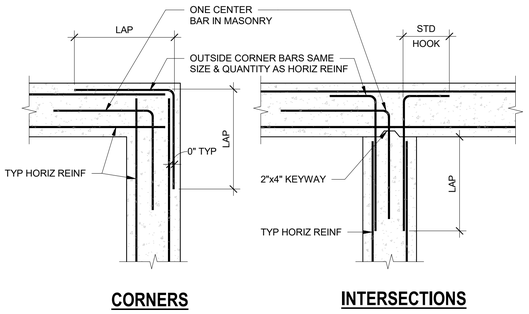
1 S4.2 **TYPICAL ELEVATOR PIT DETAIL**

3/4" = 1'-0"



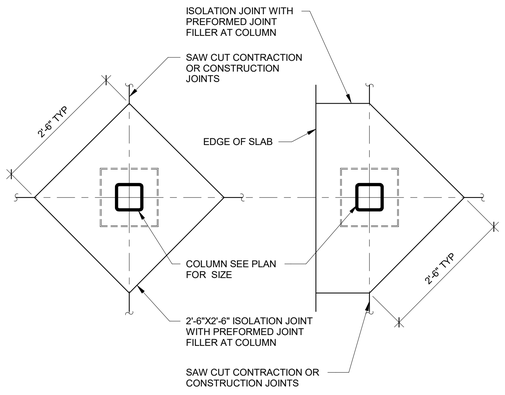
2 S4.2 **TURNED DOWN FTG AT ELEVATOR**

3/4" = 1'-0"



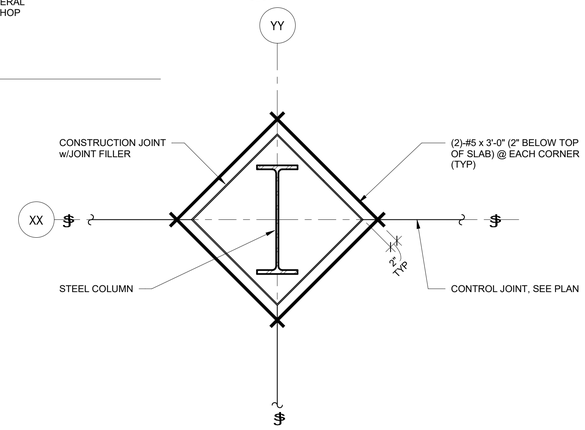
3 S4.2 **TYP HORIZ REINF IN BEAMS, WALLS AND FTGS**

3/4" = 1'-0"



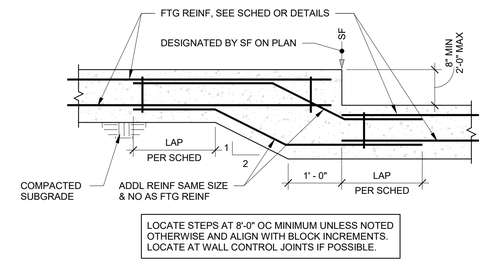
4 S4.2 **TYP. S.O.G. SECTION JOINT DETAILS @ COLUMNS**

3/4" = 1'-0"



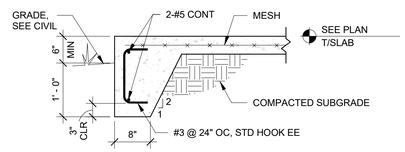
5 S4.2 **TYPICAL ADDITIONAL SLAB REINFORCING AT COLUMNS**

3/4" = 1'-0"



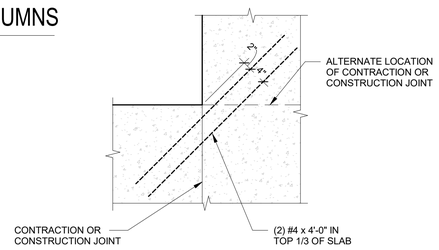
6 S4.2 **TYP STEPPED FOOTING DETAIL**

3/4" = 1'-0"



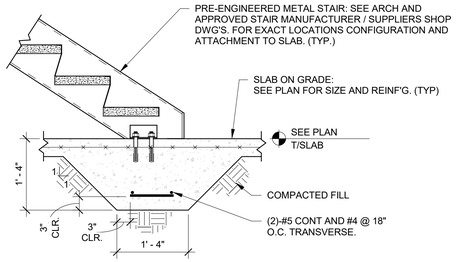
7 S4.2 **TYP SLAB EDGE DETAIL**

3/4" = 1'-0"



8 S4.2 **TYP SLAB ON GRADE JOINT AT RE-ENTANT CORNER**

3/4" = 1'-0"



SEE ARCH'L. DRAWINGS FOR RISE, RUN, NOSING DIMENSIONS AND HANDRAIL DETAILS

9 S4.2 **TYPICAL PRE-ENGINEERED STAIR BASE**

3/4" = 1'-0"

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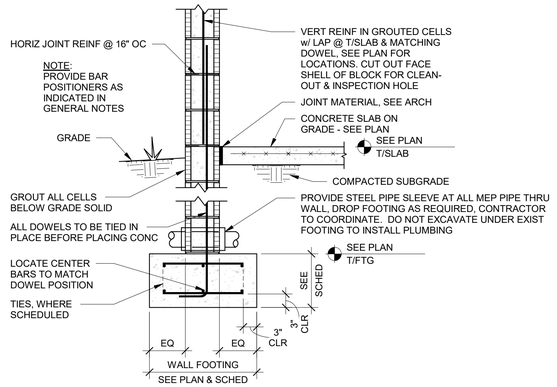
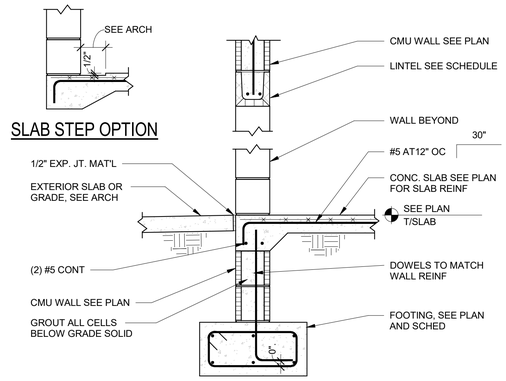


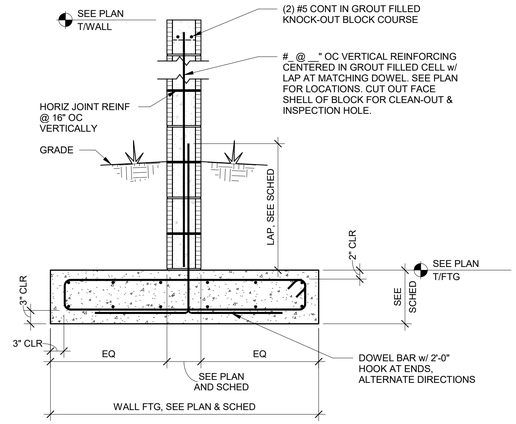
1 S4.3 EXT BLOCK WALL AT FTG

3/4" = 1'-0"



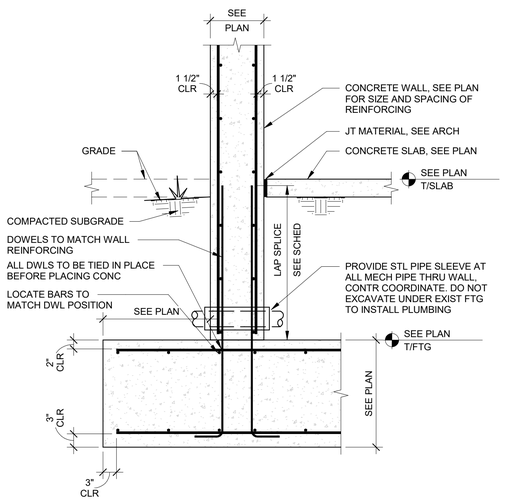
2 S4.3 TYP SECTION @ CMU STEM @ EXT OPENING

3/4" = 1'-0"



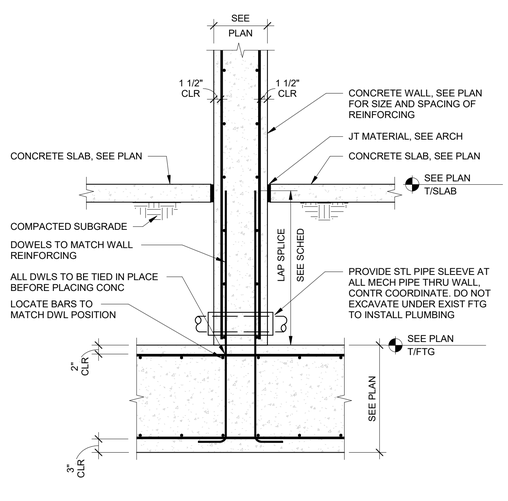
3 S4.3 SCREEN WALL SECTION

3/4" = 1'-0"



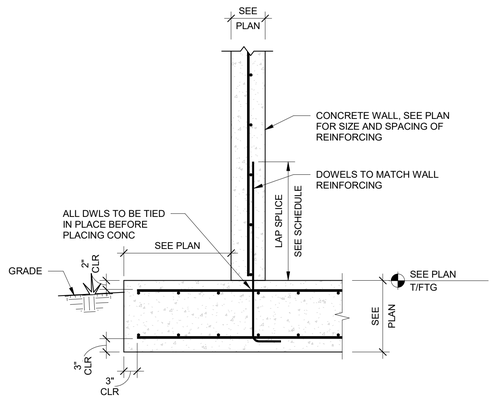
4 S4.3 EXT CONCRETE WALL AT FTG

3/4" = 1'-0"



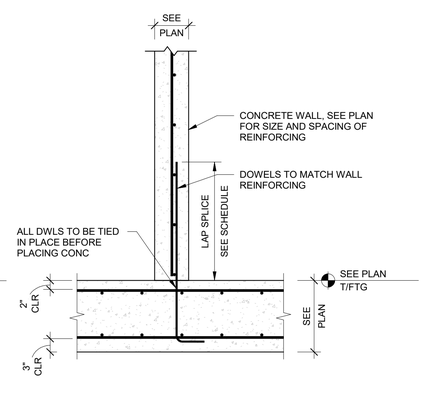
5 S4.3 INT CONCRETE WALL AT FTG

3/4" = 1'-0"



6 S4.3 EXT CONCRETE WALL AT FTG

3/4" = 1'-0"



7 S4.3 INT CONCRETE WALL AT FTG

3/4" = 1'-0"

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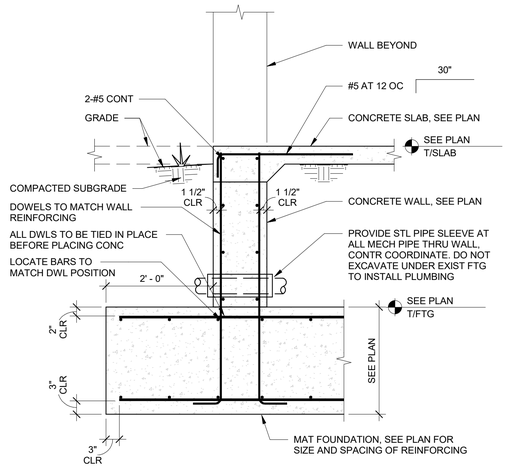
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: SH
CHECKED BY: TL
APPROVED BY: JRM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S4.3

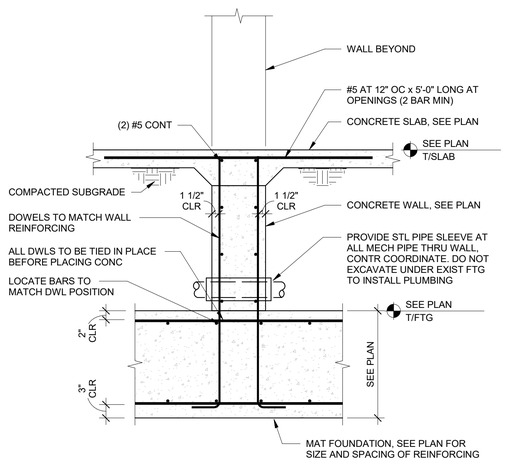
FOUNDATION DETAILS AND SECTIONS



1 S4.4

SECTION AT EXT OPENING

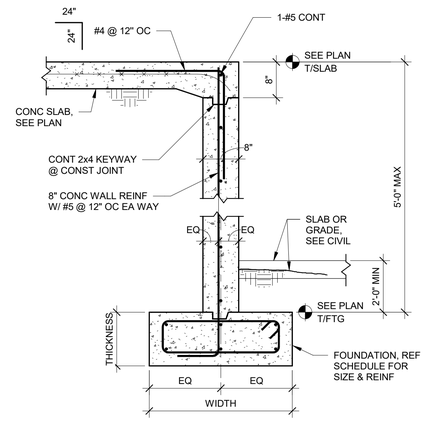
3/4" = 1'-0"



2 S4.4

SECTION AT INT OPENING

3/4" = 1'-0"



3 S4.4

ELEVATED EXTERIOR SLAB

3/4" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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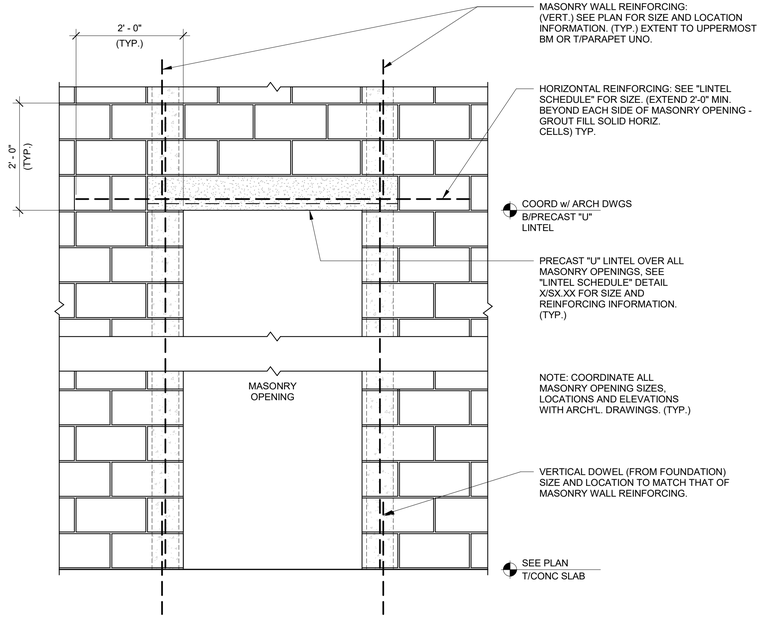
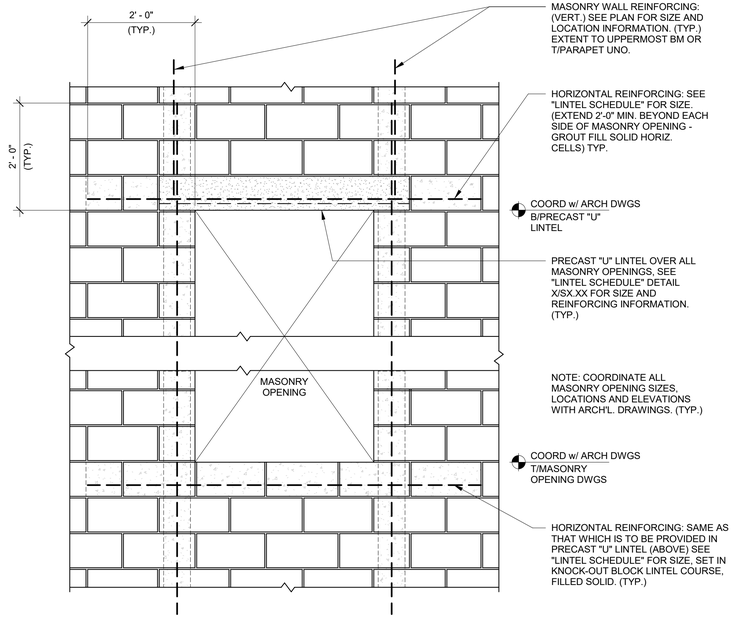
SHEET NO.: S4.4

FOUNDATION DETAILS AND SECTIONS



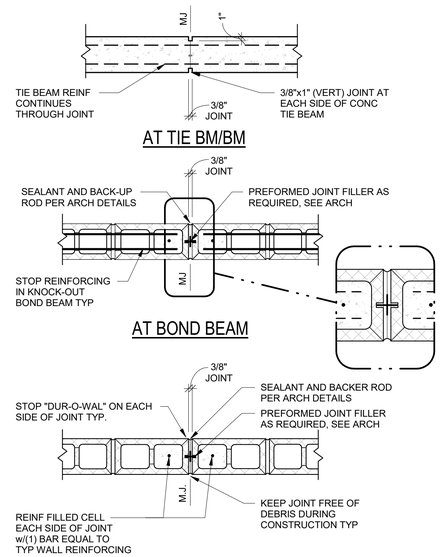
1 S4.5 TYPICAL REINFORCING AT MASONRY (DOOR) OPENINGS

3/4" = 1'-0"



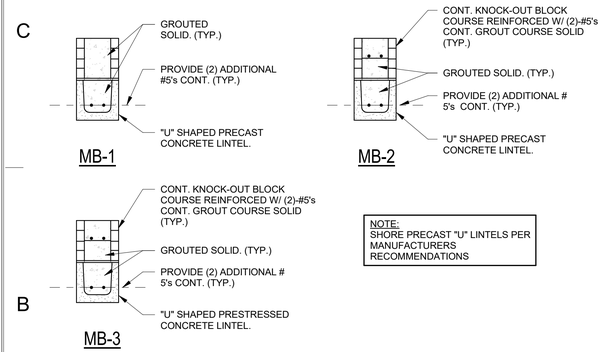
2 S4.5 TYPICAL REINFORCING AT MASONRY OPENINGS

3/4" = 1'-0"



3 S4.5 TYPICAL MASONRY JOINT DETAILS

3/4" = 1'-0"



NOTE:
SHORE PRECAST "U" LINTELS PER MANUFACTURERS RECOMMENDATIONS

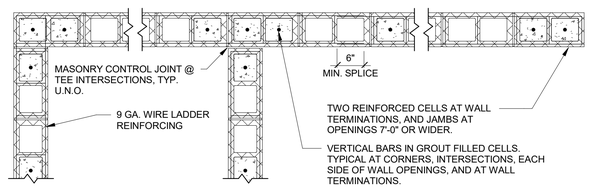
MASONRY LINTEL SCHEDULE (CAST-CRETE LINTEL)

MARK	LENGTH (L)	CAST-CRETE MARK	REMARK
MB-1	2' -10"< L ≤ 5"-10"	8F16-2B/OT PRECAST	
MB-2	5' -10"< L ≤ 14'-0"	8F16-2B/2T PRECAST	
MB-3	14' -0"< L ≤ 16'-0"	8F16-2B/2T PRESTRESSED	

1. PROVIDE MASONRY LINTEL OVER ALL OPENINGS. IF NO LINTEL IS SPECIFIED, PROVIDE MB-2.
2. PROVIDE MINIMUM END BEARING OF 8". CUT OUT BOTTOM OF LINTEL AT END TO ALLOW CONTINUATION OF FILLED CELL REINFORCING.
3. MASONRY LINTEL SUBSTITUTIONS MUST PROVIDE EQUIVALENT OR BETTER CAPACITIES AND MUST BE APPROVED BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO INSTALLATION.

4 S4.5 MASONRY LINTEL SCHEDULE

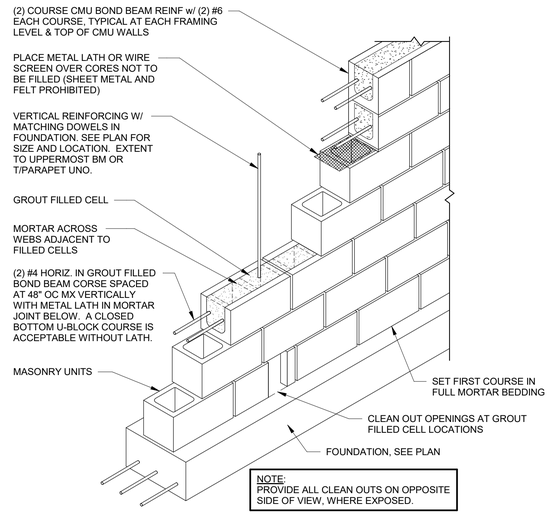
3/4" = 1'-0"



NOTES:
1. TYPICAL WIRE LADDER REINFORCEMENT FOR ALL C.M.U. WALLS SPACED @ 16" O.C. VERTICALLY. PROVIDE ADDITIONAL LAYERS ABOVE AND BELOW WINDOW AND DOOR OPENINGS.
2. SEE PLAN FOR SIZE AND SPACING OF VERT. REINF. BARS.
3. USE PREFABRICATED L-SHAPED WIRE SEGMENTS AT WALL INTERSECTIONS.
4. TERMINATE HORZ. WIRE REINFORCING AT CONTROL JOINTS.

5 S4.5 TYPICAL HORIZ. WALL REINF'G / VERT. CORNER REIN'G

3/4" = 1'-0"



NOTE:
PROVIDE ALL CLEAN OUTS ON OPPOSITE SIDE OF VIEW, WHERE EXPOSED.

6 S4.5 TYPICAL MASONRY WALL CONSTRUCTION

3/4" = 1'-0"

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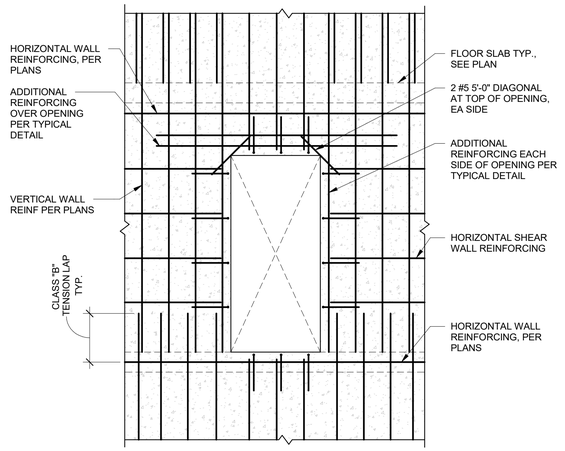
CONTRACT NO.: -

SHEET NO.: S4.5

MASONRY DETAILS AND SECTIONS

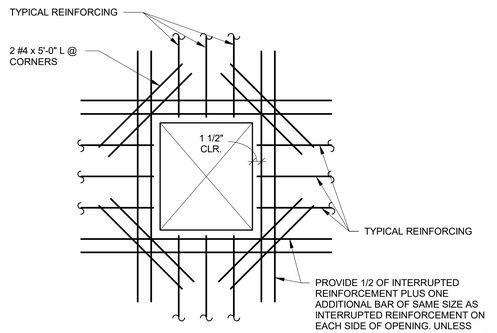


NOTE: REFER TO SHEAR WALL PLANS FOR ADDITIONAL INFO.

1 S4.6

TYPICAL CONCRETE WALL OPENING DETAIL

3/4" = 1'-0"



2 S4.6

ADD'L REINFORCING AT OPENING IN CONCRETE WALL

3/4" = 1'-0"

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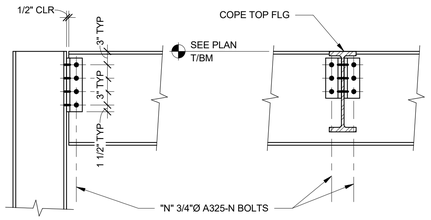
CONCRETE DETAILS AND SECTIONS

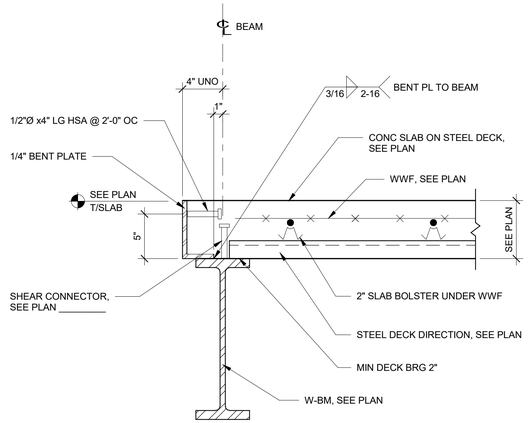


BEAM TO COLUMN **BEAM TO BEAM**

BEAM TO BEAM OR BEAM TO COLUMN SHEAR CONNECTIONS		
BEAM	ANGLE SIZE	"N" BOLTS EA LEG
W10	(2) L-3 1/2"x3 1/2"x5/16"x0'-6"	(2) 3/4"Ø A325-N
W14, W16	(2) L-3 1/2"x3 1/2"x5/16"x0'-9"	(3) 3/4"Ø A325-N
W18	(2) L-3 1/2"x3 1/2"x5/16"x1'-0"	(4) 3/4"Ø A325-N
W21	(2) L-3 1/2"x3 1/2"x5/16"x1'-0"	(4) 3/4"Ø A325-N
W24		
W27		
W30		

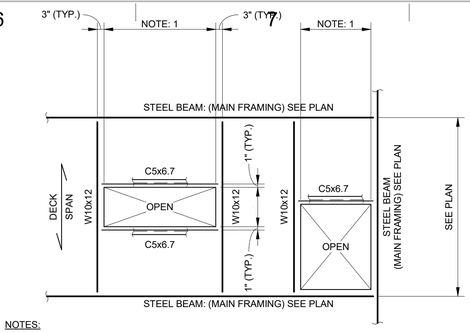
1 S5.1 BEAM TO COL & BEAM TO BEAM CONN

3/4" = 1'-0"



2 S5.1 TYP SLAB EDGE AT STEEL BM

1 1/2" = 1'-0"



NOTES:

1. THESE DETAILS APPLY FOR THRU SLAB OPENINGS WHERE DIMENSION IS UP TO 8'-0" (MAX.) (OPENING EDGE TO OPENING EDGE). DIMENSION OF THRU SLAB OPENINGS IN THE DIRECTION PERPENDICULAR TO METAL DECK SPAN. COORDINATE EXACT SIZE, LOCATION, AND QUANTITY REQUIRED OF THRU SLAB OPENINGS WITH ARCHITECTURAL AND / OR AFFECTED TRADES DRAWINGS.
2. THRU SLAB OPENINGS SHOWN HERE ARE OF VARIOUS SIZES AND LOCATIONS (MAX.SIZE PER NOTE 1). FOR THRU SLAB OPENINGS GREATER THAN 8'-0" SEE PLANS AND ADDITIONAL SECTIONS FOR FRAMING AND INFORMATION. FOR THRU SLAB OPENINGS LESS THAN OR EQUAL TO 8'-0" AND WHERE "BEAM" SIZE AT OPENING IS NOT INDICATED ON PLAN, PROVIDE MIN W10x12
3. METAL DECK MANUFACTURER / SUPPLIER IS TO PROVIDE 14 GAGE (GALV.) METAL CLOSURE EDGE ALONG THRU SLAB OPENING EDGES UNLESS OTHER- WISE NOTED IN SECTIONS / DETAILS.

3 S5.1 TYPICAL ADD'L FRAMING FOR SLAB OPENINGS (UP TO 8'-0")

3/4" = 1'-0"

4 S5.1 TYP EDGE OF SLAB AT INTERIOR

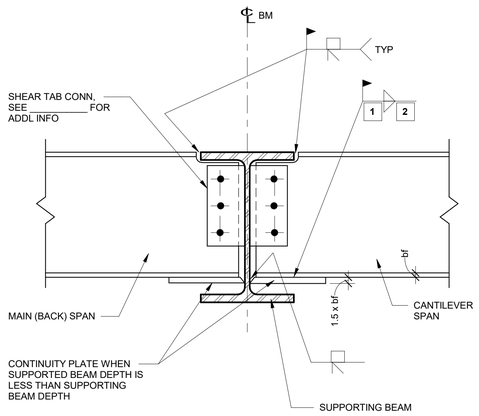
3/4" = 1'-0"

5 S5.1 CFMF BRG AT FLOOR SLAB

3/4" = 1'-0"

6 S5.1 BEAM TO COLUMN MOMENT CONN

3/4" = 1'-0"

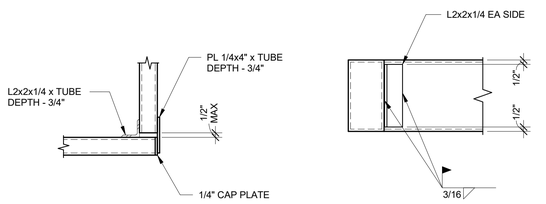


NOTES:

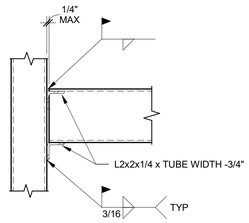
1. FILLET WELD SIZE SHALL BE MAXIMUM SIZE PER AISC SPEC 360-05, J.2.2b.
2. FILLET WELD LENGTH SHALL BE TWICE THE FLANGE WIDTH OF THE CANTILEVERED BEAM.

7 S5.1 MOMENT CONN TO BEAM

1 1/2" = 1'-0"



CORNER CONN **TUBE TO TUBE**



TUBE TO COLUMN

8 S5.1 TYPICAL TUBE TO TUBE CONNECTION

1 1/2" = 1'-0"

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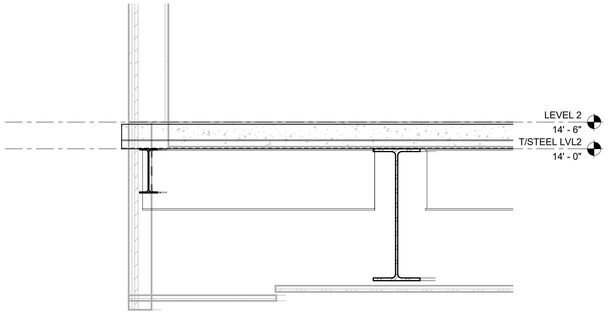
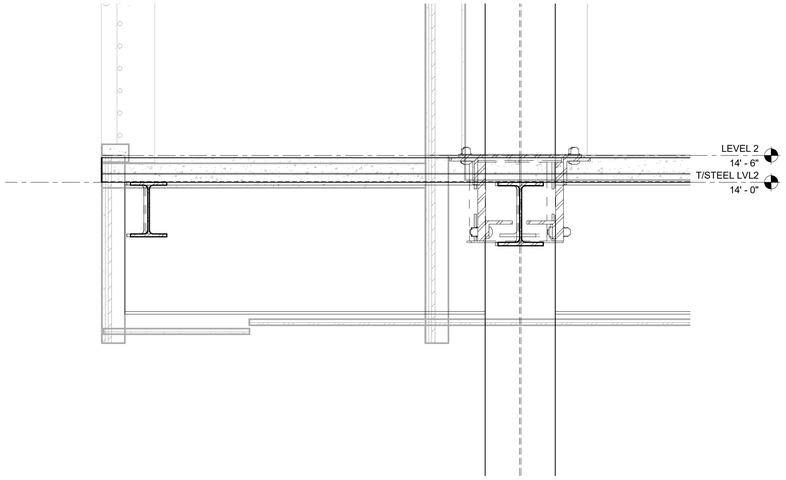
CONTRACT NO.:	-
SHEET NO.:	S5.1

FLOOR FRAMING DETAILS AND SECTIONS



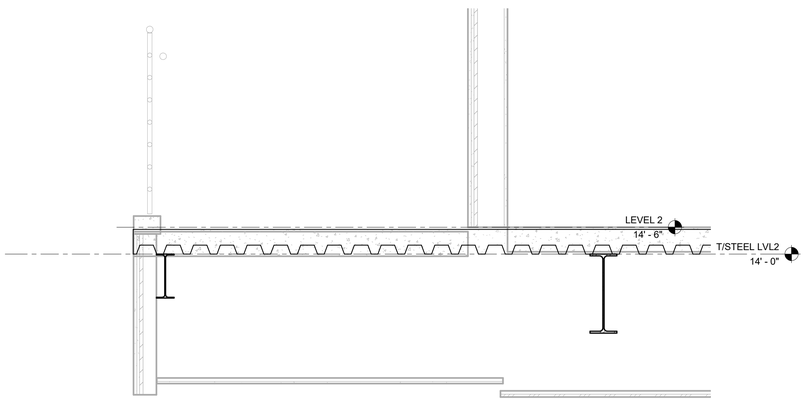
1 S5.2 — 2ND FLOOR OVERHANG SECTION

3/4" = 1'-0"



2 S5.2 — EAST BALCONY SECTION

3/4" = 1'-0"



3 S5.2 — SOUTH BALCONY SECTION

3/4" = 1'-0"

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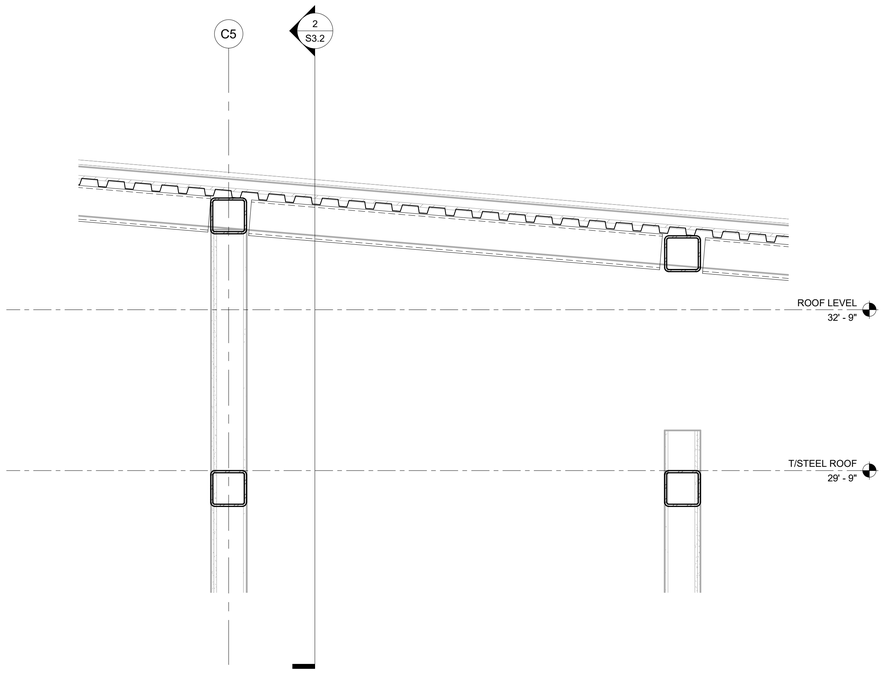
SHEET NO.: S5.2

FLOOR FRAMING DETAILS AND SECTIONS



1 / S6.1 SECTION AT ELEVATOR HOIST BEAM

3/4" = 1'-0"

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PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: S6.1

ROOF FRAMING DETAILS AND SECTIONS

PROJECT SPECIFIC INFORMATION

PROJECT NUMBER:	**23106**
SUBMITTAL NUMBER:	S-01
SUBMITTAL DATE:	02/10/2025
PROJECT NAME:	EDA INDUSTRIAL PARK VERTICAL EXPANSION
LOCATION:	ST. THOMAS, USVI
CONNECTION TYPE:	SIDEPLATE PLUS
NUMBER OF BUILDINGS:	1
APPROX. TOTAL GROSS SQUARE FOOTAGE:	24000
NUMBER OF STORIES:	2

eDATA:

a. THERE MAY BE eDATA AVAILABLE FOR YOUR PROJECT WHICH IS AVAILABLE FOR DOWNLOAD AT WWW.SIDEPLATE.COM. eDATA MAY INCLUDE:
   - eSTIMATE FILE IN EXCEL FORMAT FOR USE IN AFFIRMING SIDEPLATE CONNECTION MATERIAL QUANTITIES.
   - ComponentXML FILE FOR USE IN ASSISTING DETAILING EFFORTS.

b. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT = 56

c. ESTIMATED NUMBER OF SIDEPLATE JOINTS FOR THIS PROJECT THAT ARE **NOT** SUPPORTED BY eDATA = 6

d. MISCELLANEOUS DETAILS, TYPICALLY DESIGNATED BY M#, ARE NOT SUPPORTED.

INSTRUCTIONS TO STEEL FABRICATOR

1. SIDEPLATE LICENSE FEE:
   a. THE STEEL FABRICATOR'S BID PRICE FOR PROCUREMENT, FABRICATION AND ERECTION OF STRUCTURAL AND MISCELLANEOUS STEEL SHALL INCLUDE THE SIDEPLATE LICENSE FEE FOR THE PROJECT. EACH PROSPECTIVE STEEL FABRICATOR WHO BIDS THE PROJECT SHALL FORMALLY REQUEST THE SIDEPLATE LICENSE FEE BY ACCESSING THE SIDEPLATE WEBSITE (http://www.sideplate.com).
   b. UPON THE SUCCESSFUL STEEL FABRICATOR SIGNING A CONTRACT TO FABRICATE STRUCTURAL STEEL FOR THIS PROJECT, THE STEEL FABRICATOR SHALL SUBMIT A PURCHASE ORDER (PO) TO SIDEPLATE SYSTEMS, INC. FOR THE TOTAL AMOUNT OF THE SIDEPLATE LICENSE FEE AND SHALL INCLUDE SAID FEE IN ITS FIRST CONSTRUCTION DRAW.
   c. THE STEEL FABRICATOR SHALL MAKE PAYMENT OF THE SIDEPLATE LICENSE FEE DIRECTLY TO:

      SIDEPLATE SYSTEMS, INC.
      25909 PALA, SUITE 200
      MISSION VIEJO, CA 92691
      TEL: 949-238-8900

SUBMITTALS

1. IN ADDITION TO THE REQUIRED SUBMITTALS SPECIFIED BY THE BALANCE OF THE CONTRACT DOCUMENTS, THE FOLLOWING SUBMITTALS SHALL BE SENT TO SIDEPLATE SYSTEMS, INC. ELECTRONICALLY VIA THE STRUCTURAL ENGINEER OF RECORD FOR THEIR REVIEW AND DISPOSITION:
   a. QUALITY CONTROL PROGRAM (REQUIRED IF NOT AISC CERTIFIED)
   b. ONE ELECTRONIC COPY OF ALL STRUCTURAL STEEL DRAWINGS THAT EITHER DIRECTLY PERTAINS TO AND/OR AFFECTS THE SHOP FABRICATION OR FIELD ERECTION OF THE SIDEPLATE STEEL FRAME CONNECTION SYSTEM, INCLUDING THE INITIAL SUBMITTAL AND ALL CORRECTED RE-SUBMITTALS OF AFFECTED DRAWINGS. SIDEPLATE SYSTEMS, INC. SHALL BE GIVEN, AS A MINIMUM, THE SAME SPECIFIED REVIEW TIME (NOT LESS THAN SEVEN BUSINESS DAYS) AS THE ENGINEER OF RECORD.
   c. PROCEDURE QUALIFICATION RECORD (PQR), OR WELDING PROCEDURE SPECIFICATIONS (WPS) AND CURRENT CERTIFICATE OF CONFORMANCE BY THE WIRE MANUFACTURER.
2. SENT BY SIDEPLATE:
   a. INTELLECTUAL PROPERTY RIGHTS NOTICE LABEL.
   b. USPTO PATENT LABEL STICKERS, SEE INTELLECTUAL PROPERTY SECTION FOR PLACEMENT.

MEETINGS

1. PRE-DETAILING MEETING
   a. PRIOR TO THE START OF DETAILING OF THE SHOP DRAWINGS, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-DETAILING MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE DETAILING OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
2. PRE-FABRICATION MEETING
   a. PRIOR TO THE START OF FABRICATION, THE FABRICATION CONTRACTOR SHALL FORMALLY REQUEST A PRE-FABRICATION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR THE FABRICATION OF THE SIDEPLATE CONNECTIONS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.
3. PRE-ERECTION MEETING
   a. PRIOR TO THE START OF STEEL ERECTION, THE ERECTION CONTRACTOR SHALL FORMALLY REQUEST A PRE-ERECTION MEETING FROM SIDEPLATE SYSTEMS, INC. THIS MEETING IS TYPICALLY A WEBINAR TO DISCUSS BEST PRACTICES FOR FIELD ERECTION OF THE SIDEPLATE BEAMS AND COLUMNS, AND TO CREATE A PROACTIVE FORUM TO ANSWER ANY QUESTIONS.

GENERAL

1. THE GOVERNING CODES SHALL CONSIST OF ANSI/AWS D1.1-2015 (AWS D1.1), ANSI/AWS D1.8-2016, ANSI/AISC 358-16 (INCLUDING SUPPLEMENT NO. 2), AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES (JUNE 15, 2016), 2016 RCSC SPECIFICATIONS FOR STRUCTURAL JOINTS USING HIGH-STRENGTH BOLTS, AND ALL APPLICABLE BUILDING AND JURISDICTIONAL CODES AND PROJECT STANDARDS SPECIFIED IN THE PROJECT SPECIFICATION STRUCTURAL STEEL SECTION. WHERE THE REQUIREMENTS DIFFER BETWEEN SIDEPLATE CONNECTION NOTES, THE GENERAL STRUCTURAL NOTES, AND THE GOVERNING CODES, THE MORE STRINGENT SECTION CRITERIA SHALL CONTROL.
2. ALPHA AND NUMERIC DESIGNATORS {X} & {#} USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF PLATES, ANGLES, AND WELDS ARE DEFINED BELOW:

{A}	SIDE PLATE FOR UNIAXIAL CONNECTIONS
{B}	BEAM FLANGE COVER PLATE, AS REQUIRED
{C}	VERTICAL SHEAR PLATE OR FLAT BAR WELDED TO BEAM WEB, AS REQUIRED
{D}	HORIZONTAL SHEAR PLATE OR FLAT BAR WELDED TO COLUMN WEB, AS REQUIRED
{E}	VERTICAL ANGLE WELDED TO THE VERTICAL SHEAR PLATE {C}, AS REQUIRED
{F}	VERTICAL SHEAR ELEMENT (VSE) WHICH CONSISTS OF PLATE {C} AND ANGLE {E} MATERIAL, AS REQUIRED
{G}	LONGITUDINAL ANGLE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED
{H}	LONGITUDINAL ANGLE WELDED TO THE BOTTOM BEAM FLANGE (OR TOP BEAM FLANGE AS REQUIRED)
{T}	HORIZONTAL PLATE WELDED TO THE OUTSIDE FACE OF SIDE PLATE {A}, AS REQUIRED
{1}	FILLET WELD CONNECTING SIDE PLATE {A} TO HORIZONTAL SHEAR PLATE {D} OR COLUMN
{2}	FILLET (AND/OR FLARE BEVEL) WELD CONNECTING INSIDE FACE OF SIDE PLATE {A} TO COLUMN [DEMAND CRITICAL]
{3}	FILLET WELD CONNECTING HORIZONTAL SHEAR PLATE {D} TO COLUMN, AS REQUIRED
{4}	FILLET WELD TO CONSTRUCT VSE {F} AND TO CONNECT IT TO THE WEB OF THE BEAM, AS REQUIRED
{5}	FILLET (AND/OR PJP) WELD CONNECTING BEAM FLANGE TIPS TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED [DEMAND CRITICAL]
{5a}	FILLET WELD CONNECTING OUTSIDE FACE OF BEAM FLANGE TO COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}, AS REQUIRED [DEMAND CRITICAL]
({5p})	PJP WELD CONNECTING ANGLE {H} TO BEVELED BEAM FLANGE [DEMAND CRITICAL]
{8}	FILLET (AND/OR PJP) WELD CONNECTING LONGITUDINAL ANGLE {G} (AND/OR PLATE {T}) TO SIDE PLATE {A}, AS REQUIRED [DEMAND CRITICAL]
({8p})	PJP WELD CONNECTING PLATE {T} TO SIDE PLATE {A}, AS REQUIRED [DEMAND CRITICAL]
{9}	FILLET WELD CONNECTING SIDE PLATE {A} TO COLUMN FACE, WRAPPED AROUND THREE SIDES OF SIDE PLATE {A} [DEMAND CRITICAL]
{10}	FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY [DEMAND CRITICAL]
{10p}	PJP WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY [DEMAND CRITICAL]
{10r}	REINFORCING FILLET WELD TO CONSTRUCT SIDE PLATE SLOTTED INTERLOCK ASSEMBLY [DEMAND CRITICAL]
{13}	FILLET WELD CONNECTING BUILT UP ANGLE {H} PLATES TOGETHER, AS REQUIRED [DEMAND CRITICAL]
({13p})	PJP WELD CONNECTING BUILT UP ANGLE {H} PLATES TOGETHER, AS REQUIRED [DEMAND CRITICAL]

3. ALPHA DESIGNATORS, USED HEREIN TO SIMPLIFY THE IDENTIFICATION OF DIMENSIONS OF THE SIDEPLATE CONNECTIONS, ARE DEFINED BELOW:

GAP	PHYSICAL SEPARATION BETWEEN THE END OF THE MOMENT FRAME BEAM AND THE ADJOINING FACE OF THE COLUMN FLANGE (AKA COLUMN/BEAM SEPARATION)
A	EXTENSION OF SIDE PLATE {A} FROM THE FACE OF THE COLUMN
B	DEPTH OF SIDE PLATE {A}
C	LENGTH OF COVER PLATE {B} AND/OR LONGITUDINAL ANGLE {H}
D	LENGTH OF SLOT FROM THE TOE OF THE RADIUS IN THE COVER PLATE {B}, AS REQUIRED
E	EDGE DISTANCE OF BOLT HOLES IN COVER PLATE {B}, AS REQUIRED
F	DISTANCE FROM END OF SIDE PLATE {A} TO A CJP WELDED SPLICE IN THE FLANGES AND WEB OF THE MOMENT FRAME BEAM, AS REQUIRED
G	GAGE DISTANCE TO CENTERLINE OF BOLT HOLES IN ANGLES {G} AND {H}, AND PLATE {T}, AS REQUIRED
H	ADDED DIMENSION TO COLUMN FLANGE WIDTH TO DEFINE TOTAL COVER PLATE {B} WIDTH
J	DISTANCE FROM END OF THE BEAM TO CENTERLINE OF VERTICAL BOLT HOLES IN VSE {F}, AS REQUIRED
K	CUT DIMENSION ON ANGLE {H}
L	DISTANCE FROM CENTERLINE OF HORIZONTAL BOLT HOLES IN BOTTOM ANGLE {H} TO BOTTOM MOST BOLT HOLE IN VSE {F}
R	RADIUS OF SLOT DIMENSION IN COVER PLATE {B}
S	HORIZONTAL SPACING BETWEEN BOLT HOLES
Y	ADDED DIMENSION TO COLUMN FLANGE WIDTH FOR ALLOWABLE SPREAD OF SIDE PLATES {A}

MATERIAL

1. PLATE, FLAT BAR, AND ANGLE MATERIAL:
   a. ALL PLATE MATERIAL SHALL BE ASTM A572, GRADE 50.
   b. ANGLE AND BAR MATERIAL SHALL HAVE A HIGH STRENGTH STEEL SPECIFICATION AND SHALL HAVE A MINIMUM YIELD STRENGTH (F_y) OF 50 KSI.
   c. ALL PLATE MATERIAL 2 INCHES THICK AND GREATER SHALL MEET THE FOLLOWING ADDITIONAL REQUIREMENTS:
      - CHARPY V-NOTCH (CVN) TOUGHNESS SHALL BE CONDUCTED IN ACCORDANCE WITH THE PROVISIONS OF ASTM A673, FREQUENCY P. A MINIMUM CVN TOUGHNESS OF 20 FT-LB AT A TEMPERATURE OF 70°F, IN ACCORDANCE WITH ASTM A6. SUPPLEMENTARY REQUIREMENT S5, SHALL BE DOCUMENTED IN THE CERTIFIED MILL TEST REPORT (CMTR). IF THE CMTR DOES NOT DOCUMENT THE MINIMUM CVN VALUES, A COUPON SHALL BE REMOVED FROM THE END OF EACH PIECE OF PLATE TO BE TESTED. THE LENGTH OF COUPON SHALL BE ORIENTED PARALLEL TO THE LONGITUDINAL AXIS OF THE MEMBER. A TEST SPECIMEN SHALL BE MACHINED FROM EACH COUPON IN ACCORDANCE WITH THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, SECTION A3-1d, BUILT-UP HEAVY SHAPES.
2. WELD FILLER METAL:
   a. THE WELD FILLER METAL AND ASSOCIATED WELDING PROCESS FOR ALL FILLET WELDS SHALL MEET THE REQUIREMENTS OF AWS D1.1 AND D1.8 SECTION 6.1 AND SHALL BE ANY OF THE FOLLOWING:
      - E70T-6, E71T-8, OR E70TG-K2 FOR FCAW
      - E7XT-9 FOR FLUX CORED ARC WELDING (FCAW) WITH GAS SHIELDING
      - F7A2-EXXX FOR SUBMERGED ARC WELDING (SAW)
      - E7018 STICK ELECTRODE FOR SHIELDED METAL ARC WELDING (SMAW)
      - ER70S-X, E70C-XM, OR E70C-XC FOR GMAW (EXCEPT SHORT CIRCUIT TRANSFER)
   b. THE WELD FILLER METAL USED FOR FILLET WELDS TO CONSTRUCT THE SIDEPLATE CONNECTION SYSTEM PRESENTED HEREIN SHALL DEMONSTRATE AN ENERGY EQUIVALENT TO A MINIMUM CVN TOUGHNESS OF 20 FT-LB IMPACT STRENGTH AT A TEMPERATURE OF 0°F AND 40 FT-LB IMPACT STRENGTH AT 70°F AS DETERMINED BY AWS CLASSIFICATION TEST METHODS OR MANUFACTURER CERTIFICATION.
   c. ALL WELD FILLER METAL SHALL SATISFY A MAXIMUM DIFFUSIBLE HYDROGEN CONTENT REQUIREMENT PER AWS D1.1 AND D1.8 SECTION 6.1, OF 16 MILLILITERS OF HYDROGEN PER 100 GRAMS OF WELD METAL (H16) OR LESS.
   d. REFER TO AWS D1.8 ANNEX B FOR REQUIREMENTS WHEN MIXING FCAW-S WITH OTHER WELD PROCESSES.

3. HIGH STRENGTH BOLTS/FASTENERS:
   a. BOLTS SHALL BE TYPE 1 OR TYPE 3 AND SHALL BE ASTM F3125, GRADE A490-X OR GRADE F2280-X, OR F3148 FIXED SPLINE BOLT ASSEMBLIES. THE BOLT HEAD SHALL BE DISTINCTIVELY MARKED WITH A MINIMUM MARKING OF A490, A490TC, OR 144 RESPECTIVELY. AN ALTERNATIVE DESIGN THAT MEETS THE REQUIREMENTS OF RCSC SECTION 2.8 MAY BE USED, WITH THE WRITTEN APPROVAL FROM SIDEPLATE SYSTEMS, INC.
   b. FOR BOLTS UP TO 1 1/4 INCH DIAMETER WASHERS SHALL BE ORDINARY THICKNESS AND ASTM F436 TYPE 1 OR TYPE 3. 1 1/2 INCH DIAMETER BOLTS SHALL REQUIRE 5/16 INCH THICK WASHER.
   c. NUTS SHALL BE ASTM A563 GRADE DH OR DH3.
   d. THE BOLT ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL. F2280 AND F3148 ASSEMBLIES SHALL ONLY BE LUBRICATED BY THE SUPPLIER.
   e. THE MILL TEST REPORT (MTR) MUST HAVE DOCUMENTED LOT TRACEABILITY, STATEMENT OF DIMENSIONAL RESULTS, FULL CHEMICAL AND MECHANICAL TEST RESULTS TO THE SPECIFICATIONS ABOVE.
   f. THE USE OF FINGER SHIMS ARE ACCEPTABLE PER BOLTING SECTION 8.
4. ROLLED SHAPES:
   a. ALL ROLLED SHAPES USED FOR COLUMNS AND BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A992 GRADE 50 UNO.
   b. FOR EACH DIFFERENT SIZE OF STEEL FRAME COLUMNS AND BEAMS IDENTIFIED IN THE CONTRACT DOCUMENTS AS PART OF THE SFRS INVOLVING ASTM A6 HOT-ROLLED SHAPES WITH FLANGES 2 INCHES THICK OR THICKER, CHARPY V-NOTCH (CVN) TOUGHNESS SHALL BE CONDUCTED IN ACCORDANCE WITH THE PROVISIONS OF ASTM A673, FREQUENCY H. A MINIMUM CVN TOUGHNESS OF 20 FT-LB AT A TEMPERATURE OF 70°F, IN ACCORDANCE WITH ASTM A6, SUPPLEMENTARY REQUIREMENT S30, SHALL BE DOCUMENTED IN THE CERTIFIED MILL TEST REPORT (CMTR). IF THE CMTR DOES NOT DOCUMENT THE MINIMUM CVN VALUES, A COUPON SHALL BE REMOVED FROM THE END OF EACH MEMBER SHAPE SAMPLED. THE LENGTH OF COUPON SHALL BE ORIENTED PARALLEL TO THE LONGITUDINAL AXIS OF THE MEMBER. A TEST SPECIMEN SHALL BE MACHINED FROM EACH COUPON, IN ACCORDANCE WITH THE AISC SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS, SECTION A3-1c, ROLLED HEAVY SHAPES. THE LONGITUDINAL AXIS OF THE TEST SPECIMEN SHALL BE LOCATED AS NEAR AS PRACTICABLE TO A POINT MIDWAY BETWEEN THE INNER FLANGE SURFACE AND THE CENTER OF FLANGE THICKNESS, AT THE INTERSECTION OF THE WEB MID-THICKNESS.
5. WELDED BUILT-UP SHAPES:
   a. FOR EACH DIFFERENT SIZE OF STEEL FRAME COLUMNS AND BEAMS IDENTIFIED IN THE CONTRACT DOCUMENTS AS PART OF THE SFRS INVOLVING PLATE 2 INCHES THICK OR THICKER USED TO CONSTRUCT WELDED BUILT-UP SECTIONS SHALL FOLLOW PLATE MATERIAL SPECIFICATIONS OUTLINED IN SECTION 1.
6. HSS TUBE SHAPES:
   a. ALL HSS SHAPES USED FOR BEAMS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL, AS A MINIMUM, BE ASTM A500 GRADE B OR GRADE C OR ASTM A1085.
   b. ALL HSS SHAPES USED FOR COLUMNS IN CONSTRUCTING SIDEPLATE MOMENT FRAMES SHALL BE ASTM A500 GRADE C OR ASTM A1085.

PREPARATION

1. THE STEEL FABRICATION AND ERECTION SUBCONTRACTORS SHALL EMPLOY A DISTORTION CONTROL PROGRAM PRIOR TO THE START OF SIDEPLATE MOMENT FRAME FABRICATION. THE DISTORTION CONTROL PROGRAM SHALL BE IN ACCORDANCE WITH THE PROVISIONS OF AWS D1.1 SECTION 5.20 AND 5.21 TO ENSURE THAT THE FOLLOWING ARE MAINTAINED:
   - DIMENSIONAL ACCURACY
   - FRAMING AND ALIGNMENT TOLERANCES
   - COMPLIANCE WITH AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES, SECTION 7.0, ERECTION PROVISIONS
   - CONTROL OF DISTORTION AND WELD SHRINKAGE
2. BASE METAL SURFACE PREPARATION: SURFACES ON WHICH WELD METAL IS TO BE DEPOSITED, INCLUDING BUT NOT LIMITED TO COLUMN FLANGE TIPS (I.E., COLUMN FLANGE-TO-SIDE PLATE {A} ATTACHMENT), BEAM FLANGE TIPS (I.E., BEAM FLANGE-TO-COVER PLATE {B} ATTACHMENT), AND THERMAL CUT EDGES (E.G., LONGITUDINAL EDGES OF COVER PLATE {B}), SHALL BE SMOOTH, UNIFORM, AND FREE FROM LOOSE OR THICK SCALE, SLAG, RUST, MOISTURE, GREASE AND OTHER FOREIGN MATERIAL THAT WOULD PREVENT PROPER WELDING.
3. THERMAL CUTTING:
   a. THE ROUGHNESS OF ALL THERMAL-CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES; CUT ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.
   b. FLAME CUT SURFACES SHALL BE FREE OF GLOBULES AND LOOSE SLAG. THE THERMAL CUTTING EQUIPMENT SHALL BE ADJUSTED AND MANIPULATED SO AS TO AVOID CUTTING BEYOND (INSIDE) THE PRESCRIBED LINES.
   c. THERMAL CUTTING PROCESSES SHALL BE LIMITED TO PLASMA ARC-CUTTING OR OXYFUEL GAS PROCESSES.
4. WEB/FLANGE FILLET AREA:
   a. UNLESS NOTED OTHERWISE, VERTICAL SHEAR PLATES {C} (AS REQUIRED) AND HORIZONTAL SHEAR PLATES {D} (AS REQUIRED) SHALL HAVE THEIR INSIDE CORNERS CLIPPED PER AWS D1.8 CLAUSE 4 TO PREVENT WELDING IN THE K-AREA BETWEEN WEB AND FLANGE WHEN THE COLUMN IS DEEPER THAN W16x, THESE CLIPS SHALL START AND STOP A DISTANCE OF 1 1/2 INCHES FROM THE TANGENT OF THE FLANGE TO THE WEB FILLET (I.E., THE AISC 'k' DIMENSION). THE DIMENSIONS OF THE CLIPPED CORNER SHALL BE DETAILED ON THE SHOP DRAWINGS.
   b. WELD {3} RETURNS ON THE INSIDE FACE OF THE COLUMN FLANGES (AS OCCURS): IN NO CASE SHALL A WELD TIE-IN OF WELD {3} BE ALLOWED ACROSS THE CLIPPED CORNER OF THE HORIZONTAL SHEAR PLATES {D} AT THE K-AREA BETWEEN WEB AND FLANGE.
5. ANY HOLE MAKING METHODS ALLOWED BY AISC SPECIFICATIONS ARE ALLOWED WITH THE EXCEPTION THAT WHEN USING A THERMAL CUTTING PROCESS, IT IS NOT PERMITTED FOR PLATES THICKER THAN 2 INCHES (50 MM).

WELDING

1. WELDER QUALIFICATION: THE PERFORMANCE OF ALL WELDERS, WELDING OPERATORS AND TACK WELDERS SHALL BE QUALIFIED IN CONFORMANCE WITH AWS D1.1, SECTION 4, PART C TO DEMONSTRATE ABILITY TO PRODUCE SOUND WELDS.
2. WELDING PROCEDURE SPECIFICATIONS (WPS):
   a. THE APPROPRIATE STEEL SUB-CONTRACTOR SHALL PREPARE A SPECIFIC WRITTEN WPS FOR EVERY DIFFERENT WELDING APPLICATION. DIFFERENT WELDING APPLICATIONS INCLUDE, BUT ARE NOT LIMITED TO, THE JOINT DETAILS AND TOLERANCES, PREHEAT AND INTERPASS TEMPERATURE, SINGLE OR MULTIPLE STRINGER PASSES, WELDING CURRENT, POLARITY, ALLOWABLE AMPERAGE RANGES, ALLOWABLE VOLTAGE RANGES, ALLOWABLE TRAVEL SPEED RANGES, ELECTRODE EXTENSION, ROOT TREATMENT, WELDING POSITION, WELDING PROCESS, ELECTRODE MANUFACTURER, FILLER METAL TRADE NAME FOR THE ELECTRODE TYPE SELECTED, AND OTHER ESSENTIAL VARIABLES AS DEFINED IN AWS D1.1 REQUIRED TO COMPLETE THE FABRICATION AND ERECTION OF THE SPECIFIED SIDEPLATE CONNECTIONS. AMPERAGE, VOLTAGE, TRAVEL SPEED AND ELECTRODE EXTENSION SHALL BE WITHIN THE FILLER METAL MANUFACTURER'S RECOMMENDATIONS.
   b. EACH WPS PREPARED SHALL BE BASED ON AND REFERENCED TO A CURRENT CERTIFICATE OF CONFORMANCE PROVIDED BY THE WIRE MANUFACTURER AND SHALL BE USED AS A SUPPORTING PQR PROVIDED FULL COMPLIANCE WITH EVERY SINGLE CONDITION OF PREQUALIFICATION FOUND IN AWS D1.1 SECTION 3 AND AWS D1.8 HIGH AND LOW HEAT INPUT FOR PREQUALIFIED FILLET AND CJP WELDS. THE SELECTION OF THIS OPTION BY THE STEEL FABRICATION/ERECTION SUBCONTRACTOR IS PREDICATED ON ITS ACKNOWLEDGEMENT THAT ITS CERTIFIED WELDERS ARE EXPERIENCED AND CONFIDENT IN THE USE AND SETTINGS SPECIFIED IN THE CERTIFICATE OF CONFORMANCE WITH THE ALLOWABLE TOLERANCES FOR ESSENTIAL VARIABLES FOUND IN TABLE 3.7 OF AWS D1.1. PREVIOUS WPS QUALIFICATION PERFORMED BY THE STEEL FABRICATION/ERECTION SUBCONTRACTOR THAT COMPLIES WITH THE PROVISIONS HEREIN MAY SATISFY THIS REQUIREMENT. EACH WPS SHALL BE SUBMITTED TO THE ENGINEER OF RECORD, AND TO SIDEPLATE SYSTEMS, INC. FOR REVIEW AND DISPOSITION PRIOR TO STARTING THE WORK.
   c. THE APPROVED WPS FOR EACH APPLICABLE PRODUCTION WELD SHALL BE CLEARLY DISPLAYED TO PROVIDE READY ACCESS BY THE ASSIGNED WELDERS, WELDING SUPERVISORS AND INSPECTORS.
   d. IN LIEU OF THE ABOVE PROCEDURE, THE STEEL FABRICATOR MAY CHOOSE TO QUALIFY A WPS BASED ON A PROCEDURE QUALIFICATION RECORD (PQR), WHICH SHALL BE PROVIDED WITH THE WPS SUBMITTAL. THE PQR SHALL CONFORM TO THE REQUIREMENTS OF AWS D1.1, SECTION 4.1 AND EMPLOY THE PROPER ACCEPTANCE CRITERIA.
3. TACK WELDS:
   a. TACK WELDS SHALL BE PLACED WHERE THEY WILL BE INCORPORATED INTO A FINAL WELD AND SHALL BE SUBJECT TO THE SAME QUALITY REQUIREMENTS AS THE FINAL WELDS, INCLUDING POSITION, PREHEAT AND UNDERCUT, IN ACCORDANCE WITH AWS D1.1. THESE QUALITY REQUIREMENTS SHALL APPLY EQUALLY TO TACK WELDING OF ANY OTHER CONSTRUCTION AIDS.
4. PREHEAT AND INTERPASS TEMPERATURE REQUIREMENTS:
   a. THE MINIMUM PREHEAT AND INTERPASS TEMPERATURES FOR A GIVEN THICKNESS OF BASE METAL TO BE WELDED SHALL BE IN ACCORDANCE WITH AWS D1.1 TABLE 3.3.
   b. IN NO CASE, REGARDLESS OF THE WELDING PROCESS, SHALL THE PREHEAT TEMPERATURE BE LESS THAN THAT REQUIRED TO DRIVE OFF ANY SURFACE MOISTURE OR CONDENSATION WHICH MAY BE PRESENT ON THE STEEL ELEMENTS TO BE WELDED.
   c. MAXIMUM INTERPASS TEMPERATURE SHALL NOT EXCEED 550°F, MEASURED AT A DISTANCE NOT EXCEEDING THREE INCHES FROM THE START OF THE WELD PASS. WELDING OPERATORS AND INSPECTORS SHALL UTILIZE TEMPERATURE MEASURING DEVICES.
5. ALL SLAG SHALL BE REMOVED AFTER EACH WELD PASS AND THE WELD AND THE ADJACENT BASE METAL SHALL BE CLEANED BEFORE WELDING OVER PREVIOUSLY DEPOSITED WELD METAL. THIS REQUIREMENT SHALL APPLY NOT ONLY TO SUCCESSIVE LAYERS BUT ALSO TO SUCCESSIVE BEADS AND TO THE CRATER AREA WHEN WELDING IS RESUMED AFTER ANY INTERRUPTION, IN ACCORDANCE WITH AWS D1.1 SECTION 5.29.1.
6. PEENING SHALL NOT BE ALLOWED.
7. WELD RUN-OFF TABS SHALL NOT BE USED FOR FILLET WELDS (EXCEPT WHEN REQUIRED ON INTERLOCKING SLOTTED SIDE PLATES).
8. WELD END TERMINATIONS FOR RECESSED PLATE {D} OPTION ONLY WHERE WELD {3} IS REQUIRED ON BOTH WEB AND FLANGES:
   a. IN ORDER TO PREVENT TIE-IN BETWEEN THE ENDS OF WELD {3} CLOSEST TO THE INSIDE FACE OF SIDE PLATE {A} AND WELD {1} THE ENDS OF WELD {1} SHALL BE HELD BACK FROM THE FLANGE TIP OF COLUMN BY A DISTANCE EQUAL TO WELD {3} PLUS 1/8 INCH. IN NO CASE SHALL A WELD TIE-IN BETWEEN THE ENDS OF WELD {1} AND THE ENDS OF WELD {3} CLOSEST TO THE INSIDE FACE OF SIDE PLATE {A} BE ALLOWED. FABRICATION PRIORITY IS GIVEN HEREIN TO WELDING FILLET WELD {3} ALONG ITS FULL DISTANCE.
9. ARC STRIKES WITHIN THE SIDEPLATE PROTECTED ZONE ON BOTH THE INTERIOR AND EXTERIOR FACES OF SIDE PLATES {A}, WITHIN THE TOP AND BOTTOM 6 INCHES OF PLATE HEIGHT SHALL BE AVOIDED. IF THEY DO OCCUR, THEY SHALL BE REMOVED BY GRINDING AND SHALL HAVE BASE METAL REPAIRED PER AWS D1.8 SECTION 6.18, WITH NON-DESTRUCTIVE TESTING AS NECESSARY.

BOLTING

1. BOLTS/FASTENERS SHALL BE INSTALLED TO PRETENSIONED CONDITION USING ONE OF THE METHODS PRESCRIBED HERE: TURN-OF-NUT (A490), CALIBRATED WRENCH (A490), TWIST-OFF-TYPE TENSION-CONTROL BOLT (F2280), TORQUE AND ANGLE METHOD (F3148), OR WHEN HEAVY HEX BOLTS ARE REQUIRED DIRECT TENSION INDICATORS (DTI) (ASTM F959). FOR TURN-OF-NUT THE THREAD AND NUT SHOULD BOTH BE MARKED TO MAKE SURE THE REQUIRED TURN IS ACHIEVED.
2. FOR ALL PRETENSIONING METHODOLOGIES, ALL FASTENER ASSEMBLIES WITHIN THE JOINT SHALL FIRST BE BROUGHT TO A SNUG TIGHT CONDITION, FOLLOWED BY A SYSTEMATIC PRETENSIONING PROCESS. PRETENSIONING SHALL BEGIN AT THE MOST RIGID PART OF THE JOINT AND CONTINUE IN A MANNER THAT WILL MINIMIZE THE RELAXATION OF PREVIOUSLY PRETENSIONED FASTENERS, UNTIL THE CONNECTED PLIES ARE IN AS FIRM CONTACT AS POSSIBLE. IN ADDITION:
   a. FOR TURN-OF-NUT METHOD, AFTER SNUGGING THE JOINT USING AT LEAST A FEW IMPACTS OF A WRENCH OF THE FULL EFFORT OF AN IRON WORKER, ROUTINE MARKING OR MATCH-MARKING SHALL BE USED TO VERIFY RELATIVE BOLT AND NUT ROTATION TO RCSC TABLE 8.2. FOR BOLT LENGTHS OVER 4D UP TO 8D, THE BOLT OR NUT SHALL BE ROTATED 180 DEGREES (1/2 TURN). FOR BOLT LENGTHS OVER 8D UP TO 12D, THE BOLT OR NUT SHALL BE ROTATED 240 DEGREES (2/3 TURN). FOR BOLT LENGTHS OVER 12D, THE REQUIRED ROTATION SHALL BE DETERMINED USING A BOLT TENSION MEASUREMENT DEVICE.
   b. FOR CALIBRATED WRENCH METHOD, THE WRENCH AND BOLT ASSEMBLY LOT SHALL BE TESTED IN COMBINATION DAILY. IF NECESSARY, THE WRENCH SHALL BE CALIBRATED FOR PROPER OUTPUT PRIOR TO USE. THE INSTALLATION TORQUE NEED NOT PRODUCE A RELATIVE ROTATION EQUAL TO OR GREATER THAN THAT SPECIFIED BY THE TURN-OF-NUT METHOD.
   c. FOR TWIST-OFF-TYPE TENSION-CONTROL BOLT METHOD, FINAL TENSIONING SHALL BE DONE IN A CONTINUOUS OPERATION UNTIL THE TIP TWISTS OFF. ANY BOLT ASSEMBLIES WHICH HAD THE TIP TWIST OFF DURING THE SNUGGING OPERATION SHALL BE REMOVED AND REPLACED.
   d. FOR COMBINED METHOD (ASTM F3148), AFTER SNUGGING THE JOINT USING THE APPROPRIATE INSTALLATION TOOL, ROUTINE MONITORING OR MATCH MARKING SHALL BE USED TO VERIFY RELATIVE BOLT AND NUT ROTATION. FOR BOLT LENGTHS UP TO 4D, THE NUT SHALL BE ROTATED 90 DEGREES (1/4 TURN). FOR BOLT LENGTHS OVER 4D UP TO 8D, THE NUT SHALL BE ROTATED 120 DEGREES (1/3 TURN). FOR BOLT LENGTHS OVER 8D THE MANUFACTURER SHALL PROVIDE APPROPRIATE INSTALLATION REQUIREMENTS.
3. REUSE OF A490, F2280, AND F3148 BOLT ASSEMBLIES SHALL NOT BE ALLOWED. TOUCHING UP OR RE-TIGHTENING BOLTS THAT MAY HAVE BEEN LOOSENED BY THE INSTALLATION OF ADJACENT BOLTS SHALL NOT BE CONSIDERED TO BE A REUSE.
4. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS.
5. THE BOLT LENGTH USED SHALL BE SUCH THAT THE BOLT THREAD EXTENDS BEYOND OR IS AT LEAST FLUSH WITH THE OUTER FACE OF THE NUT WHEN PROPERLY INSTALLED.
6. FASTENER COMPONENTS SHALL BE PROTECTED FROM DIRT AND MOISTURE

QUALITY CONTROL

1. THE FABRICATOR AND ERECTOR SHALL BE RESPONSIBLE FOR QUALITY CONTROL BY PROVIDING, AS A MINIMUM, IN-PROCESS VISUAL INSPECTION OF ALL FABRICATION AND ERECTION ACTIVITIES TO ENSURE THAT MATERIALS AND WORKMANSHIP MEET THE REQUIREMENTS OF THE CONTRACT DOCUMENTS, AND SHALL INCLUDE WORK PERFORMED PRIOR TO ASSEMBLY. SUCH WORK SHALL INCLUDE, BUT NOT BE LIMITED TO, VERIFYING THAT EFFECTIVE PROCEDURES AND METHODS HAVE BEEN EMPLOYED IN THE FORM OF A DISTORTION CONTROL PROGRAM TO ACCOUNT FOR AND COUNTERACT THE EFFECTS OF WELD SHRINKAGE, EXISTING BEAM SWEEP AND CAMBER, AND CHANGES IN MOMENT FRAME GEOMETRY DUE TO SKEWED AND CURVED DESIGN CONFIGURATIONS (AS OCCURS), TO ENSURE COMPLIANCE WITH SPECIFIED ERECTION AND ALIGNMENT TOLERANCES. QC INSPECTION SHALL INCLUDE **HOLD POINTS** FOR THE FOLLOWING:
   a. COLUMN TREE ASSEMBLY
      1. VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES {A} ARE TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (-3/16 INCH MAX), CONTACT SIDEPLATE SYSTEMS, INC FOR APPROPRIATE RECOMMENDATIONS.
      2. MINIMUM CLEAR DIMENSION SHALL BE VERIFIED AFTER PLACEMENT OF WELD {2}, COOLING OF WELD {2}, AND REMOVAL OF TEMPORARY SHOP CONSTRUCTION AID(S). VERIFY THAT A MINIMUM ACTUAL COLUMN FLANGE WIDTH DIMENSION OCCURS ANYWHERE IN BETWEEN THE SIDE PLATES {A} FROM TOP TO BOTTOM. THE SIDE PLATES SHALL BE PARALLEL TO ONE ANOTHER. IN NO CASE SHALL THEY BE LESS THAN THE ACTUAL COLUMN FLANGE WIDTH.
      3. MAXIMUM SPREAD DIMENSION OF SIDE PLATE {A} SHALL NOT EXCEED ACTUAL COLUMN FLANGE WIDTH PLUS THE SCHEDULED SPREAD DIMENSION Y. THE FIELD CONSTRUCTION AID SHALL BE PLACED AND HOLD THE SIDE PLATES IN THIS FLARED CONDITION UNTIL THE BEAM HAS BEEN SAFELY ERECTED. IN NO CASE SHALL THE SPREAD CAUSE PERMANENT DEFORMATION IN THE SIDE PLATES.
      4. VERIFICATION OF BOLT HOLE ELEVATION AND SPACING FOR POSITION OF SIDE PLATE {A} AND PROPER POSITION AND ELEVATION OF ANGLES {G}.
   b. BEAM ASSEMBLY
      1. VERIFICATION OF PERPENDICULAR ALIGNMENT BETWEEN THE TOP COVER PLATE {B} AND BOTTOM ANGLES {H} TO THE WEB OF THE BEAM, TO MINIMIZE, IF NOT ELIMINATE, ANY MISALIGNMENT OF BOLT HOLES DUE TO BEAM FLANGE TILT WHEN THE BEAM HAS BEEN LOWERED INTO PLACE.
      2. VERIFICATION OF BOLT HOLE SPACING AND POSITION ON COVER PLATE {B} AND ANGLES {H}. CONSIDERATION SHALL BE GIVEN TO THE CUPPING EFFECT OF THE TOP COVER PLATE {B}, DUE TO WELD SHRINKAGE.
      3. VERIFICATION OF THE DISTANCE BETWEEN EXTERIOR ANGLE {H} FACES AND THEIR RESPECTIVE BOLT HOLE PLACEMENT TO EACH OTHER (VERTICALLY AND HORIZONTALLY).
      4. VERIFICATION THAT IN NO CASE SHALL THE OUTSIDE FACE OF VSE {F} EXTEND BEYOND THE OUTSIDE FACES OF THE LONGITUDINAL ANGLES {H}.
      5. VERIFICATION THAT VERTICAL PLACEMENT OF VSE {F} IS IN THE CORRECT LOCATION.
2. EXPOSURE LIMITATIONS ON ELECTRODES:
   a. AFTER REMOVAL FROM PROTECTIVE PACKAGING, THE PERMISSIBLE ATMOSPHERIC EXPOSURE TIME OF ELECTRODES SHALL NOT EXCEED THE ELECTRODE MANUFACTURER'S GUIDELINES AND SHALL BE IN ACCORDANCE WITH AWS D1.8 SECTION 6.4.
3. FILLET WELD FIT-UP TOLERANCES:
   a. THE PARTS TO BE JOINED BY FILLET WELDS SHALL BE BROUGHT INTO AS CLOSE CONTACT AS PRACTICABLE, USING AS NECESSARY SUITABLE CLAMPING MEANS. THE ROOT OPENING (I.E., THE FIT-UP GAP) SHALL NOT EXCEED 1/4 INCH. FOR FILLET WELD ROOT GAPS GREATER THAN 1/16 INCH, THE LEG SIZE (I.E., THE SPECIFIED SIZE) OF FILLET WELD SHALL BE INCREASED BY THE AMOUNT OF THE ROOT OPENING.
4. THERMAL CUTTING:
   a. THE ROUGHNESS OF ALL THERMAL-CUT SURFACES SHALL BE NO GREATER THAN AN ANSI SURFACE ROUGHNESS VALUE OF 1000 MICRO-INCHES. ROUGHNESS EXCEEDING THIS VALUE AND NOTCHES OR GOUGES NOT MORE THAN 3/16 INCH DEEP SHALL BE REMOVED BY MACHINING OR GRINDING. NOTCHES OR GOUGES IN THE THERMALLY CUT EDGES DEEPER THAN 3/16 INCH SHALL BE REPAIRED PER AWS.
5. TENSION CALIBRATION FOR PRE-INSTALLATION:
   a. TENSION CALIBRATION SHALL BE USED TO CONFIRM THE SUITABILITY OF THE COMPLETE FASTENER ASSEMBLY, AND THE PROCEDURE TO BE USED BY THE BOLTING CREW. A MINIMUM SAMPLING OF AT LEAST THREE COMPETE FASTENER ASSEMBLIES OF EACH COMBINATION OF DIAMETER, LENGTH, GRADE, WASHER REQUIREMENTS, LUBRICATION, AND LOT SHALL BE USED. CALIBRATION SHALL BE DONE AT THE SITE OF INSTALLATION IN A TENSION CALIBRATOR TO VERIFY THAT THE PRETENSIONING METHOD IS ADEQUATE.
   b. IF FASTENER ASSEMBLIES ARE FOUND TO BE LESS THAN THE FOLLOWING SPECIFIED REQUIREMENTS (PRETENSION VALUE AS FOLLOWS: 1 INCH DIAMETER = 67 KIPS, 1 1/8 INCH DIAMETER = 84 KIPS, 1 1/4 INCH DIAMETER = 107 KIPS, AND 1 1/2 INCH DIAMETER = 155 KIPS), THE CAUSE(S) SHALL BE DETERMINED AND RESOLVED BEFORE USING SAID FASTENER ASSEMBLIES.
6. EXPOSURE LIMITATION ON FASTENERS:
   a. FASTENER COMPONENTS SHALL BE PROTECTED FROM CONTAMINATION (DIRT AND MOISTURE) AND SHALL BE STORED IN CLOSED CONTAINERS AT THE SITE OF INSTALLATION. FASTENER COMPONENTS THAT ARE NOT INCORPORATED INTO THE WORK SHALL BE RETURNED TO PROTECTED STORAGE AT THE END OF THE WORK SHIFT. FASTENER COMPONENTS THAT ACCUMULATE RUST OR DIRT SHALL NOT BE USED UNLESS THEY ARE RE-QUALIFIED AND APPROVED BY SIDEPLATE SYSTEMS, INC.
7. WASHERS:
   a. CONFIRM F436 CORRECTLY SIZED WASHERS HAVE BEEN USED IN THE FASTENER ASSEMBLY.

QUALITY ASSURANCE

IN ADDITION TO ALL OTHER QUALITY ASSURANCE INSPECTION ACTIVITIES, THE OWNER'S VERIFICATION INSPECTOR SHALL BE RESPONSIBLE FOR:

1. WELDING:
   a. TO ASSURE THE PROPER AMPERAGE AND VOLTAGE OF THE WELDING PROCESS, THE USE OF HAND HELD CALIBRATED AMP AND VOLT METERS SHALL BE USED. THIS EQUIPMENT SHALL BE USED BY THE FABRICATOR AND THE INSPECTOR. AMPERAGE AND VOLTAGE SHALL BE MEASURED NEAR THE ARC. TRAVEL SPEED AND ELECTRODE STICK OUT SHALL BE VERIFIED TO BE IN COMPLIANCE WITH THE APPROVED WPS.
   b. VISUAL INSPECTION SHALL BE PERFORMED ON ALL SHOP WELDS.
   c. EACH WELDER EMPLOYED ON THE PROJECT SHALL UNDERSTAND ALL THE REQUIREMENTS OF THE WELDING PROCEDURE SPECIFICATION(S) BEFORE WELDING ON THE PROJECT.
   d. AS-BUILT BEAM TO COLUMN GAP PER CONNECTION SCHEDULE IS ALLOWED TO BE INSTALLED WITH A TOLERANCE OF PLUS OR MINUS 1/2 INCH.
   e. EACH WPS AND QUALIFICATION DOCUMENT SHALL BE SUBMITTED TO THE SEOR AND TO SIDEPLATE SYSTEMS, INC. FOR REVIEW AND APPROVAL PRIOR TO THE START OF FABRICATION.
   f. WHERE WELDING OF HORIZONTAL SHEAR PLATE {D} OR VERTICAL SHEAR PLATE {C} ENCROACH INTO THE K-AREA (AS DEFINED IN AISC 341), THE WEB SHALL BE VISUALLY CHECKED FOR CRACKS. THE INSPECTION AREA SHALL INCLUDE THE K-AREA BASE METAL WITHIN 3 INCHES OF THE WELD. THE INSPECTION SHALL BE PERFORMED NO SOONER THAN 48 HOURS FOLLOWING COMPLETION OF THE WELDING PER AWS D1.8, SECTION 7.4.
2. FAYING SURFACES:
   a. THE SURFACES ADJACENT TO THE BOLT HEAD AND NUT SHALL BE FREE OF DIRT AND OTHER FOREIGN MATERIAL. COMPRESSIBLE MATERIALS, OTHER THAN THE SPECIFIED COATINGS, SHALL NOT BE PLACED WITHIN THE GRIP OF THE BOLT.
   b. FAYING SURFACES ARE PERMITTED TO BE COATED AND UNCOATED AND SHALL BE FREE OF LOOSE SCALE AND ANY INADVERTENT OVERSPRAY. IF COATING IS REQUIRED, THE FAYING SURFACES SHALL FIRST BE BLAST CLEANED AND SUBSEQUENTLY COATED WITH A COATING THAT QUALIFIES AS A CLASS 'A' COATING.
   c. GALVANIZED FAYING SURFACES SHALL BE IN ACCORDANCE WITH ASTM A123.
   d. TO AVOID COMPRESSIVE CREEP AND SIGNIFICANT LOSS OF PRETENSION, THE COMBINED COATING THICKNESS SHALL IN NO CASE BE MORE THAN 15 MILS PER SURFACE.
   e. AFTER THE CONNECTIONS HAVE BEEN ASSEMBLED, VISUALLY ENSURE THAT THE PLIES OF THE CONNECTED ELEMENTS HAVE BEEN BROUGHT INTO AS CLOSE OF CONTACT AS PRACTICABLE WITH ONE ANOTHER. GAPS UP TO 1/8 INCH BETWEEN THE SURFACES SHALL BE ALLOWED. GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE SYSTEMS, INC.
3. PRETENSIONING:
   A MINIMUM BOLT PRETENSION VALUE SHALL BE AS FOLLOWS: 1 INCH DIAMETER = 64 KIPS, 1 1/8 INCH DIAMETER = 80 KIPS, 1 1/4 INCH DIAMETER = 102 KIPS, AND 1 1/2 INCH DIAMETER = 148 KIPS. THE INSPECTOR SHALL OBSERVE THE PRE-INSTALLATION VERIFICATION TESTING. SUBSEQUENTLY, THE INSPECTOR SHALL ENSURE THAT THE BOLTING CREW PROPERLY FOLLOW THE CORRECT METHODOLOGY OUTLINED BELOW:
   a. FOR TURN OF THE NUT METHOD: BY VISUAL OBSERVATION ENSURE THAT THE NUT AND PROTRUDING END OF THE BOLT HAVE BEEN MATCH-MARKED SO THAT THE PROPERLY ROTATED TURNED ELEMENT CAN BE VISUALLY OBSERVED RELATIVE TO THE UNTURNED ELEMENT.
   b. FOR CALIBRATED WRENCH METHOD: BY ROUTINE OBSERVATION ENSURE THAT THE CALIBRATED WRENCH IS BEING APPLIED TO THE TURNED ELEMENT.
   c. FOR TWIST-OFF-TYPE TENSION CONTROL METHOD: BY ROUTINE OBSERVATION ENSURE THAT THE SPLINED ENDS ARE PROPERLY SEVERED DURING INSTALLATION.
   d. FOR COMBINED METHOD (ASTM F3148): BY ROUTINE OBSERVATION OR MATCH MARKING TO VERIFY RELATIVE BOLT AND NUT ROTATION.

PROTECTED ZONES

1. PER AISC 341 SEISMIC PROVISIONS FOR STRUCTURAL STEEL BUILDINGS, SECTION D1.3, SPECIAL (SMF) AND INTERMEDIATE (IMF) MOMENT FRAME SYSTEMS SHALL HAVE DESIGNATED PROTECTED ZONES AS FOLLOWS:
   BEAM
   - THE PROTECTED ZONE STARTS FROM THE VERTICAL EDGE OF SIDE PLATE {A}, AWAY FROM THE COLUMN, AND ENDS ON THE BEAM AS DETAILED. SEE SIDEPLATE TYPICAL DETAILS. SHOT PINS AND PUDDLE WELDS ARE ALLOWED IN THE BEAM PROTECTED ZONE TO SECURE METAL DECKING. THEY SHALL NOT PENETRATE MORE THAN 85% OF THE BEAM FLANGE THICKNESS.

   SIDE PLATE
   - THE PROTECTED ZONE FOR THE SIDE PLATES {A} ARE DEFINED AS A RECTANGULAR AREA AT THE COLUMN/BEAM SEPARATION (GAP) ALONG THE TOP AND BOTTOM EDGES OF THE SIDE PLATES {A}. SEE SIDEPLATE TYPICAL DETAILS. NOTE: WELDS {2} AND {8} ARE EXEMPT FROM THIS PROTECTED ZONE AND SHALL BE PERFORMED FOR THEIR ENTIRE REQUIRED LENGTH.
2. IDENTIFICATION OF PROTECTED ZONES
   a. THE CONTRACTOR SHALL IDENTIFY THE PROTECTED ZONES ON BOTH THE BEAM AND THE SIDE PLATES {A} BY USING ANY SUITABLE NON-DESTRUCTIVE MEANS.
   b. ONCE THE STEEL DECKING IS IN PLACE, THE CONTRACTOR SHALL USE ANY SUITABLE NON-DESTRUCTIVE MEANS TO IDENTIFY THE PROTECTED ZONES PRIOR TO THE INSTALLATION OF SHEAR STUDS, DECK ATTACHMENTS, ETC.
   c. AFTER SPRAYED ON FIRE-RESISTIVE MATERIAL HAS BEEN APPLIED, THE CONTRACTOR SHALL USE ANY SUITABLE NON-DESTRUCTIVE MEANS TO IDENTIFY THE PROTECTED ZONES FOR OTHER DISCIPLINES TO PRECLUDE UNAUTHORIZED ATTACHMENTS.

HOT DIPPED GALVANIZING

1. SIDEPLATE CONNECTIONS REQUIRING THIS TYPE OF FINISH SHALL FOLLOW THE SAME CONSTRUCTION SEQUENCING AS PREVIOUSLY OUTLINED WITH THE FOLLOWING MODIFICATIONS:
   a. AS APPLICABLE, HORIZONTAL SHEAR PLATES {D} SHALL HAVE AN INCREASED CLIP SIZE WHICH SHALL BE 1 5/8 INCH BY 1 5/8 INCH TO PROVIDE ADEQUATE VENTILATION AND DRAINAGE. CONTACT SIDEPLATE SYSTEMS, INC. IN THE EVENT THAT THE GALVANIZING CONTRACTOR SPECIFICATIONS REQUIRE A LARGER OPENING THAN THAT SPECIFIED HEREIN.
   b. SEAL WELDING SHALL NOT BE ALLOWED ON THE SIDE PLATES {A}, COVER PLATES {B}, AND/OR ANGLES {H}; THUS, THEY SHALL HAVE A 1/16 INCH GAP INTRODUCED BETWEEN THEM AND THE OUTBOARD FACE OF THE BEAM FLANGES TO ALLOW THE MOLTEN ZINC TO PENETRATE AND SEAL BETWEEN THEIR FAYING SURFACES.
   c. ANGLE {H} PLACEMENT AND COVER PLATE {B} WIDTH AND HOLE LOCATIONS SHOULD BE ADJUSTED AND DETAILED CORRECTLY IN THE DETAILING OF THE BEAM DUE TO AN INCREASED OUT-TO-OUT DIMENSION OF GAPPED SIDE PLATES {A} ON THE MOMENT FRAME COLUMN.
   d. DEPTH OF SIDE PLATES {A} AND HOLE LOCATIONS SHOULD BE ADJUSTED AND DETAILED CORRECTLY FOR THE INCREASED DIMENSION BETWEEN GAPPED COVER PLATE {B} (WHEN APPLICABLE) AND GAPPED ANGLES {H} ON THE MOMENT FRAME BEAM.
   e. ANY DEVIATIONS TO THESE MODIFICATIONS SHALL BE COORDINATED WITH SIDEPLATE SYSTEMS, INC. AND THE SEOR.
2. IF CONTRACTOR ELECTS TO USE A490 BOLTS AND THE PROJECT SPECIFICATIONS REQUIRE GALVANIZATION, ADDITIONAL REQUIREMENTS SHALL BE APPLIED TO THE A490 MATERIAL. A490 BOLTS SHALL NOT BE HOT DIP GALVANIZED. FOR ASTM A490 BOLTS, THE PROPER CORROSION PROTECTION SHALL BE A ZINC/ALUMINUM INORGANIC COATING THAT IS IN CONFORMANCE WITH ASTM F1136 GRADE 3. EXAMPLES OF SUCH COATINGS, BUT NOT ENDORSED HEREIN, ARE MAGNI® 556 AND GEOMET® 321, OR DACROMET®.
3. STANDARD ASTM A490 BOLTS WITH ADDITIONAL PROCESS FOR COATINGS SHALL REQUIRE EVIDENCE OF COMPLIANCE (CERTIFICATION, LETTER, OR SIMILAR) FROM THE APPLICATOR.

FIREPROOFING

1. WHEN REQUIRED BY THE GOVERNING CODE FOR CERTAIN TYPES OF CONSTRUCTION, SIDEPLATE CONNECTIONS SHALL HAVE A FIRE-RESISTANCE RATING LIKE THAT OF A STEEL "STRUCTURAL FRAME".
2. THE MINIMUM THICKNESS OF SPRAY-APPLIED FIRE-RESISTIVE MATERIAL (SFRM) FOR STEEL SIDEPLATE CONNECTIONS PLATES THAT ARE NOT ENCASED IN CONCRETE, SHALL BE DETERMINED JUST LIKE THAT OF A PIPE/TUBE COLUMN SECTION WITH A CONSTANT STEEL WALL THICKNESS USING THE THICKNESS OF SIDE PLATE {A} FOR EACH SIDEPLATE CONNECTION ID PER THE SIDEPLATE CONNECTION SCHEDULE, WHICH ARE UNIFORMLY HEATED AND PROTECTED (THE FIRE EXPOSURE OF A PIPE/TUBE COLUMN IS DIRECTLY ANALOGOUS TO A PLATE WITH A 1-SIDED FIRE EXPOSURE AND PROTECTION). THE SFRM SHALL HAVE BEEN TESTED IN ACCORDANCE WITH ASTM E119 AND LISTED FOR FIRE RESISTIVE PIPE/TUBE COLUMN APPLICATIONS FOR NO LESS THAN THE REQUIRED RATED TIME.
3. WHEN NO VSE OPTION IS SPECIFIED, PROVIDE A CLOSURE PLATE, AS NEEDED, FOR SFRM MATERIAL TO MORE EASILY ADHERE TO. ATTACH CLOSURE PLATE TO THE INSIDE OR OUTSIDE FACE OF THE SIDE PLATE {A} WITHIN THE MIDDLE HALF OF THE SIDE PLATE {A} HEIGHT. DO NOT ATTACH CLOSURE PLATE TO THE BEAM FLANGES OR WEB.
4. THE CONTRACTOR SHALL PROVIDE THE MEANS, TYPICALLY DONE WITH A LAYERING TECHNIQUE, FOR FIREPROOFING ACROSS THE PHYSICAL COLUMN/BEAM SEPARATION (GAP) BETWEEN THE BOTTOM BEAM FLANGE AND THE FACE OF THE COLUMN FLANGE. IF CLOSURE IS REQUIRED FOLLOW ITEMS 4a AND 4b IN THE SIDEPLATE CONSTRUCTION GUIDELINES.

INTELLECTUAL PROPERTY

1. IN ORDER TO SAFEGUARD THE AUTHORIZED USE AND INTELLECTUAL PROPERTY OF THE PATENTED SIDEPLATE CONNECTION TECHNOLOGY, THE STEEL FABRICATION SUBCONTRACTOR SHALL SATISFY THE FOLLOWING REQUIREMENTS:
   a. A NOTICE OF INTELLECTUAL PROPERTY, IDENTICAL TO THAT PROVIDED ON THIS SHEET, SHALL BE AFFIXED ON EACH SHEET OF SHOP DETAIL AND FIELD ERECTION DRAWINGS CONTAINING SIDEPLATE SYSTEM INFORMATION WHICH DISCLOSES IN ANY WAY THE SIDEPLATE CONNECTION CONCEPT PRIOR TO RELEASING SUCH INFORMATION FOR ITS INTENDED USE. SUCH NOTICE SHALL BE PROVIDED TO THE STEEL FABRICATION SUBCONTRACTOR BY SIDEPLATE SYSTEMS, INC. IN A FORMAT (E.G. WORD OR AUTOCAD) SUITABLE TO THE NEEDS OF THE STEEL FABRICATION SUBCONTRACTOR'S DETAILER.
   b. PATENT LABELS SHALL BE APPLIED IN COMPLIANCE WITH THE GOVERNING PATENT AND INTELLECTUAL PROPERTY LAWS AND SHALL BE PLACED, AS A MINIMUM, IN THE FOLLOWING LOCATIONS:
      1. COLUMN SUB ASSEMBLY:
         - IF PLATE {D} IS REQUIRED, PLACE ONE STICKER ON THE OUTSIDE FACE OF ONE OF THE TWO BOTTOM HORIZONTAL PLATES {D}
         - IF PLATE {D} IS NOT REQUIRED, PLACE ONE STICKER ON THE WEB OF THE COLUMN BEHIND THE SIDE PLATES {A}.
      2. BEAM SUB ASSEMBLY:
         - PLACE ONE STICKER ON ONE END OF THE BEAM BETWEEN THE TOP AND BOTTOM FLANGES WHERE SIDEPLATE CONNECTIONS OCCUR.

INTELLECTUAL PROPERTY RIGHTS NOTICE
The SIDEPLATE® steel frame connection system is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,178,296; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts.
Other U.S. and foreign applications pending.

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GSE EXISTING

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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THINK. LISTEN. CREATE.

SIDEPLATE® POWERED BY MiTek

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.2

SIDEPLATE GENERAL NOTES (2 OF 2)

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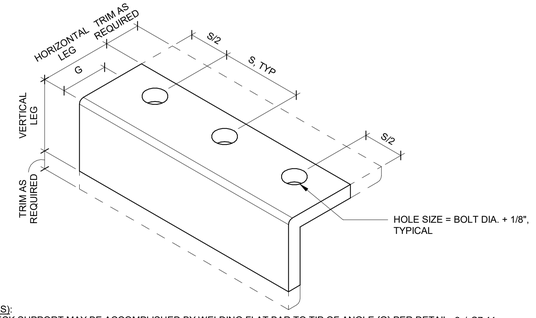
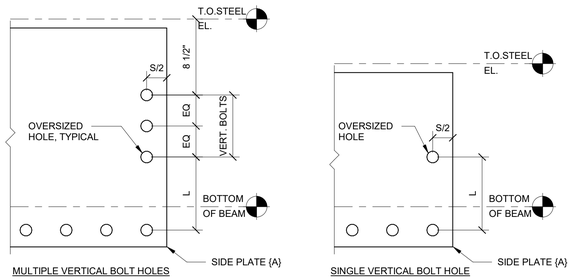
v17.32.08
SMF BOLTED



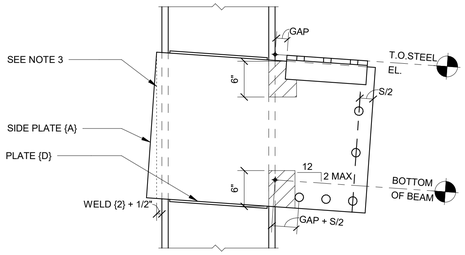
NOTE(S):
1. DECK SUPPORT MAY BE ACCOMPLISHED BY WELDING FLAT BAR TO TIP OF ANGLE {G} PER DETAIL 6 / S7.11 OR BY EXTENDING HORIZONTAL LEG OF ANGLE {G} PER DETAIL 7 / S7.11
2. EDGE SUPPORT MAY BE ACCOMPLISHED BY WELDING FLAT BAR TO TIP OF ANGLE {G} PER DETAIL 5 / S7.11 OR BY EXTENDING HORIZONTAL LEG OF ANGLE {G} PER DETAIL 8 / S7.11
3. SUGGESTED ANGLE {G} SIZE PROVIDED IN SCHEDULE IS BASED ON COMMONLY AVAILABLE SHAPES PER AISC. IF THE ANGLE {G} SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER AVAILABLE ANGLE SECTION.
4. ANY ANGLE OF THE REQUIRED THICKNESS THAT IS CAPABLE OF MEETING THE PROPER MATERIAL SPECIFICATIONS AND THE DIMENSIONS SHOWN ABOVE MAY BE USED FOR ANGLE {G}.
5. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE STEEL FABRICATOR WHAT THE PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA WILL BE.

6 ANGLE {G} DETAIL
N.T.S.



NOTE(S):
1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.
2. TOP ANGLE {G} NOT SHOWN FOR CLARITY.

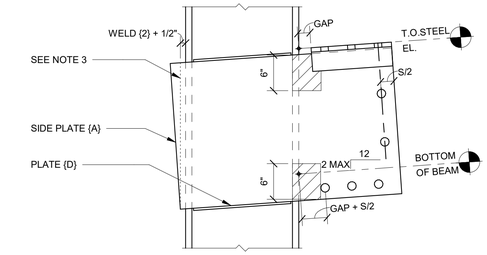
5 SIDE PLATE {A} VSE BOLT HOLE DETAIL (AS APPLICABLE)
N.T.S.



FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. AT CONTRACTOR'S DISCRETION, SIDE PLATE {A} MAY BE CUT AS SHOWN.

4 SLOPED DOWN CONNECTION
N.T.S.



FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. AT CONTRACTOR'S DISCRETION, SIDE PLATE {A} MAY BE CUT AS SHOWN.

3 SLOPED UP CONNECTION
N.T.S.

ID	COLUMN PANEL ZONE DESIGN (INCHES)						SIDE PLATE {A} EXTENSION DESIGN (INCHES)															
	COLUMN	PLATE	WELD				BEAM			PLATE			ANGLE			WELD	BOLT					
	SERIES	{D}	{1}	{2}		{3}	SHAPE	GAP	VSE APPLICABILITY	{A}			{G}			{8}	DIAMETER	HORIZONTAL #	VERTICAL #	G	L	S
		THICKNESS	SIZE	SIZE	HOLD BACK	SIZE				THICKNESS	B	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE						
A10	W14x	3/8	1/4	5/16	1 1/2	1/4	W12X22	1 3/4	Yes	3/4	15 3/4	1 5/8	L6X6X3/4	4 to 6	4	1/4	1 1/8	4	1	2 1/2	8	4 1/2
A20, A21	W14x	3/8	3/8	11/16	2 1/2	3/8	W14X74	3 3/4	Yes	1 3/4	17 5/8	1 1/4	L6X6X3/4	4 to 6	4 to 6	7/16	1 1/8	4	1	2 1/2	8	4 1/2
A40	W30x	3/8	1/4	5/16	1 1/2	1/4	W24X76	2 1/4	Yes	1	27 3/8	1 1/4	L6X6X1	4 to 6	4 to 6	9/16	1 1/8	4	2	2 1/2	8	3 1/2

NOTE(S):
1. FOR VERTICAL BOLT HOLE INFORMATION SEE DETAIL 5 / S7.3
2. AS APPLICABLE, FOR MISCELLANEOUS DETAILS INDICATED BY IDS "M#", REFER TO SIDEPLATE MISCELLANEOUS DETAILS SHEET.

2 A TYPE COLUMN CONNECTION SCHEDULE
N.T.S.

PLAN VIEW

ISOMETRIC

FRONT ELEVATION

SIDE ELEVATION

NOTE(S):
1. PROTECTED ZONE ON SIDE PLATE EXTENDS FROM INSIDE FACE OF COLUMN FLANGE TO EDGE OF BOLT HOLE.
2. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS.
3. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.
4. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.
5. DIMENSION A = GAP+(HORIZONTAL BOLTS)*(S)

1 A TYPE BOLTED CONNECTION
N.T.S.

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.3

SIDEPLATE COLUMN DETAILS, A TYPE

INTELLECTUAL PROPERTY RIGHTS NOTICE
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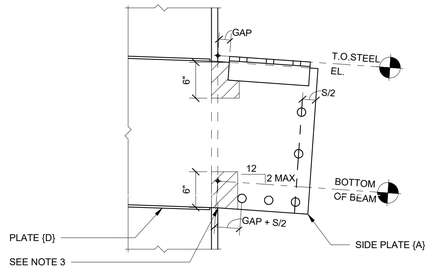
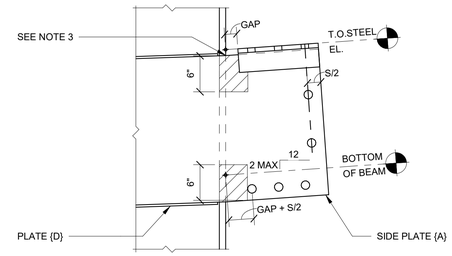
SMF BOLTED



FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NOTE THAT SLOPE OF SIDE PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.

4 SLOPED DOWN CONNECTION
N.T.S.



FRONT ELEVATION

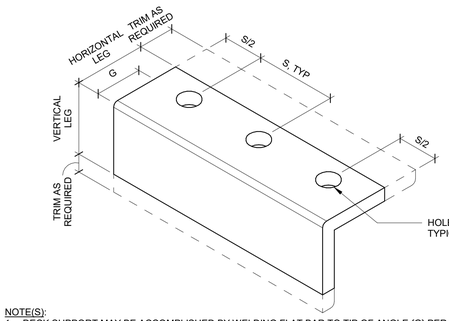
NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. BEGIN SLOPE OF SIDE PLATE AT OUTSIDE FACE OF COLUMN FLANGE, TYPICAL. NOTE THAT SLOPE OF SIDE PLATE WITHIN THE COLUMN EXTENTS MAY NOT MATCH SLOPE OF BEAM.

3 SLOPED UP CONNECTION
N.T.S.

ID	COLUMN PANEL ZONE DESIGN (INCHES)						SIDE PLATE {A} EXTENSION DESIGN (INCHES)														
	COLUMN	PLATE	WELD				BEAM			PLATE			ANGLE			WELD	BOLT					
	SERIES	{D}	{1}	{2}		{3}	SHAPE	GAP	VSE APPLICABILITY	{A}			{G}			{8}	DIAMETER	HORIZONTAL #	VERTICAL #	G	L	S
		THICKNESS	SIZE	SIZE	HOLD BACK	SIZE				THICKNESS	B	Y	SUGGESTED SIZE	HORIZONTAL LEG	VERTICAL LEG	SIZE						
B40	W30x	3/8	1/4	7/16	1 1/2	1/4	W24X76	2 1/4	Yes	1	27 3/8	1 1/4	L6X6X1	4 to 6	4 to 6	9/16	1 1/8	4	2	2 1/2	8	3 1/2

NOTE(S):
1. FOR VERTICAL BOLT HOLE INFORMATION SEE DETAIL 5 / S7.4
2. AS APPLICABLE, FOR MISCELLANEOUS DETAILS INDICATED BY IDS "M#", REFER TO SIDEPLATE MISCELLANEOUS DETAILS SHEET.

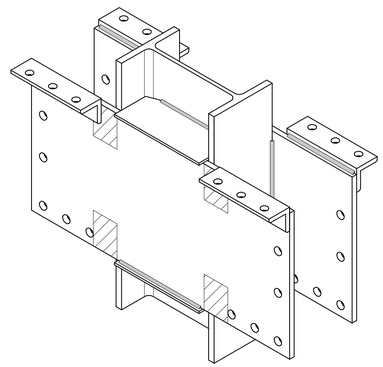
2 B TYPE COLUMN CONNECTION SCHEDULE
N.T.S.



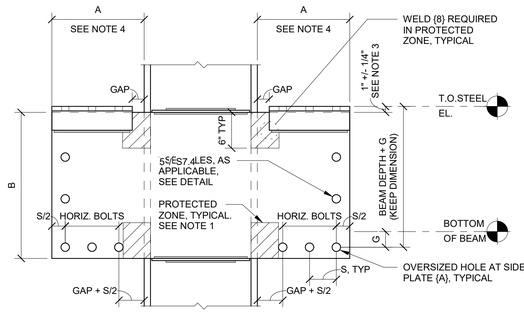
NOTE(S):
1. DECK SUPPORT MAY BE ACCOMPLISHED BY WELDING FLAT BAR TO TIP OF ANGLE {G} PER DETAIL 6 / S7.11 OR BY EXTENDING HORIZONTAL LEG OF ANGLE {G} PER DETAIL 7 / S7.11
2. EDGE SUPPORT MAY BE ACCOMPLISHED BY WELDING FLAT BAR TO TIP OF ANGLE {G} PER DETAIL 5 / S7.11 OR BY EXTENDING HORIZONTAL LEG OF ANGLE {G} PER DETAIL 8 / S7.11
3. SUGGESTED ANGLE {G} SIZE PROVIDED IN SCHEDULE IS BASED ON COMMONLY AVAILABLE SHAPES PER AISC. IF THE ANGLE {G} SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER AVAILABLE ANGLE SECTION.
4. ANY ANGLE OF THE REQUIRED THICKNESS THAT IS CAPABLE OF MEETING THE PROPER MATERIAL SPECIFICATIONS AND THE DIMENSIONS SHOWN ABOVE MAY BE USED FOR ANGLE {G}.
5. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE STEEL FABRICATOR WHAT THE PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA WILL BE.

6 ANGLE {G} DETAIL
N.T.S.

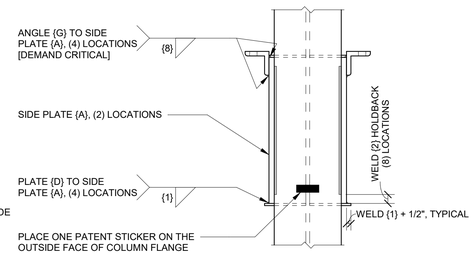
PLAN VIEW



ISOMETRIC



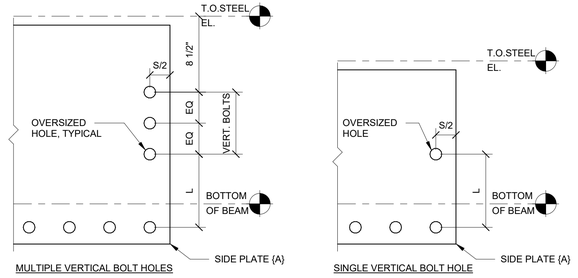
FRONT ELEVATION



SIDE ELEVATION

NOTE(S):
1. PROTECTED ZONE ON SIDE PLATE EXTENDS FROM INSIDE FACE OF COLUMN FLANGE TO EDGE OF BOLT HOLE.
2. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS.
3. THE +/- 1/4 INCH TOLERANCE FOR PLACEMENT OF ANGLES {G} IS TO ENSURE CORRECT TOP OF STEEL PLACEMENT RELATIVE TO THE CENTERLINE OF THE BOTTOM HORIZONTAL ROW OF BOLT HOLES. THE PLACEMENT OF ANGLES {G} SHALL NEVER BE MEASURED FROM THE BOTTOM EDGE OF SIDE PLATE {A} TO ESTABLISH THE CORRECT TOP OF STEEL.
4. DIMENSION A = GAP+(HORIZONTAL BOLTS)*(S)

1 B TYPE BOLTED CONNECTION
N.T.S.



NOTE(S):
1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.
2. TOP ANGLE {G} NOT SHOWN FOR CLARITY.

5 SIDE PLATE {A} VSE BOLT HOLE DETAIL (AS APPLICABLE)
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.4

SIDEPLATE COLUMN DETAILS, B TYPE

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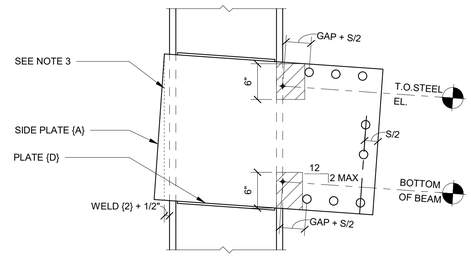
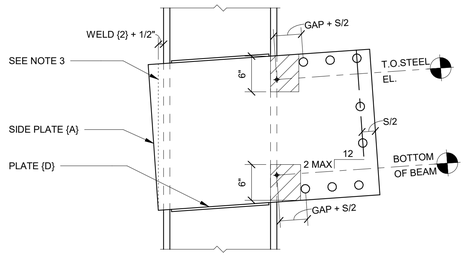


FRONT ELEVATION

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. AT CONTRACTOR'S DISCRETION, SIDE PLATE {A} MAY BE CUT AS SHOWN.

5 SLOPED DOWN CONNECTION
N.T.S.



FRONT ELEVATION

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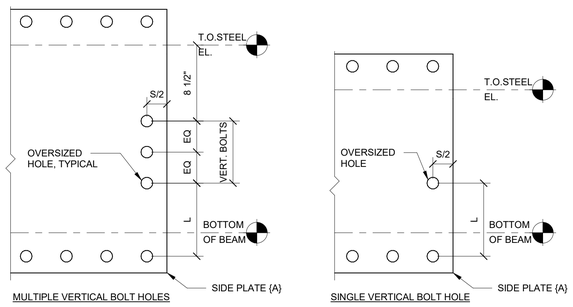
2. COORDINATE PLATES, ANGLES, AND DIMENSIONS WITH RESPECT TO THE SLOPE OF THE CONNECTION.
3. AT CONTRACTOR'S DISCRETION, SIDE PLATE {A} MAY BE CUT AS SHOWN.

4 SLOPED UP CONNECTION
N.T.S.



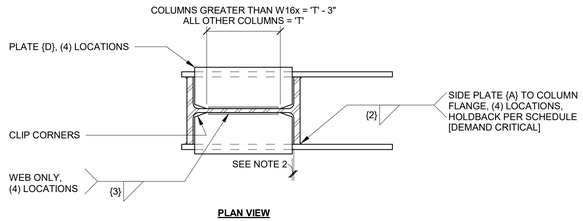
NOTE(S):
1. SEE COLUMN SCHEDULE FOR BOLT QUANTITY.

3 SIDE PLATE {A} VSE BOLT HOLE DETAIL (AS APPLICABLE)
N.T.S.

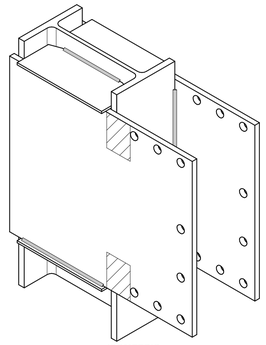
ID	COLUMN PANEL ZONE DESIGN (INCHES)						SIDE PLATE {A} EXTENSION DESIGN (INCHES)											
	COLUMN	PLATE	WELD				BEAM			PLATE				BOLT				
	SERIES	{D}	{1}	{2}		{3}	SHAPE	GAP	VSE APPLICABILITY	{A}				DIAMETER	HORIZONTAL #	VERTICAL #	G	L	S
		THICKNESS	SIZE	SIZE	HOLD BACK	SIZE				THICKNESS	B	E	Y						
A25	W14x	3/8	3/8	9/16	2	3/8	W14X74	2 3/4	Yes	1 1/4	23 5/8	1 5/8	1 1/2	1 1/2	3	1	2 7/8	8	5 3/4

NOTE(S):
1. FOR VERTICAL BOLT HOLE INFORMATION SEE DETAIL 3 / S7.5
2. AS APPLICABLE, FOR MISCELLANEOUS DETAILS INDICATED BY IDS "M#", REFER TO SIDEPLATE MISCELLANEOUS DETAILS SHEET.

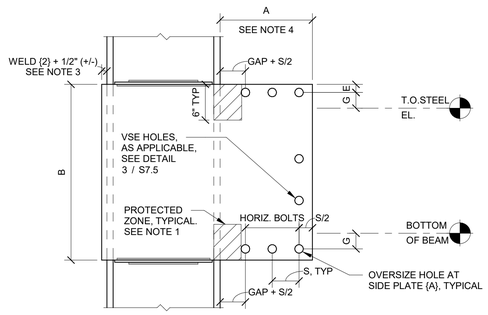
2 A TYPE NARROW COLUMN CONNECTION SCHEDULE
N.T.S.



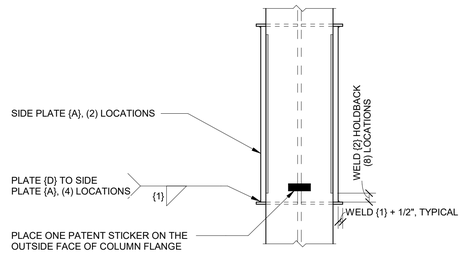
PLAN VIEW



ISOMETRIC



FRONT ELEVATION



SIDE ELEVATION

NOTE(S):
1. PROTECTED ZONE ON SIDE PLATE EXTENDS FROM INSIDE FACE OF COLUMN FLANGE TO EDGE OF BOLT HOLE.
2. DIMENSION BETWEEN PLATE {D} AND INSIDE FACE OF COLUMN FLANGE SHALL NOT EXCEED 1/4 INCH. THIS DIMENSION MAY VARY DEPENDING ON THE DETAILER'S PREFERENCE TO ACCOMMODATE MILL TOLERANCE AND/OR THE UNIFORMITY OF PIECE MARKS.
3. THE 1/2 INCH OVERHANG ON THE SIDE PLATE {A} IS TO ENSURE SUFFICIENT ROOM FOR WELD {2}, THE +/- TOLERANCE IS APPLIED SO THAT IF DESIRED, THE DETAILER CAN MAKE THE SIDE PLATES {A} THE SAME LENGTH WITH SLIGHTLY VARYING COLUMN DEPTHS WITHIN A GROUP OF THE SAME CONNECTION ID'S.
4. DIMENSION A = GAP+(HORIZONTAL BOLTS)*(S)

1 A TYPE NARROW BOLTED CONNECTION
N.T.S.

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
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DATE: 02.14.2025
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CONTRACT NO.: -

SHEET NO.: S7.5

SIDEPLATE COLUMN DETAILS, A TYPE NARROW

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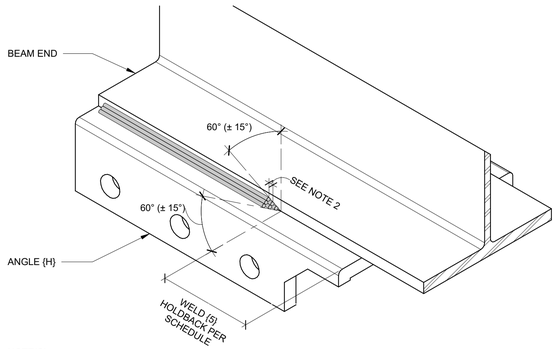
v17.32.08
SMF BOLTED

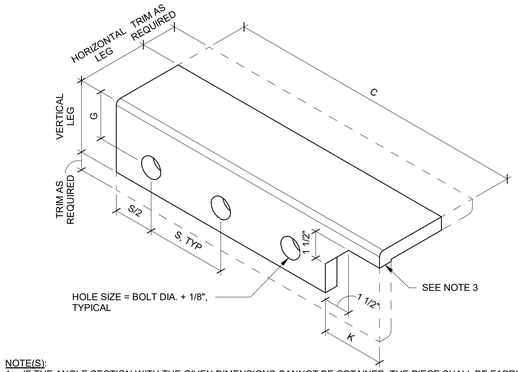
ID	BEAM DESIGN (INCHES)																									
	BEAM			PLATE						ANGLE						WELD					BOLT					
				{B}					{C}	{H}					{E}	{4}	{5}		{5a}							
	SHAPE	GAP	VSE APPLICABILITY	THICKNESS	D	E	R	H	THICKNESS	SUGGESTED SIZE	C	K	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	HOLD BACK	SIZE	HOLD BACK	DIAMETER	HORIZONTAL #	VERTICAL #	G	J	L	S
A10	W12X22	1 3/4	Yes	3/4	15	1 1/2	1	9 1/2	3/8	L7X4X3/4	20	2	6 3/4	4	L4X4X5/8	1/4	1/4	4	1/4	2	1 1/8	4	1	2 1/2	15 3/4	8	4 1/2
A20, A21	W14X74	3 3/4	Yes	3/4	8 1/2	1 1/2	2	11 1/2	3/8	L6X4X3/4	20 1/2	2 1/2	6	4	L4X4X5/8	1/4	1/2	4 1/2	1/2	2 1/2	1 1/8	4	1	2 1/2	15 3/4	8	4 1/2
A40, B40	W24X76	2 1/4	Yes	1 1/4	10	1 1/2	2	10	3/8	L6X6X1	18	4	5	4	L4X4X5/8	1/4	9/16	6	9/16	4	1 1/8	4	2	2 1/2	12 1/4	8	3 1/2

4 BEAM END SCHEDULE
N.T.S.



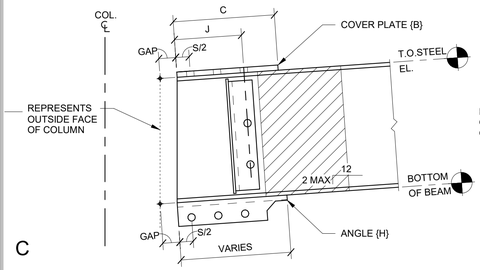
NOTE(S):
1. WELD {5} IS SHOWN FROM THE BOTTOM BEAM FLANGE TO THE BOTTOM ANGLE {H}, BUT IT ALSO OCCURS ON THE TOP BEAM FLANGE WHERE IT CONNECTS TO COVER PLATE {B} OR TOP ANGLE {H}. THERE ARE FOUR TOTAL LOCATIONS PER BEAM END.
2. USING APPROXIMATELY A 1/4 INCH STEP BETWEEN ALL WELD PASSES WILL PROVIDE THE REQUIRED TRANSITION AS DEMONSTRATED THROUGH PHYSICAL TESTING. END PROFILE SHALL BE APPROXIMATELY 60 DEGREES +/- 15 DEGREES.

3 WELD {5} HOLDBACK AND END PROFILE
N.T.S.



NOTE(S):
1. IF THE ANGLE SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER ROLLED ANGLE SECTION.
2. SUGGESTED ANGLE {H} SIZE PROVIDED IN SCHEDULE IS BASED ON COMMONLY AVAILABLE SHAPES PER AISC. IF THE ANGLE {H} SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER AVAILABLE ANGLE SECTION.
3. ANY ANGLE OF THE REQUIRED THICKNESS THAT IS CAPABLE OF MEETING THE PROPER MATERIAL SPECIFICATIONS AND LEG DIMENSION REQUIREMENTS GIVEN IN THE SCHEDULE MAY BE USED FOR ANGLE {H}.
4. MITER CUT NEED NOT BE FLUSH WITH HORIZONTAL LEG. DIMENSION MITER CUT TO UNDERSIDE OF HORIZONTAL LEG SHALL NOT EXCEED 1/2 INCH.

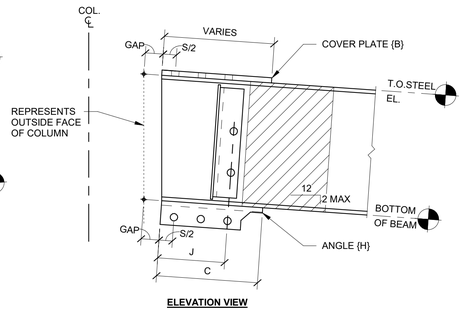
2 ANGLE {H} DETAIL
N.T.S.



ELEVATION VIEW

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

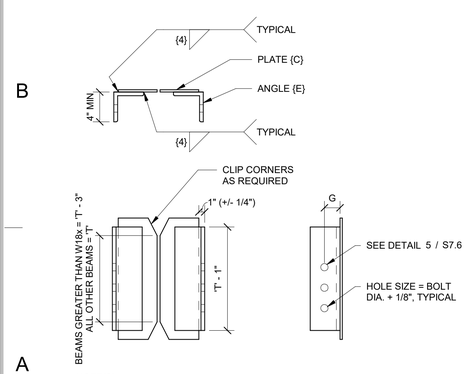
8 SLOPED UP BEAM END
N.T.S.



ELEVATION VIEW

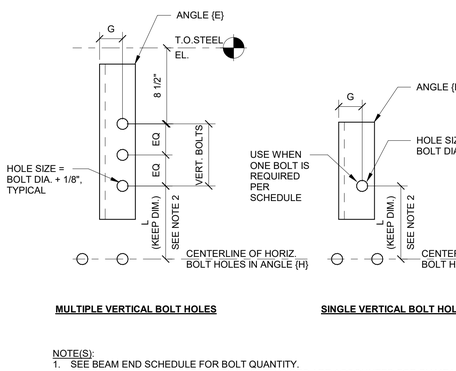
NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

6 SLOPED DOWN BEAM END
N.T.S.



NOTE(S):
1. SEE SIDEPLATE SCHEDULE FOR BOLT QUANTITY

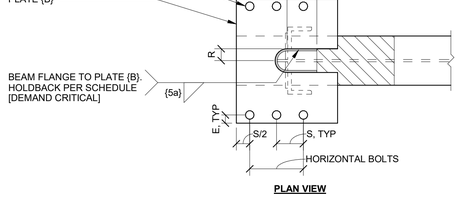
7 VSE {F} DETAIL (AS APPLICABLE)
N.T.S.



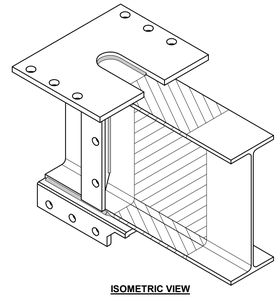
MULTIPLE VERTICAL BOLT HOLES **SINGLE VERTICAL BOLT HOLE**

NOTE(S):
1. SEE BEAM END SCHEDULE FOR BOLT QUANTITY.
2. EFFECTS OF MILL AND FABRICATION TOLERANCES ARE ACCOUNTED FOR BY MEASURING FROM THE CENTERLINE OF THE HORIZONTAL ROW OF BOLTS IN THE BOTTOM ANGLES {H}.

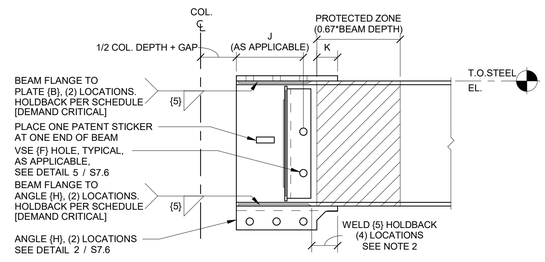
5 VSE {F} HOLES AND PLACEMENT (AS APPLICABLE)
N.T.S.



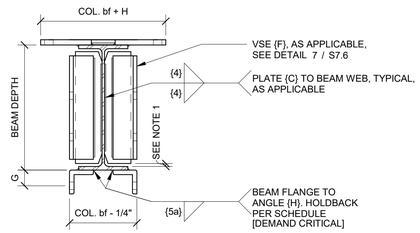
PLAN VIEW



ISOMETRIC VIEW



FRONT ELEVATION



SECTION VIEW

NOTE(S):
1. DIMENSION BETWEEN PLATE {C} AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE {C} SHALL BE CENTERED ON THE DEPTH OF THE BEAM.
2. THIS HOLDBACK AND WELD END PROFILE ARE CRITICAL TO THE PERFORMANCE OF THE CONNECTION. SEE DETAIL 3 / S7.6

1 BEAM END DETAIL
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.6

SIDEPLATE BEAM DETAILS

INTELLECTUAL PROPERTY RIGHTS NOTICE
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Other U.S. and foreign applications pending.

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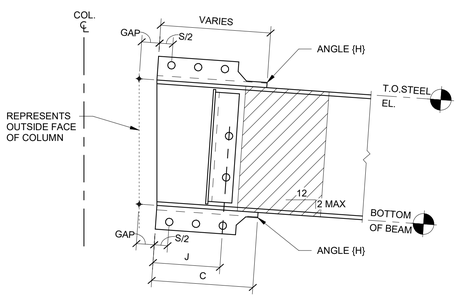
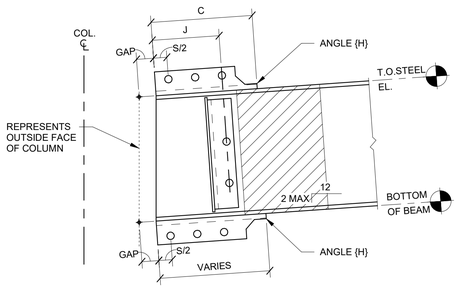
v17.32.08 SMF BOLTED



ELEVATION VIEW

NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

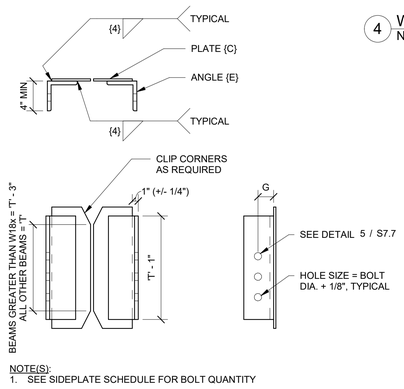
8 SLOPED DOWN BEAM END
N.T.S.



ELEVATION VIEW

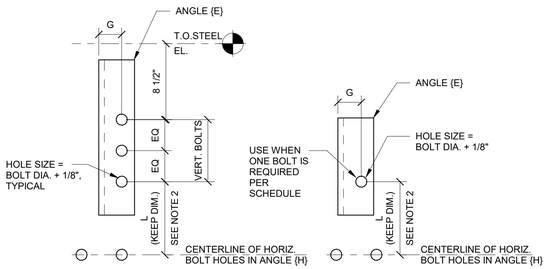
NOTE(S):
1. FOR BEAM SLOPES GREATER THAN 2 INCHES PER FOOT, CONTACT SIDEPLATE SYSTEMS, INC.

7 SLOPED UP BEAM END
N.T.S.



NOTE(S):
1. SEE SIDEPLATE SCHEDULE FOR BOLT QUANTITY

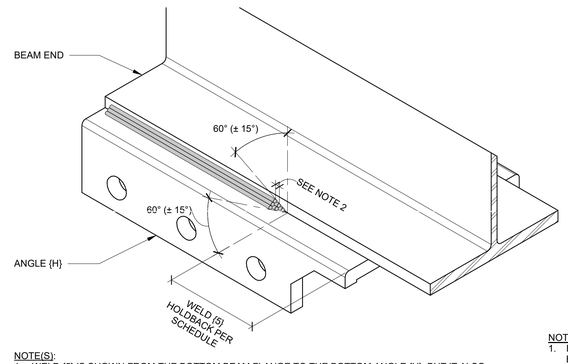
6 VSE {F} DETAIL (AS APPLICABLE)
N.T.S.



MULTIPLE VERTICAL BOLT HOLES **SINGLE VERTICAL BOLT HOLE**

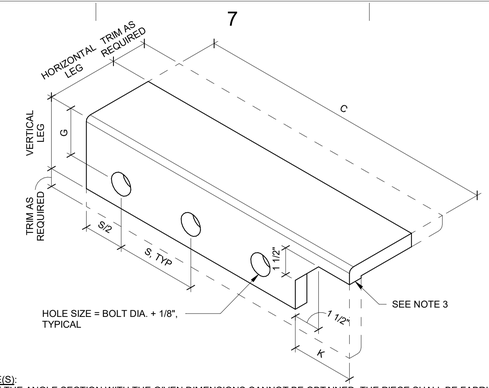
NOTE(S):
1. SEE BEAM END SCHEDULE FOR BOLT QUANTITY.
2. EFFECTS OF MILL AND FABRICATION TOLERANCES ARE ACCOUNTED FOR BY MEASURING FROM THE CENTERLINE OF THE HORIZONTAL ROW OF BOLTS IN THE BOTTOM ANGLES {H}.

5 VSE {F} HOLE DETAIL (AS APPLICABLE)
N.T.S.



NOTE(S):
1. WELD {5} IS SHOWN FROM THE BOTTOM BEAM FLANGE TO THE BOTTOM ANGLE {H}, BUT IT ALSO OCCURS ON THE TOP BEAM FLANGE WHERE IT CONNECTS TO COVER PLATE {B} OR TOP ANGLE {H}. THERE ARE FOUR TOTAL LOCATIONS PER BEAM END.
2. USING APPROXIMATELY A 1/4 INCH STEP BETWEEN ALL WELD PASSES WILL PROVIDE THE REQUIRED TRANSITION AS DEMONSTRATED THROUGH PHYSICAL TESTING. END PROFILE SHALL BE APPROXIMATELY 60 DEGREES +/- 15 DEGREES.

4 WELD {5} HOLDBACK AND END PROFILE
N.T.S.

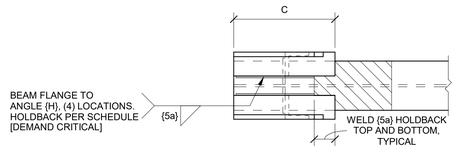


NOTE(S):
1. IF THE ANGLE SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER ROLLED ANGLE SECTION.
2. SUGGESTED ANGLE {H} SIZE PROVIDED IN SCHEDULE IS BASED ON COMMONLY AVAILABLE SHAPES PER AISC. IF THE ANGLE {H} SECTION WITH THE GIVEN DIMENSIONS CANNOT BE OBTAINED, THE PIECE SHALL BE FABRICATED BY TRIMMING ONE OR BOTH LEGS OF A LARGER AVAILABLE ANGLE SECTION.
3. ANY ANGLE OF THE REQUIRED THICKNESS THAT IS CAPABLE OF MEETING THE PROPER MATERIAL SPECIFICATIONS AND LEG DIMENSION REQUIREMENTS GIVEN IN THE SCHEDULE MAY BE USED FOR ANGLE {H}.
4. MITER CUT NEED NOT BE FLUSH WITH HORIZONTAL LEG. DIMENSION MITER CUT TO UNDERSIDE OF HORIZONTAL LEG SHALL NOT EXCEED 1/2 INCH.

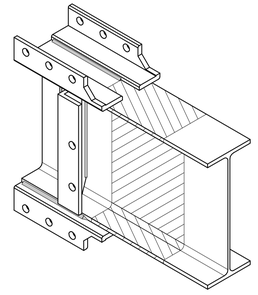
3 ANGLE {H} DETAIL
N.T.S.

ID	BEAM DESIGN (INCHES)																				
	BEAM			PLATE	ANGLE						WELD					BOLT					
	SHAPE	GAP	VSE APPLICABILITY	{C}	{H}					{E}	{4}	{5}		{5a}		DIAMETER	HORIZONTAL #	VERTICAL #	G	J	L	S
				THICKNESS	SUGGESTED SIZE	C	K	HORIZONTAL LEG	VERTICAL LEG	SIZE	SIZE	SIZE	HOLD BACK	SIZE	HOLD BACK							
A25	W14X74	2 3/4	Yes	1/2	L6X6X1	19 3/4	2 1/2	6	4 1/2	L6X6X3/4	5/16	9/16	4 1/2	9/16	2 1/2	1 1/2	3	1	2 7/8	14 3/8	8	5 3/4

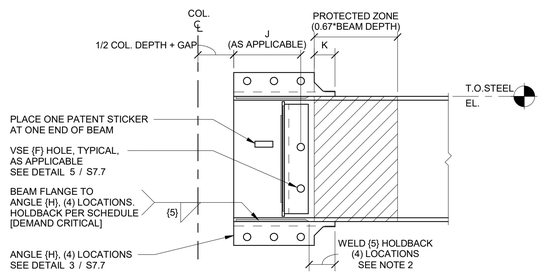
2 NARROW BEAM END SCHEDULE
N.T.S.



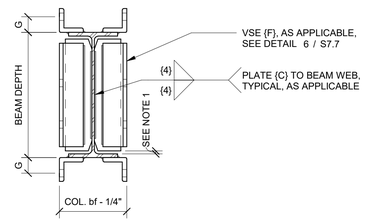
PLAN VIEW



ISOMETRIC VIEW



FRONT ELEVATION



SECTION VIEW

NOTE(S):
1. DIMENSION BETWEEN PLATE {C} AND INSIDE FACE OF BEAM FLANGE SHALL NOT EXCEED 1/4 INCH, AND MAY VARY DEPENDING ON BEAM MILL TOLERANCES. PLATE {C} SHALL BE CENTERED ON THE DEPTH OF THE BEAM.
2. THIS HOLDBACK AND WELD END PROFILE ARE CRITICAL TO THE PERFORMANCE OF THE CONNECTION. SEE DETAIL 4 / S7.7

1 NARROW BEAM END DETAIL
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.7

SIDEPLATE BEAM DETAILS, NARROW

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Other U.S. and foreign applications pending.

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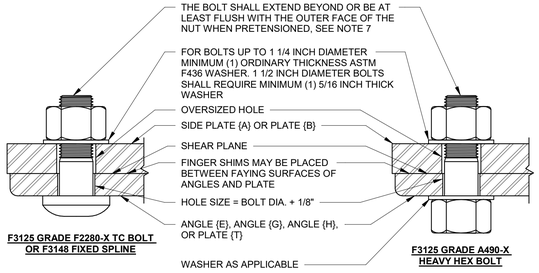
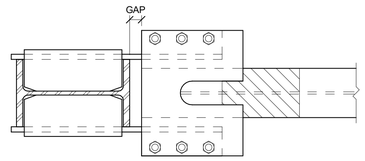
v17.32.08 SMF BOLTED

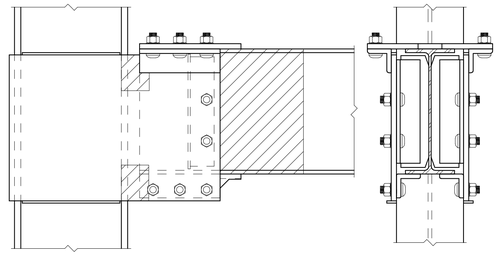


NOTE(S):
1. BOLTS SHALL BE INSTALLED AS SHOWN TO KEEP THREADS OUTSIDE OF SHEAR PLANE.
2. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS. FIRST TO A SNUG TIGHT CONDITION, AND THEN PRETENSIONED.
3. USE FINGER SHIMS FOR GAPS GREATER THAN 1/8 INCH UP TO 1/4 INCH. CONTACT SIDEPLATE SYSTEMS, INC. IF GAPS ARE GREATER THAN 1/4 INCH.
4. NUT SHALL BE ASTM A563.
5. THE BOLT/FASTENER ASSEMBLY SHALL BE COVERED IN A LIGHT PROTECTIVE OIL.
6. FOLLOW QUALITY CONTROL SECTION FOR EXPOSURE LIMITATION ON BOLTS/FASTENERS.
7. STEEL DETAILER TO COORDINATE BOLT LENGTHS WITH REQUIRED WASHERS AND POTENTIAL SHIMMING THICKNESS WITH STEEL FABRICATOR.
8. ALL BOLT HOLES SHALL BE ALIGNED TO PERMIT INSERTION OF THE BOLTS WITHOUT UNDUE DAMAGE TO THE THREADS.
9. THE MINIMUM EDGE DISTANCE FROM THE CENTER OF THE HOLE TO THE EDGE OF THE CONNECTED PART IS PERMITTED TO BE LESS THAN THE MINIMUM EDGE DISTANCE PRESCRIBED BY AISC TABLE J3.4 FOR EACH BOLT DIAMETER, BUT SHALL NOT BE LESS THAN ONE BOLT DIAMETER.
10. BOLT ORIENTATION IS PERMITTED TO BE FLIPPED IF THE FOLLOWING CONDITIONS ARE MET:
    A. IF A HEAVY HEX BOLT IS USED, AN ADDITIONAL WASHER ON THE OVERSIZED HOLE SIDE IS REQUIRED. VERIFY THREAD ARE EXCLUDED FROM THE SHEAR PLANE.
    B. IF A TC BOLT IS USED, NO ADDITIONAL WASHER IS REQUIRED. VERIFY THREADS ARE EXCLUDED FROM THE SHEAR PLANE.
11. WHEN USING DIRECT TENSION INDICATORS (DTI) FOR PRETENSIONING. VERIFY IF ADDITIONAL WASHER IS REQUIRED TO ENSURE DTIs CAN WORK EFFECTIVELY WHEN PRETENSIONED.

2 FIELD BOLTING DETAIL
N.T.S.



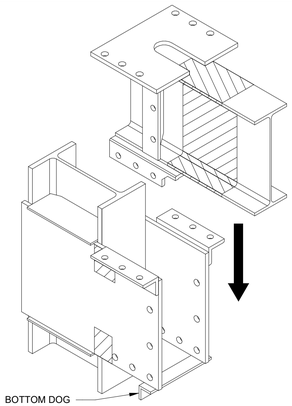
PLAN VIEW



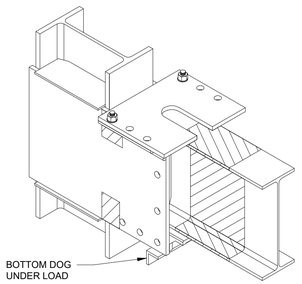
FRONT ELEVATION

SECTION VIEW

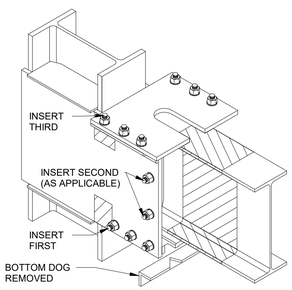
TYPICAL SEQUENCE OF ERECTION:
1. LOWER THE BEAM INTO PLACE FROM ABOVE.
2. INSERT A FEW BOLTS TO SECURE ASSEMBLY.
3. **BOTTOM DOG SHALL BE REMOVED**. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. **IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD!**
4. BOLTS SHALL BE INSERTED INTO HOLES IN THE BEAM COVER PLATE {B} AND THE SIDE PLATES {A}.
5. SYSTEMATICALLY PRE-TENSION BOLTS PER RCSC SPECIFICATIONS.
6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.



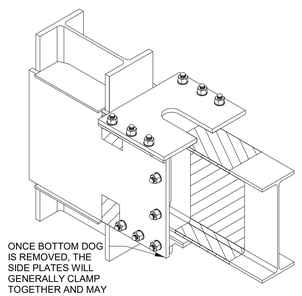
1. LOWER BEAM INTO PLACE



2. STUFF A FEW BOLTS TO SECURE ASSEMBLY



3. CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. THEN INSERT ALL REMAINING BOLTS STARTING WITH THE BOTTOM ROW, THEN THE VERTICAL ROW, AND LAST THE TOP ROW. SNUG TIGHTEN ALL BOLTS.



4. SYSTEMATICALLY PRE-TENSION BOLTS PER SPECIFICATIONS, STARTING WITH THE TIGHTEST PLIES FIRST. (NOTE: COORDINATE LOT TESTING AND INSPECTIONS WITH THIRD PARTY INSPECTION TEAM AS APPROPRIATE)

ID	ERECTION DESIGN (INCHES)				
	BEAM	BOLT			
	SHAPE	DIAMETER	HORIZONTAL #	VERTICAL #	TOTAL # PER BEAM END
A10	W12X22	1 1/8	4	1	18
A20, A21	W14X74	1 1/8	4	1	18
A40, B40	W24X76	1 1/8	4	2	20

3 BEAM ERECTION SCHEDULE
N.T.S.

1 BEAM ERECTION DETAIL
N.T.S.

REVISIONS

NO.	DATE	REMARKS

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website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.8

SIDEPLATE FIELD ERECTION DETAILS (1 OF 2)

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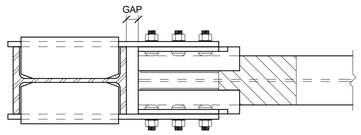
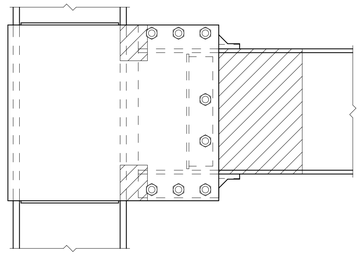
v17.32.08 SMF BOLTED

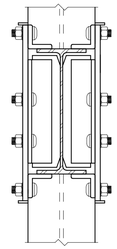
ID	ERECTION DESIGN (INCHES)				
	BEAM	BOLT			
	SHAPE	DIAMETER	HORIZONTAL #	VERTICAL #	TOTAL # PER BEAM END
A25	W14X74	1 1/2	3	1	14



PLAN VIEW



FRONT ELEVATION

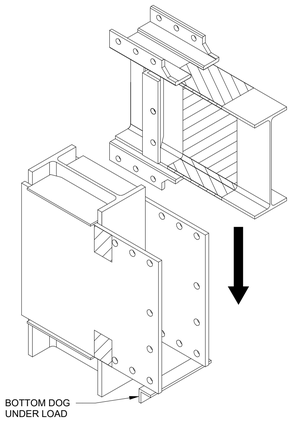


SECTION VIEW

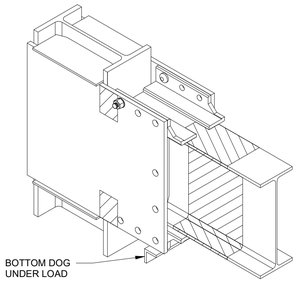
TYPICAL SEQUENCE OF ERECTION:
1. LOWER THE BEAM INTO PLACE FROM ABOVE.
2. INSERT A FEW BOLTS TO SECURE ASSEMBLY.
3. **BOTTOM DOG SHALL BE REMOVED**. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A "V" SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. **IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD!**
4. BOLTS SHALL BE INSERTED INTO HOLES IN THE SIDE PLATES {A}.
5. SYSTEMATICALLY PRE-TENSION BOLTS PER RCSC SPECIFICATIONS.
6. THE WELD REMNANTS OF THE BOTTOM DOG MAY REMAIN IN PLACE AND DO NOT NEED TO BE GROUND SMOOTH.

1 NARROW BEAM ERECTION DETAIL
N.T.S.

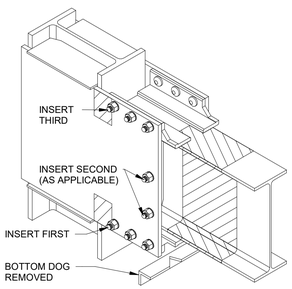
2 NARROW BEAM ERECTION SCHEDULE
N.T.S.



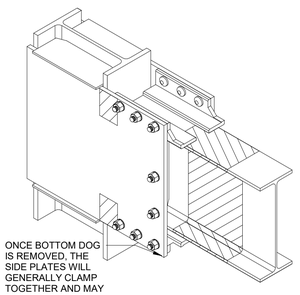
1. **LOWER BEAM INTO PLACE**



2. **STUFF A FEW BOLTS TO SECURE ASSEMBLY**



3. **CAREFULLY REMOVE BOTTOM DOG AS IT IS UNDER LOAD. THEN INSERT ALL REMAINING BOLTS STARTING WITH THE BOTTOM ROW, THEN THE VERTICAL ROW, AND LAST THE TOP ROW.**



4. **SYSTEMATICALLY PRE-TENSION BOLTS PER SPECIFICATIONS, STARTING WITH THE TIGHTEST PLIES FIRST. (NOTE: COORDINATE LOT TESTING AND INSPECTIONS WITH THIRD PARTY INSPECTION TEAM AS APPROPRIATE)**

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SIDEPLATE POWERED BY MiTek

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telephone: 340.777.1600
e-mail: admin@jaredian.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.9

SIDEPLATE FIELD ERECTION DETAILS (2 OF 2)

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Other U.S. and foreign applications pending.

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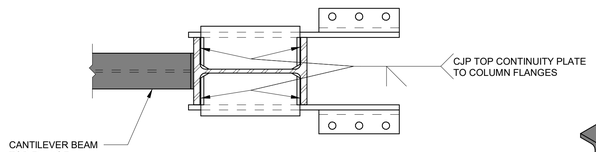
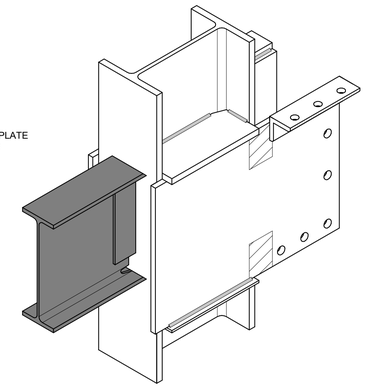
SMF BOLTED

MISCELLANEOUS ID	SEE DETAIL
M1	1/S7.10
M2	3/S7.10

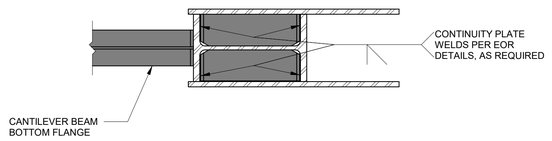
2 MISCELLANEOUS DETAILS SCHEDULE
N.T.S.



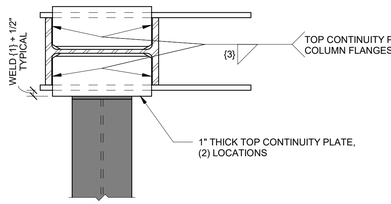
PLAN VIEW



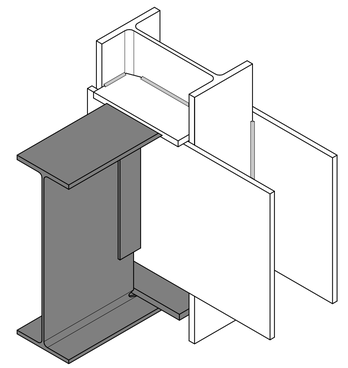
ISOMETRIC



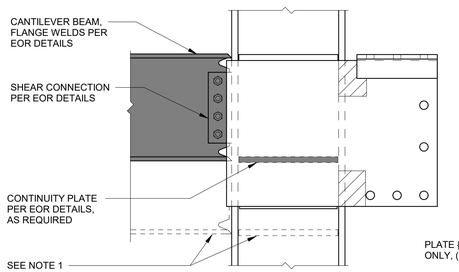
SECTION VIEW LOOKING DOWN



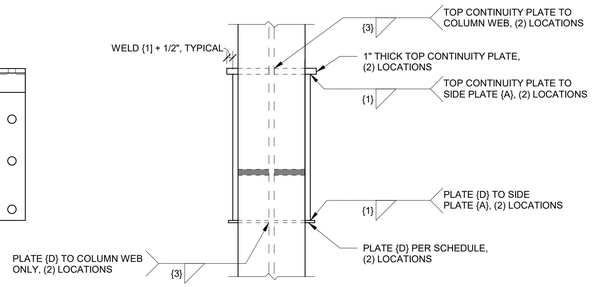
PLAN VIEW



ISOMETRIC



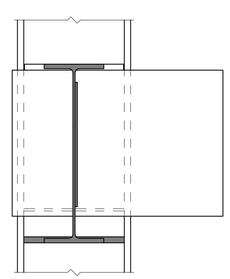
FRONT ELEVATION



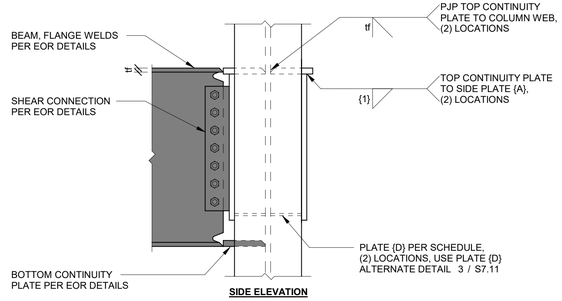
SIDE ELEVATION

NOTE(S):
1. WHERE CANTILEVER BEAM IS DEEPER THAN THE SIDEPLATE CONNECTION, PROVIDE BOTTOM CONTINUITY PLATES AS REQUIRED PER EOR DETAILS IN ADDITION TO PLATE {D}.

3 CANTILEVER TO COLUMN FLANGE AT SIDEPLATE CONNECTION
N.T.S.



FRONT ELEVATION



SIDE ELEVATION

NOTE(S):
1. ATTACHMENT SHOWN ON ONE SIDE OF SIDEPLATE CONNECTION FOR ILLUSTRATION. ATTACHMENT CAN OCCUR ON LEFT SIDE, RIGHT SIDE, OR BOTH SIDES OF CONNECTION AS APPLICABLE.

1 DEEP BEAM WITH TOP FLANGE CONTINUITY PLATE
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

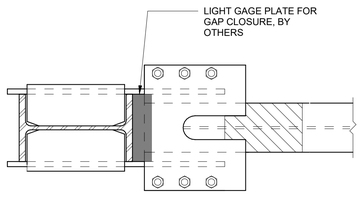
NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

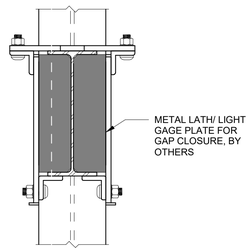
SHEET NO.: S7.10

SIDEPLATE MISCELLANEOUS DETAILS



PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE GAP CLOSURE.
2. SEE GENERAL NOTES FIREPROOFING SECTION FOR MORE DETAILS.



SECTION VIEW

NOTE(S):
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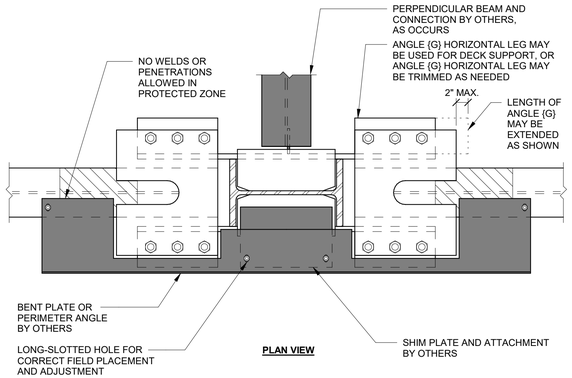
2. SEE GENERAL NOTES FIREPROOFING SECTION FOR MORE DETAILS.

10 (OPTIONAL) GAP CLOSURE DETAIL
N.T.S.

9 (OPTIONAL) NO VSE BEAM GAP CLOSURE DETAIL
N.T.S.



PLAN VIEW

NOTE(S):
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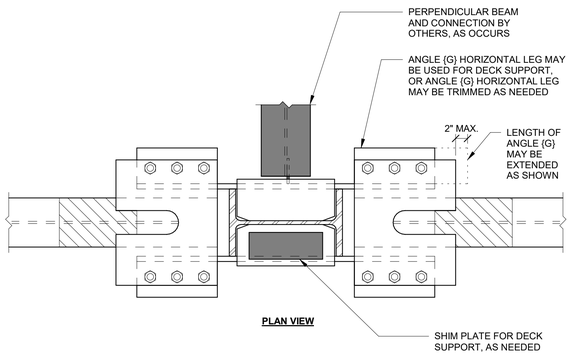
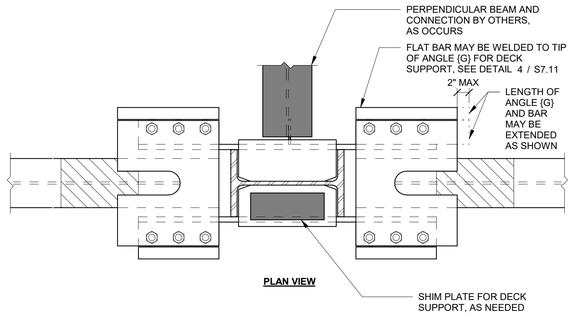
8 (OPTIONAL) ANGLE {G} HORIZONTAL LEG FOR SLAB EDGE SUPPORT DETAIL
N.T.S.



PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

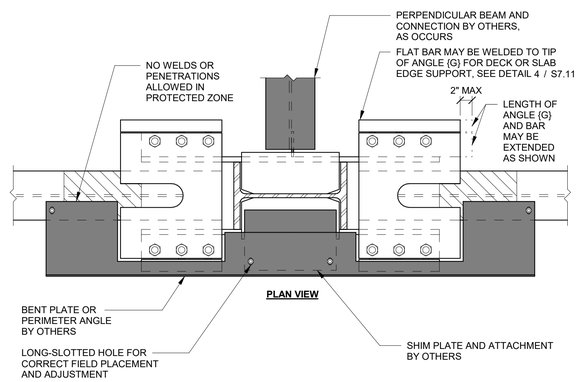
7 (OPTIONAL) ANGLE {G} HORIZONTAL LEG DECK SUPPORT DETAIL
N.T.S.



PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

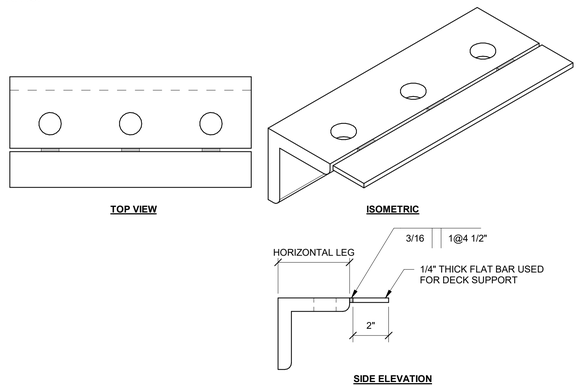
6 (OPTIONAL) WELDED FLAT BAR DECK SUPPORT DETAIL
N.T.S.



PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

5 (OPTIONAL) WELDED FLAT BAR FOR SLAB EDGE SUPPORT DETAIL
N.T.S.



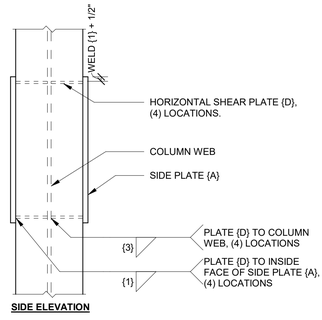
TOP VIEW

ISOMETRIC

SIDE ELEVATION

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

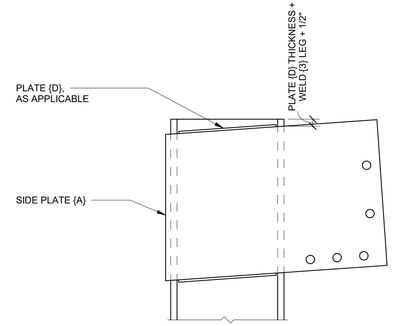
4 (OPTIONAL) WELDED FLAT BAR TO ANGLE {G} FOR DECK SUPPORT
N.T.S.



SIDE ELEVATION

NOTE(S):
1. LONGITUDINAL ANGLES {G} NOT SHOWN FOR CLARITY.
2. SEE SCHEDULE FOR INFORMATION NOT SHOWN.

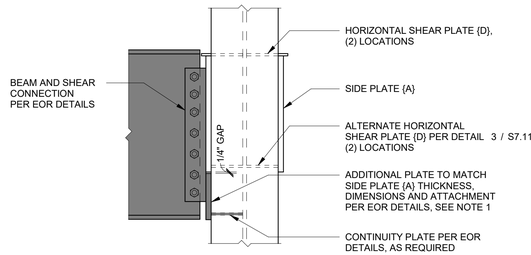
3 PLATE {D} ALTERNATE DETAIL
N.T.S.



FRONT ELEVATION

NOTE(S):
1. SLOPED CONDITION SHOWN, CONNECTION MAY BE FLAT.
2. LONGITUDINAL ANGLES {G} NOT SHOWN FOR CLARITY.

2 DISCONTINUOUS COLUMN DETAIL
N.T.S.



SIDE ELEVATION

NOTE(S):
1. PLATE SHALL BE A572 GRADE 50. NO WELD TIE-IN ACROSS 1/4 INCH GAP.
2. LONGITUDINAL ANGLES {G} NOT SHOWN FOR CLARITY.
3. SEE SCHEDULE FOR INFORMATION NOT SHOWN.

1 DEEP SHEAR CONNECTION TO SIDEPLATE CONNECTION (AS APPLICABLE)
N.T.S.

SMF BOLTED

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REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	TC
CHECKED BY:	AK
APPROVED BY:	MM
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CONTRACT NO.:	-
SHEET NO.:	S7.11

SIDEPLATE COORDINATION ITEMS (1 OF 2)

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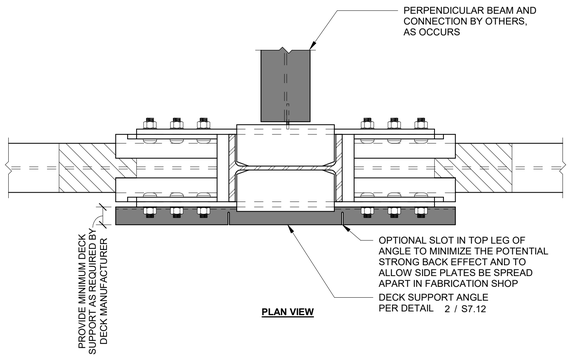
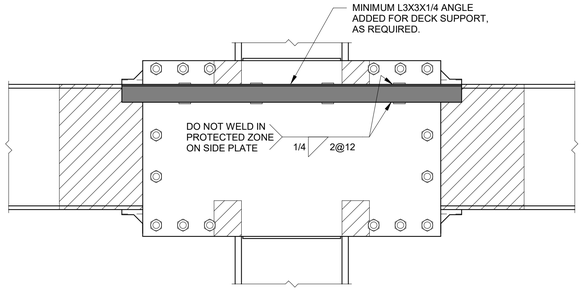


PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

3 (OPTIONAL) NARROW CONFIGURATION DECK SUPPORT DETAIL
N.T.S.



FRONT ELEVATION

NOTE(S):
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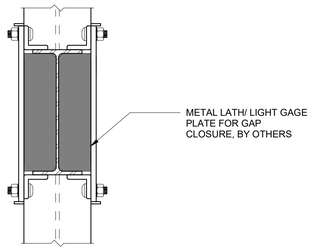

SECTION VIEW

NOTE(S):
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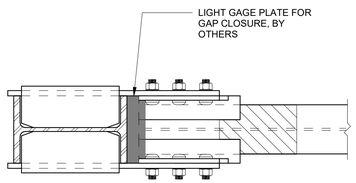
2. SEE GENERAL NOTES FIREPROOFING SECTION FOR MORE DETAILS.

5 (OPTIONAL) NO VSE BEAM GAP CLOSURE DETAIL NARROW
N.T.S.

2 (OPTIONAL) DECK SUPPORT ANGLE DETAIL
N.T.S.



PLAN VIEW

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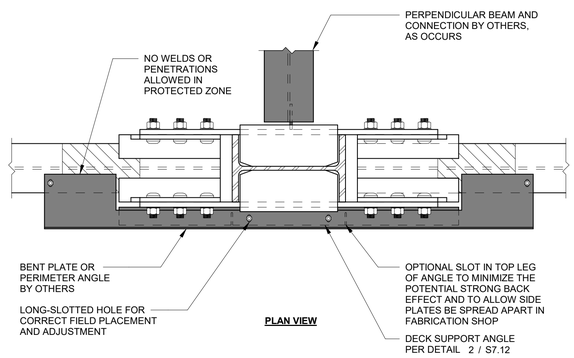
4 (OPTIONAL) GAP CLOSURE DETAIL NARROW
N.T.S.



PLAN VIEW

NOTE(S):
1. THE STEEL DETAILER SHOULD CONFIRM AND COORDINATE WITH THE GENERAL CONTRACTOR AND/OR STEEL FABRICATOR WHICH PREFERRED OPTION OR PROJECT SPECIFIC CRITERIA TO USE FOR THE DECK SUPPORT.

1 (OPTIONAL) NARROW CONFIGURATION SLAB EDGE DETAIL
N.T.S.

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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THINK. LISTEN. CREATE.

SIDEPLATE POWERED BY MiTek

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.12

SIDEPLATE COORDINATION ITEMS (2 OF 2)

SMF BOLTED

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v17.32.08

CONSTRUCTION GUIDELINES

1. THE CONTRACTOR SHALL ASSUME FULL AND COMPLETE RESPONSIBILITY FOR THE MEANS AND METHODS OF CONSTRUCTING THE STEEL FRAME USING THE SIDEPLATE BOLTED SYSTEM. CONSTRUCTION MEANS AND METHODS SHALL BE COMPLIANT WITH THE CURRENT PROVISIONS OF AWS D1.1, AWS D1.8, THE AISC 360 CODE OF STANDARD PRACTICE, THE RCSC HIGH-STRENGTH BOLTING SPECIFICATIONS, AND THE CONSTRUCTION GUIDELINES PROVIDED HEREIN AND SHALL INCLUDE, BUT ARE NOT LIMITED TO:
   a. DIMENSIONAL VERIFICATION AND CONTROL
   b. FILLET WELD END PROFILE AND HOLD BACK DISTANCES FROM EDGE OF PLATE, SEE GRAPHIC NO. 7 AND GRAPHIC NO. 8
   c. STEEL DETAILING AND ERECTION DRAWINGS
   d. FABRICATION AND ERECTION PROCEDURES (INCLUDING METHODS FOR CONTROLLING DISTORTION DUE TO WELD SHRINKAGE, AND FOR CONTROLLING COMBINED MILL, FABRICATION AND ERECTION TOLERANCES)
   e. WELDING PROCEDURES
   f. WELDING SEQUENCE OF FABRICATED AND ERECTED ELEMENTS
   g. QUALITY CONTROL MEASURES
   h. CONSTRUCTION AIDS SUCH AS ERECTION RIGGING AND SHORING
   i. PROPER BOLT HOLE ALIGNMENT
   j. PROPER PRETENSIONING OF BOLTS
   2. THE SEQUENCE OF CONSTRUCTION OPTIONS PROVIDED BELOW IN THESE CONSTRUCTION GUIDELINES HAVE PROVEN TO BE SUCCESSFUL BY STEEL FABRICATORS AND ERECTORS TO COST EFFICIENTLY CONSTRUCT THE BOLTED SIDEPLATE CONNECTION SYSTEM. VARIATIONS TO THESE CONSTRUCTION SEQUENCE OPTIONS PROVIDED BELOW SHALL BE SUBMITTED FOR REVIEW AND DISPOSITION TO SIDEPLATE SYSTEMS, INC.
   3. A PRE-DETAILING MEETING, PRE-FABRICATION MEETING, AND PRE-ERECTION MEETING WITH A SIDEPLATE SYSTEMS, INC REPRESENTATIVE ARE REQUIRED FOR ALL PROJECTS. THE MEETINGS ARE INTENDED TO SHARE BEST PRACTICES AND COMMON MISTAKES TO AVOID.
4. COLUMN/BEAM SEPARATION (GAP) CLOSURE
   a. PROVIDE A FOLDED STRIP OF LIGHT GAGE METAL, OR SIMILAR, SECURED TO STEEL SURFACES BY DUCT TAPE OR A TACK WELD LOCATED AS CLOSE AS PRACTICABLE TO THE MID SECTION OF THE BEAM FLANGES OR TOP COVER PLATE {B} ACROSS THE PHYSICAL COLUMN/BEAM SEPARATION (GAP) BETWEEN THE BEAM FLANGES OR TOP COVER PLATE {B} AND THE FACE OF COLUMN FLANGE. THIS SHALL PREVENT CONCRETE FILL FROM ENTERING THROUGH THE SEPARATION (GAP).
   b. IN NO CASE SHALL THE FOLDED STRIP OF LIGHT GAGE MATERIAL BE WELDED TO THE EDGE OF SIDE PLATE {A}, OR TO THE FACE OF COLUMN FLANGE TO ACHIEVE CLOSURE OF THE PHYSICAL COLUMN/BEAM SEPARATIONS.
5. WHEN NO VSE OPTION IS SPECIFIED, PROVIDE A CLOSURE PLATE, AS NEEDED, FOR SFRM MATERIAL TO MORE EASILY ADHERE TO. ATTACH CLOSURE PLATE TO THE INSIDE OR OUTSIDE FACE OF THE SIDE PLATE {A} WITHIN THE MIDDLE HALF OF THE SIDE PLATE {A} HEIGHT. DO NOT ATTACH CLOSURE PLATE TO THE BEAM FLANGES OR WEB.

SHOP FABRICATION OF THE SIDEPLATE BOLTED SYSTEM

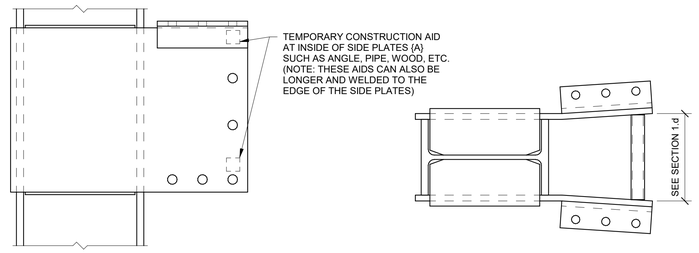
1. COLUMN TREE ASSEMBLY
   a. TACK SIDE PLATES {A} AND HORIZONTAL SHEAR PLATES {D} TO THE COLUMN.
      - QC INSPECTION SHALL PROVIDE VERIFICATION THAT ACTUAL COLUMN FLANGE WIDTH IS AT LEAST NOMINAL COLUMN FLANGE WIDTH WHERE THE SIDE PLATES {A} ARE TO BE INSTALLED. IN THE UNLIKELY EVENT ACTUAL COLUMN FLANGE WIDTH IS LESS THAN NOMINAL, BUT WITHIN AISC STANDARD MILL TOLERANCES (-3/16 INCH), OR THERE IS A FLANGE OFFSET CREATING A CONDITION WHERE THE SIDE PLATES {A} ARE NOT PARALLEL TO ONE ANOTHER, THE FOLLOWING MAY BE APPLIED:
        - PLACE A STRUCTURAL GRADE SHIM (A572 GRADE 50KSI) BETWEEN THE SIDE PLATE {A} AND THE COLUMN FLANGE TO OFFSET THE DEFICIENT DISTANCE. THE CORRESPONDING WELD {2} SHALL THEN BE INCREASED BY THE SHIM THICKNESS.
      - QC INSPECTION SHALL PROVIDE VERIFICATION THAT THE CENTERLINE OF THE BOTTOM ROW OF THE LONGITUDINAL BOLT HOLES ARE SET TO THEIR CORRECT ELEVATION. THIS CENTERLINE MARK WILL BECOME THE WORKING POINT FOR THE CORRECT PLACEMENT OF ANGLE {G}, AS REQUIRED, AND IS ULTIMATELY USED TO SET THE ELEVATION FOR THE TOP OF STEEL IN THE SYSTEM.
   b. WELD HORIZONTAL SHEAR PLATES {D} TO COLUMN WEB USING FILLET WELDS {3} ON OUTER FACE OF PLATE. WHEN WELD {3} OCCURS ON THE COLUMN FLANGES, A WELD TIE-IN OF FILLET WELDS {3} SHALL NOT BE ALLOWED ACROSS THE CLIPPED CORNER OF HORIZONTAL SHEAR PLATE {D} AT THE K-LINE AREA. SEE **PREPARATION** NOTE 4.a IN SIDEPLATE GENERAL NOTES.
   c. WELD HORIZONTAL SHEAR PLATES {D} TO SIDE PLATE {A} USING FILLET WELD {1}.
   d. TEMPORARILY MAINTAIN A SLIGHT (E.G. 1/2 INCH +/-) FLARED-OUT SEPARATION OF SIDE PLATES {A} (CARE SHALL BE TAKEN TO NOT OVER SPREAD IN THIS CONDITION, AS THAT MAY BREAK THE TACKS HOLDING THE SIDE PLATE {A} IN PLACE). IN ANTICIPATION OF WELD {2} SHRINKAGE EFFECTS, PLACE SUITABLE CONSTRUCTION AIDS (SUCH AS ANGLE, PIPES OR WOOD BLOCK) AT THE TOP AND BOTTOM OF SIDE PLATE {A}. AIDS MAY BE CUT TO FIT AND BEAR ON THE INSIDE FACE OF SIDE PLATES {A} OR CUT LONG AND TACKED TO THE EDGES OF SIDE PLATES {A}. SUCH CONSTRUCTION AIDS ACT AS SPACERS DURING THE WELDING AND SUBSEQUENT COOL DOWN OF FILLET WELDS {2}. SEE GRAPHIC NO. 1.
   e. WELD INSIDE FACE OF SIDE PLATES {A} TO THE FLANGE TIPS OF COLUMN USING FILLET WELDS {2}. REFER TO GRAPHIC NO. 2 FOR CONDITIONS WITH NON-SQUARE COLUMN FLANGE TOES DUE TO MILL ROLLING.
      - SHIMS ARE PERMITTED BETWEEN SIDE PLATE {A} AND COLUMN FLANGE IN CONDITIONS DUE TO MILL ROLLING.
      - THE MATERIAL GRADE OF THE SHIM SHALL MATCH THE GRADE USED ON THE SIDE PLATE {A}.
      - FILLET WELD SIZE SHALL BE INCREASED BY THE THICKNESS OF THE SHIM USED.
      - REFER TO GRAPHIC NO. 2A FOR MORE DETAIL.
   f. COLUMN FLANGE TRIMMING IS PERMITTED FOR CONDITIONS DUE TO MILL ROLLING.
      - TRIMMING MAY BE PERFORMED VIA GRINDING TO SMOOTH OUT ROUGH TRANSITIONS.
      - TRIMMING SHALL BE DONE FOR THE FULL LENGTH OF THE SIDE PLATE {A} DEPTH (DIMENSION B) PLUS 12 INCHES ABOVE AND BELOW THE TOP AND BOTTOM EDGES OF SIDE PLATE {A}.
      - IF REPAIR WELDING IS REQUIRED TO FIX GAPS AND GOUGES LARGER THAN 3/16 INCH, IT SHALL BE DONE FOLLOWING AWS PROTOCOLS AND WITH AN APPROVED WPS.
      - THE FINAL SURFACE SHALL HAVE A MINIMUM 500 MICRO-INCH ANSI WORKMANLIKE FINISH.
      - ANY AND ALL TRANSITIONS SHALL BE 10:1.
      - THE FINAL FLANGE WIDTH DIMENSION SHALL BE, AS A MINIMUM, THE NOMINAL DESIGN DIMENSION.
   g. AS REQUIRED, WELD ANGLES {G} TO THE TOP EDGE AND FACE OF SIDE PLATE {A} USING FILLET WELDS {8}. CARE SHALL BE GIVEN TO THE PLACEMENT OF ANGLE {G} AT THE CORRECT ELEVATION RELATIVE TO THE 'WORKING POINT' OF THE CENTERLINE OF THE BOTTOM LONGITUDINAL BOLT HOLES IN SIDE PLATE {A}.
   h. AFTER COOL DOWN OF COMPLETED FILLET WELDS {2}, REMOVE TOP CONSTRUCTION AID. VERIFY MINIMUM CLEAR SPREAD DIMENSION (ACTUAL COLUMN FLANGE WIDTH) BETWEEN SIDE PLATES {A}, SEE GRAPHIC NO. 3. THIS IS THE REQUIRED CLEARANCE FOR INSERTING THE BEAM ASSEMBLY.
      - IF CLEARANCE VERIFICATION IS SUFFICIENT (I.E. NOT LESS THAN THE ACTUAL COLUMN FLANGE WIDTH AND NOT EXCEEDING THE MAXIMUM SPREAD DIMENSION Y), A SUITABLE ERECTION AID (AKA BOTTOM 'DOG') SHALL BE WELDED TO THE BOTTOM EDGE OF THE SIDE PLATES {A}. IT SHALL BE SUFFICIENTLY STRONG TO REMAIN IN PLACE DURING TRANSPORTATION AND ERECTION OF THE BEAM. THIS ERECTION AID WILL BE UNDER LOAD AND CARE SHALL BE TAKEN WHEN REMOVING.
      - IF CLEARANCE VERIFICATION IS NOT SUFFICIENT (I.E. LESS THAN THE NOMINAL COLUMN FLANGE WIDTH), FLEX SIDE PLATES {A} BY JACKING THEM APART TO A DIMENSION GREATER THAN THE WIDTH OF COLUMN FLANGE, MAXIMUM SEPARATION SHALL NOT EXCEED WIDTH OF COLUMN FLANGE PLUS SCHEDULED SPREAD DIMENSION Y. ONCE NECESSARY CLEARANCE HAS BEEN ESTABLISHED, A SUITABLE ERECTION AID (AKA BOTTOM 'DOG') SHALL THEN BE WELDED TO BOTTOM EDGE OF SIDE PLATES {A}. IT SHALL BE SUFFICIENTLY STRONG TO REMAIN IN PLACE DURING TRANSPORTATION AND ERECTION OF BEAM ASSEMBLY. THIS ERECTION AID WILL BE UNDER LOAD AND CARE SHALL BE TAKEN WHEN REMOVING.
      - ALTERNATELY, HEAT STRAIGHTENING OF THE PLATES MAY BE USED TO MEET THE MINIMUM CLEAR DIMENSION SPREAD WITH THE FOLLOWING CRITERIA:
        - USING A 3/4 INCH ROSE BUD HEATING TIP, HEAT THE OUTSIDE FACE OF THE SIDE PLATE ADJUNCT TO THE FREE UNSUPPORTED SIDE OF THE COLUMN FLANGE FOR MAJORITY OF THE FULL DIMENSION B DEPTH (HOLD HEATING BACK FROM TOP AND BOTTOM ENDS OF SIDE PLATE {A} BY 2 INCHES). THE WIDTH OR HEAT ZONE SHALL BE 3/4 INCH TO 1 1/4 INCH. TRAVEL SPEED OF HEATING SHALL BE 2 INCHES PER MINUTE AND MAXIMUM TEMPERATURE INPUT SHALL NOT BE MORE THAN 950 DEGREES.

2. BEAM ASSEMBLY
   a. THE COVER PLATE {B} AND LONGITUDINAL ANGLES {H} SHALL BE PLACED AS PERPENDICULAR AS POSSIBLE TO THE WEB OF BEAM, REGARDLESS OF POSSIBLE FLANGE TILT (AS ROLLED BY THE MILL). SEE GRAPHIC NO. 4.
   b. PLACEMENT OF ANGLES {H} SHALL BE SUCH THAT THEIR BOLT HOLES ARE ALIGNED WITH EACH OTHER AS MUCH AS POSSIBLE IN BOTH THEIR VERTICAL AND HORIZONTAL POSITIONS, ALLOWING THE CENTERLINE OF THE LONGITUDINAL ROWS TO BE THE 'WORKING POINT' FOR THE PLACEMENT OF VSE {F}. THIS WILL PROVIDE ASSURANCE THAT THE LONGITUDINAL BOLT HOLES IN THE BEAM ASSEMBLY WILL LINE UP WITH THE LONGITUDINAL BOLT HOLTS IN THE COLUMN ASSEMBLY. ANY OVER RUN IN THE AS BUILT DIMENSIONS WILL CREATE A GAP BETWEEN ANGLES {G} AND COVER PLATE {B}. SHIM AS REQUIRED. ANY UNDER RUN IN THE AS BUILT DIMENSIONS SHOULD BE ACCOMMODATED BY THE OVERSIZED BOLT HOLE TOLERANCES.
   c. IN THE EVENT THE RADIUS OF THE ANGLE IS LARGER THAN THE EDGE DISTANCE REQUIRED TO PLACE FILLET WELD {5} FROM THE ANGLE {H} TO THE BEAM FLANGE TIPS, ROTATE ASSEMBLY ON ITS SIDE SO THAT THE WEB IS PARALLEL TO THE GROUND. IN THIS POSITION THE RADIUS OF THE ANGLE CAN BE BUILT UP. SEE GRAPHIC NO. 5.
   d. NOTE THAT IN SOME CONDITIONS ANGLES MAY NOT BE 90 DEGREES. CARE SHALL BE GIVEN TO MEASURE THE SUGGESTED WIDTH FROM THE MOST EXTREME POINT ON THE EXTERIOR FACE OF ANGLE {H} (WHICH MAY OCCUR AT THE TOE OF THE ANGLE VERSES ITS APEX).
   e. THE ENTIRE EXTERIOR FACE OF THE OUTSTANDING LEG OF ANGLE {H} SHALL BE PLACED PARALLEL TO THE WEB OF THE BEAM AS MUCH AS PRACTICABLE.
   f. WELD COVER PLATES {B} AND ANGLES {H} TO BEAM FLANGE USING FILLET WELD {5} AND {5a}. SEE GRAPHIC NO. 8.
   g. WELD VSE {F} TO THE BEAM WEB, AS REQUIRED. SEE GRAPHIC NO. 8. IT IS RECOMMENDED THAT THE VSE {F} BE WELDED LAST AS A DISTORTION CONTROL METHOD DUE TO THE POTENTIAL WARPING EFFECTS OF WELD {5}. THE EXTERIOR FACE OF THE VSE {F} SHALL EITHER BE FLUSH WITH THE EXTERIOR FACE OF ANGLE {H}, OR SLIGHTLY RECESSED TOWARDS THE BEAM WEB UP TO 1/16 INCH FROM THE EXTERIOR FACE OF ANGLE {H}, SEE GRAPHIC NO. 4. THE VERTICAL PLACEMENT OF THE VSE {F} SHALL BE SET BY THE KEEP DIMENSION FROM ITS BOTTOM BOLT HOLE TO THE CENTERLINE OF THE BOTTOM LONGITUDINAL ROW OF BOLT HOLES IN ANGLES {H} (I.E. THE 'WORKING POINT' OF THE BEAM). THIS WILL VERIFY PROPER ALIGNMENT OF THE BOLT HOLES IN THE COLUMN ASSEMBLY WITH THOSE IN THE BEAM ASSEMBLY, IRRESPECTIVE OF MILL TOLERANCES.
   h. TACK WELDING IN THE BEAM PROTECTED ZONE SHALL NOT BE PERMITTED. IF THIS SHOULD OCCUR, IT SHALL BE REPAIRED BY FOLLOWING AWS PROTOCOLS.

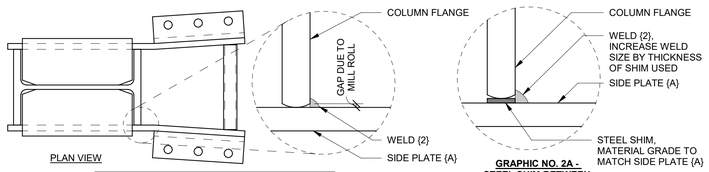
3. THE FOLLOWING FABRICATION AND SEPARATION MEASURES ARE IMPLEMENTED AND MAINTAINED THROUGH DELIVERY TO THE FIELD IN ORDER TO FACILITATE THE LOWERING INTO PLACE OF THE BEAM ASSEMBLY BETWEEN SIDE PLATES {A} OF OPPOSING COLUMN ASSEMBLIES.
   a. MAINTAIN, AS A MINIMUM, THE ACTUAL COLUMN FLANGE WIDTH ANYWHERE IN BETWEEN THE INTERIOR FACES OF SIDES PLATES {A} FROM TOP TO BOTTOM, TO ENSURE SUFFICIENT RATTLE SPACE UPON LOWERING THE BEAM ASSEMBLY INTO PLACE.
   b. THE DETAILED DISTANCE FROM EXTERIOR FACES OF ANGLES {H} ON THE BEAM ASSEMBLY SHALL BE CORRESPONDINGLY DIMENSIONED PER THE TABLE IN GRAPHIC NO. 4. THIS REDUCTION SHALL BE BASED ON THE FABRICATOR'S EXPERIENCE WITH THE TYPICAL MILL OVERRUNS IN THE COLUMN FLANGE WIDTH FOR THE COLUMN SECTIONS BEING USED. THE PROPER DETAILED DIMENSIONING OF THESE ANGLES {H} IS TO FACILITATE ERECTION CLEARANCE DURING THE LOWERING OF THE BEAM ASSEMBLY INTO PLACE.
      - IT IS IMPORTANT TO MAINTAIN SUFFICIENT EDGE DISTANCE BETWEEN THE LONGITUDINAL FACE OF THE BOTTOM ANGLES {H} WITH RESPECT TO ITS APEX AND RADIUS AND THE CORRESPONDING FLANGE TIP OF THE BEAM FOR PLACEMENT OF WELD {5}, SEE GRAPHIC NO. 5.

FIELD ERECTION OF SIDEPLATE BOLTED SYSTEM

1. LOWER THE BEAM ASSEMBLY INTO OPPOSING COLUMN ASSEMBLIES. SEE GRAPHIC NO. 9
   a. RACK BEAM SUCH THAT WEB IS PERPENDICULAR TO THE GROUND, FACILITATING MAXIMUM TOLERANCE WITHOUT THE BEAM BEING TILTED AS IT IS LOWERED DOWN BETWEEN THE SIDE PLATES {A}, SEE GRAPHIC NO. 10.
   b. IF THE OPPOSING COLUMNS ARE SLIGHTLY SKEWED IN RELATION TO ONE ANOTHER, MAKING IT DIFFICULT TO LOWER THE BEAM DOWN BETWEEN THEM SIMULTANEOUSLY, THE BEAM ASSEMBLY CAN BE TILTED AND PLACED FIRST ON ONE COLUMN (LIKELY THE ONE WITH THE GREATEST TWIST). THEN A CHOKER ON THE OPPOSITE END OF THE BEAM CAN BE USED TO PULL IT OVER SO THAT IT IS IN ALIGNMENT ALLOWING THE FREE END OF THE BEAM TO BE DROPPED DOWN INTO PLACE. **IN NO CASE SHALL THE BEAM BE OVERDRAWN OR TWISTED SUCH THAT THE NON-PARALLEL COLUMNS' ERECTION AIDS FAIL.**
2. CAREFULLY REMOVE BOTTOM 'DOG' WHICH IS UNDER LOAD. IT IS RECOMMENDED THAT IT BE REMOVED BY TORCH CUTTING A 'V' SECTION OUT OF ONE OF THE ANGLE LEGS TO ALLEVIATE THE LOAD AND THEN PROCEED TO REMOVE IT. **IT IS NOT RECOMMENDED TO USE A GRINDING WHEEL TO REMOVE THE WELDS WHILE THE DOG IS UNDER LOAD!** BOTTOM 'DOG' ATTACHMENT WELD MAY REMAIN AND IS NOT NECESSARY TO BE GROUND FLUSH, BOTTOM DOGS SHALL BE REMOVED PRIOR TO PRETENSIONING SIDEPLATE BOLTS.
3. INSTALL BOLTS WHICH SERVE AS CLAMPING DEVICES TO 1) PULL SIDE PLATES {A} IN AS CLOSE AS PRACTICABLE TO THE ANGLES {H} OF THE BEAM ASSEMBLY AND 2) PULL ANGLES {G} ON COLUMN TREE ASSEMBLY AS CLOSE AS PRACTICABLE TO COVER PLATE {B}. BOLTS SHALL BE SYSTEMATICALLY INSTALLED AS OUTLINED IN THE BOLTING SPECIFICATIONS, FIRST TO A SNUG TIGHT CONDITION AND THEN PRETENSIONED. PRIOR TO PRETENSIONING, THE DISTANCE BETWEEN FAYING SURFACES SHALL BE MEASURED. IF DISTANCE EQUALS 1/8 INCH OR LESS, NO FURTHER ACTION IS REQUIRED PRIOR TO PRETENSIONING. IF DISTANCE IS GREATER THAN 1/8 INCH UP TO 1/4 INCH, A SINGLE-PLY FINGER SHIM SHALL BE INSTALLED BETWEEN FAYING SURFACES PRIOR TO PRETENSIONING. FOR GAPS GREATER THAN 1/4 INCH, CONTACT SIDEPLATE FOR REMEDIAL MEASURES. WASHER IS REQUIRED UNDER THE NUT (AND IN SOME CASES UNDER THE HEAD OF HEAVY HEX). BOLTS SHALL BE INSTALLED AS SHOWN IN GRAPHIC NO. 11.
4. BOLT IN FIELD AS SHOWN IN GRAPHIC NO. 12 AND GRAPHIC NO. 13 TO COMPLETE ASSEMBLY.



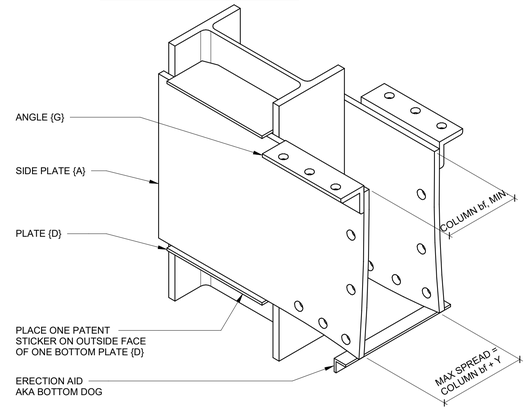
GRAPHIC NO. 1 - COLUMN TREE ASSEMBLY PREPARATION FOR WELD {2}



GRAPHIC NO. 2A - STEEL SHIM BETWEEN COLUMN FLANGE AND SIDE PLATE {A}

- IF MILL ROLL GAP ≤ 1/16", PERFORM FILLET WELD {2} AS SCHEDULED.
- IF MILL ROLL GAP > 1/16", RUN AN INITIAL PASS TO FILL GAP. THEN PERFORM FILLET WELD {2} AS SCHEDULED.

GRAPHIC NO. 2 - MILL ROLL INDUCED GAP



GRAPHIC NO. 3 - COMPLETED SIDEPLATE BOLTED COLUMN TREE ASSEMBLY

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	TC
CHECKED BY:	AK
APPROVED BY:	MM

PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: S7.13

SIDEPLATE CONSTRUCTION GUIDELINES (1 OF 3)

SMF BOLTED

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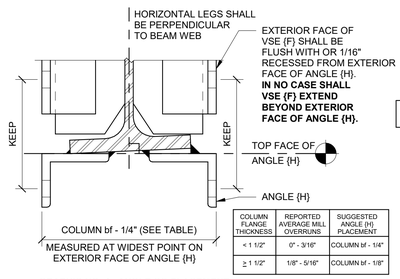
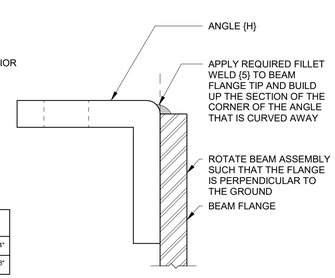
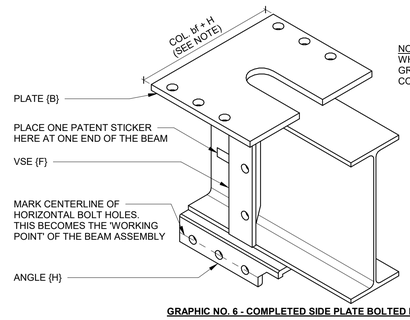


COLUMN FLANGE THICKNESS	REPORTED AVERAGE MILL OVERRUNS	SUGGESTED ANGLE {H} PLACEMENT
< 1 1/2"	0" - 3/16"	COLUMN bf - 1/4"
≥ 1 1/2"	1/8" - 5/16"	COLUMN bf - 1/8"

GRAPHIC NO. 4 - ANGLE {H} PLACEMENT

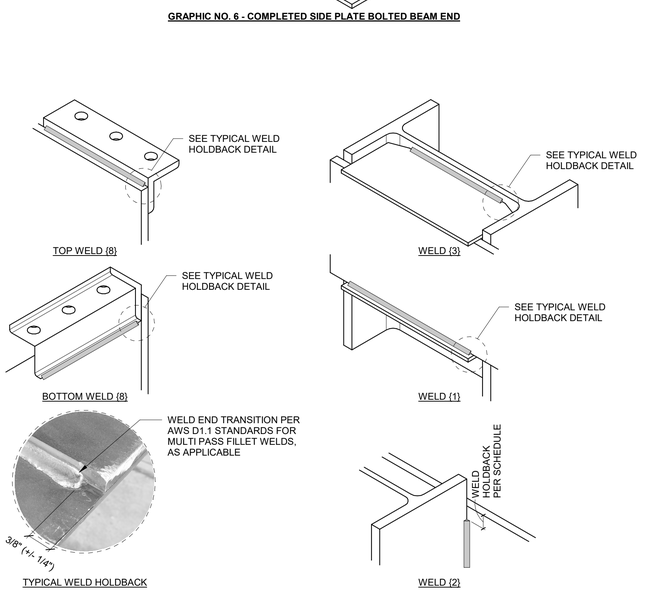


GRAPHIC NO. 5 - ROTATE BEAM ASSEMBLY TO FACILITATE WELD {5} MATERIAL AT MINIMAL EDGE DISTANCE

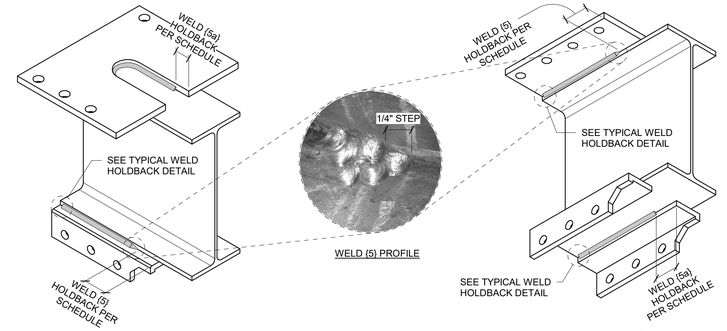


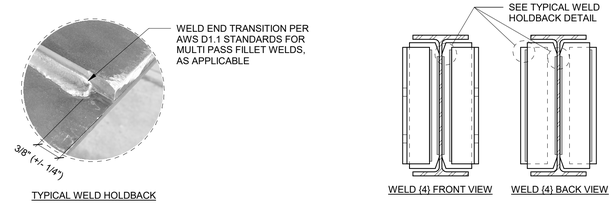
NOTE:
WHEN COLUMN FLANGE THICKNESS IS GREATER THAN 1 1/2 INCHES, INCREASE COVER PLATE WIDTH BY 1/8 INCH.

GRAPHIC NO. 6 - COMPLETED SIDE PLATE BOLTED BEAM END

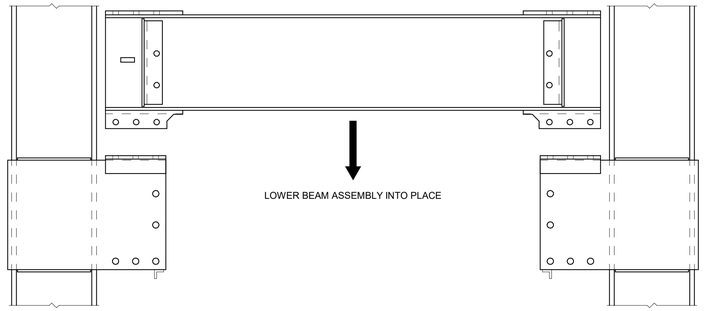


GRAPHIC NO. 7 - COLUMN ASSEMBLY WELD END PROFILES AND HOLDBACKS

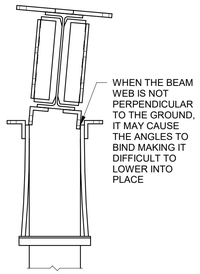




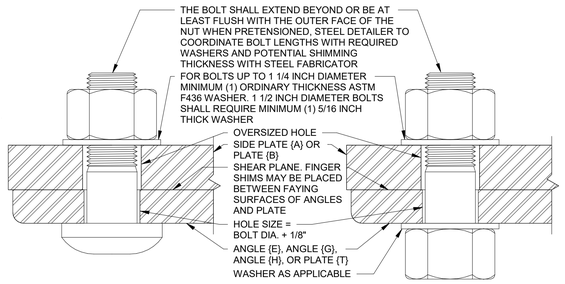
GRAPHIC NO. 8 - BEAM ASSEMBLY WELD END PROFILES AND HOLDBACKS



GRAPHIC NO. 9 - FIELD ERECTION OF SIDEPLATE BEAM ASSEMBLY



GRAPHIC NO. 10 - INCORRECTLY RACKED BEAM



GRAPHIC NO. 11 - BOLTING DETAIL

REVISIONS

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www.tlc-engineers.com
TLC No.:123515
THINK. LISTEN. CREATE.

SIDEPLATE POWERED BY MiTek

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St. Thomas, U.S. Virgin Islands 00804-6218
telephone: 340.777.1600
e-mail: admin@jaredian.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: TC
CHECKED BY: AK
APPROVED BY: MM

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: S7.14

SIDEPLATE CONSTRUCTION GUIDELINES (2 OF 3)

SMF BOLTED

INTELLECTUAL PROPERTY RIGHTS NOTICE
The SIDEPLATE® steel frame connection systems is covered by one or more of U.S. Pat. Nos. 6,138,427; 6,516,583; 6,591,573; 7,176,256; 8,122,671; 8,122,672; 8,146,322; 8,176,706; 8,205,408; and 9,091,065 and foreign counterparts. Other U.S. and foreign applications pending.

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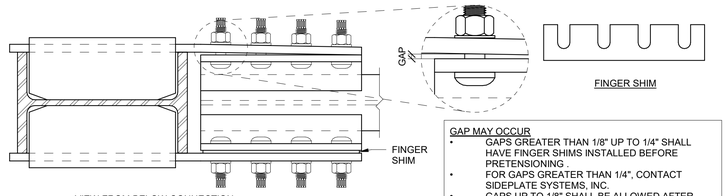
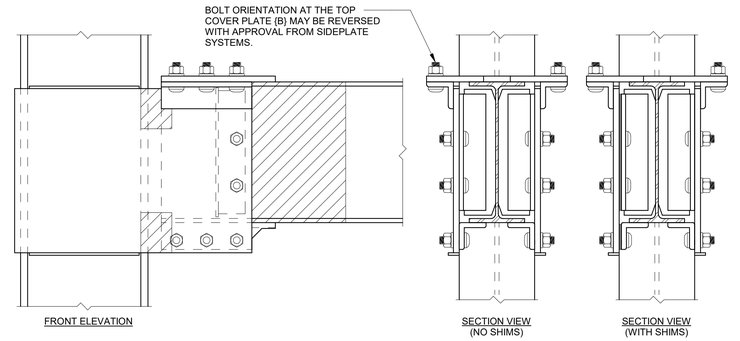
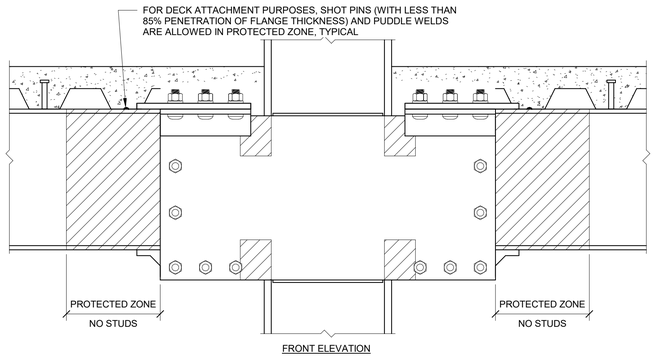


GAP MAY OCCUR
- GAPS GREATER THAN 1/8" UP TO 1/4" SHALL HAVE FINGER SHIMS INSTALLED BEFORE PRETENSIONING
- FOR GAPS GREATER THAN 1/4", CONTACT SIDEPLATE SYSTEMS, INC.
- GAPS UP TO 1/8" SHALL BE ALLOWED AFTER BOLTS HAVE BEEN PRETENSIONED

GRAPHIC NO. 12 - CONDITION PRIOR TO TIGHTENING BOLTS



GRAPHIC NO. 13 - COMPLETED SIDEPLATE BOLTED CONNECTION



GRAPHIC NO. 14 - STUDS AND PUDDLE WELDS AT BEAM PROTECTED ZONE

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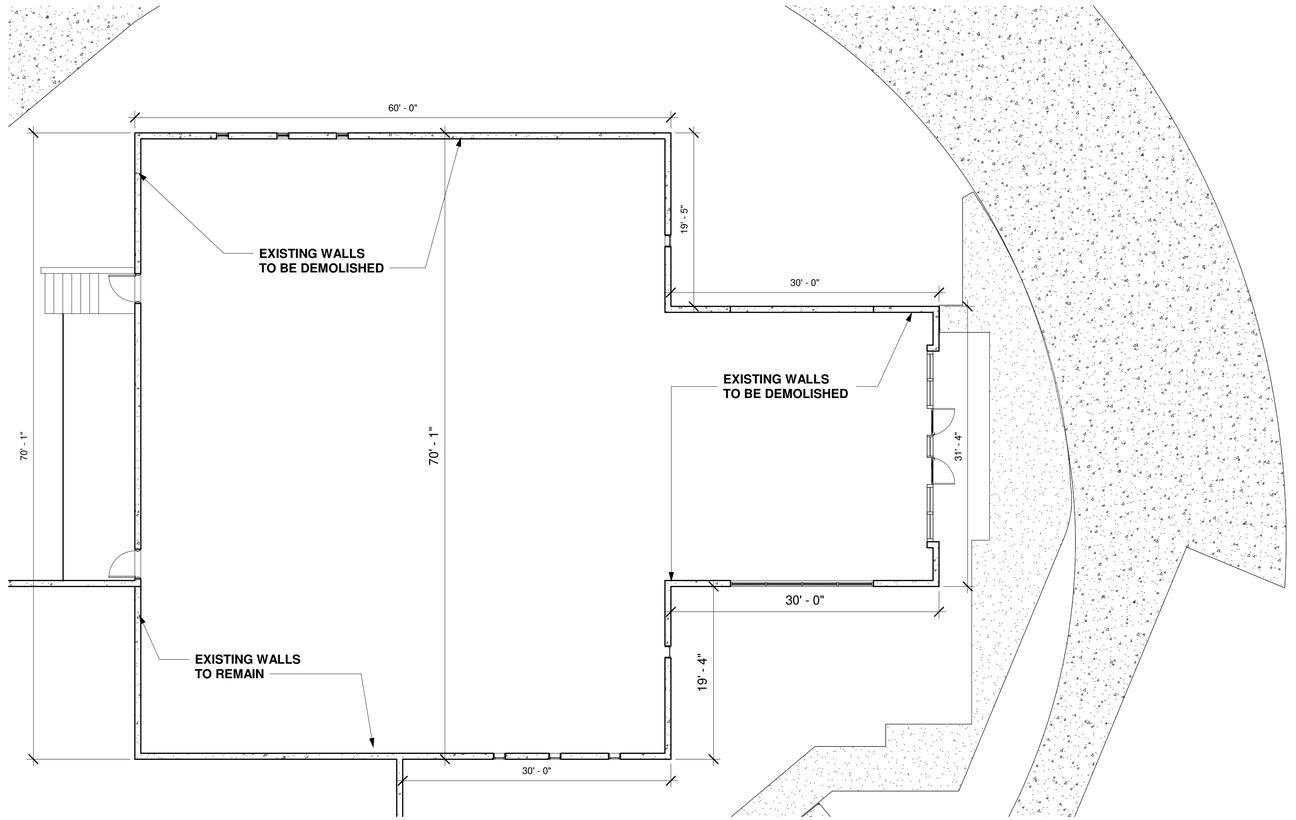
SIDEPLATE CONSTRUCTION GUIDELINES (3 OF 3)



1 EXISTING LEVEL 1

1/8" = 1'-0"

REVISIONS

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ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

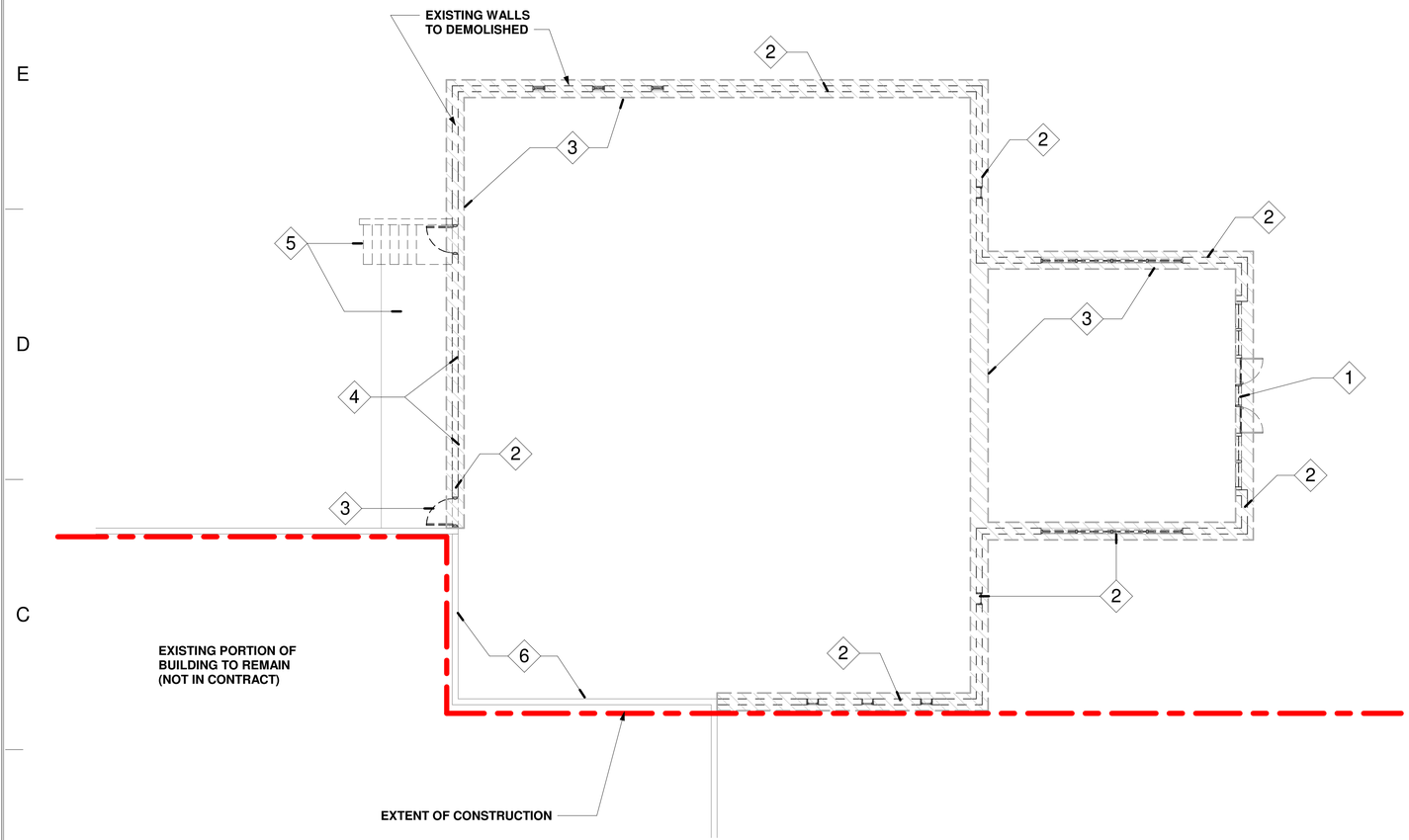
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A0.0

LEVEL 1 EXISTING

LICENSED ARCHITECT U.S. VIRGIN ISLANDS



1 DEMOLITION LEVEL 1
1/8" = 1'-0"

DEMOLITION NOTES:

1. REMOVE EXISTING STOREFRONT ASSEMBLY AND METAL STUD FRAMING. REPLACE WITH NEW.
2. DEMOLISH EXISTING MASONRY WALL AND DISCARD ANY DOOR OR WINDOWS. SEE STRUCTURAL.
3. DEMOLISH EXISTING CONCRETE FOOTING. SEE STRUCTURAL.
4. REMOVE EXISTING OVERHEAD COILING DOOR.INSTALL NEW DOOR PER SCHEDULE IN NEW WALL.
5. REMOVE EXISTING WOODEN STAIR ASSEMBLY. REPLACE WITH NEW CONCRETE STAIR AND LOADING DOCK.
6. EXISTING WALL AND FOUNDATION TO REMAIN. NOT IN CONTRACT.

REVISIONS

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ST. THOMAS, USVI

DATE: 02.14.2025
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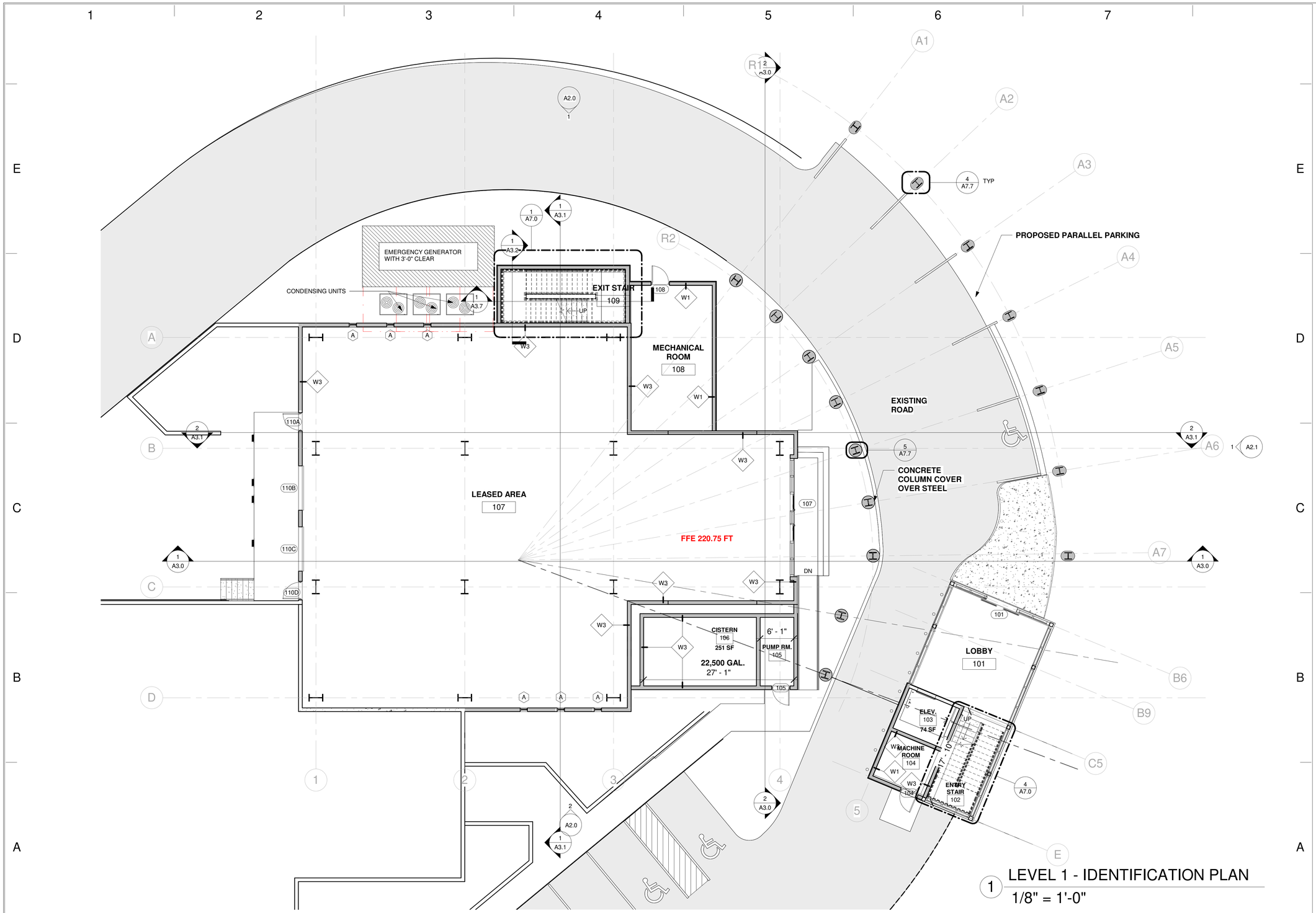
CHECKED BY: J.P.W
APPROVED BY: J.P.W.

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A0.1

LEVEL 1 DEMOLITION FLOOR PLAN

LICENSED ARCHITECT U.S. VIRGIN ISLANDS



1 LEVEL 1 - IDENTIFICATION PLAN
1/8" = 1'-0"

REVISIONS		
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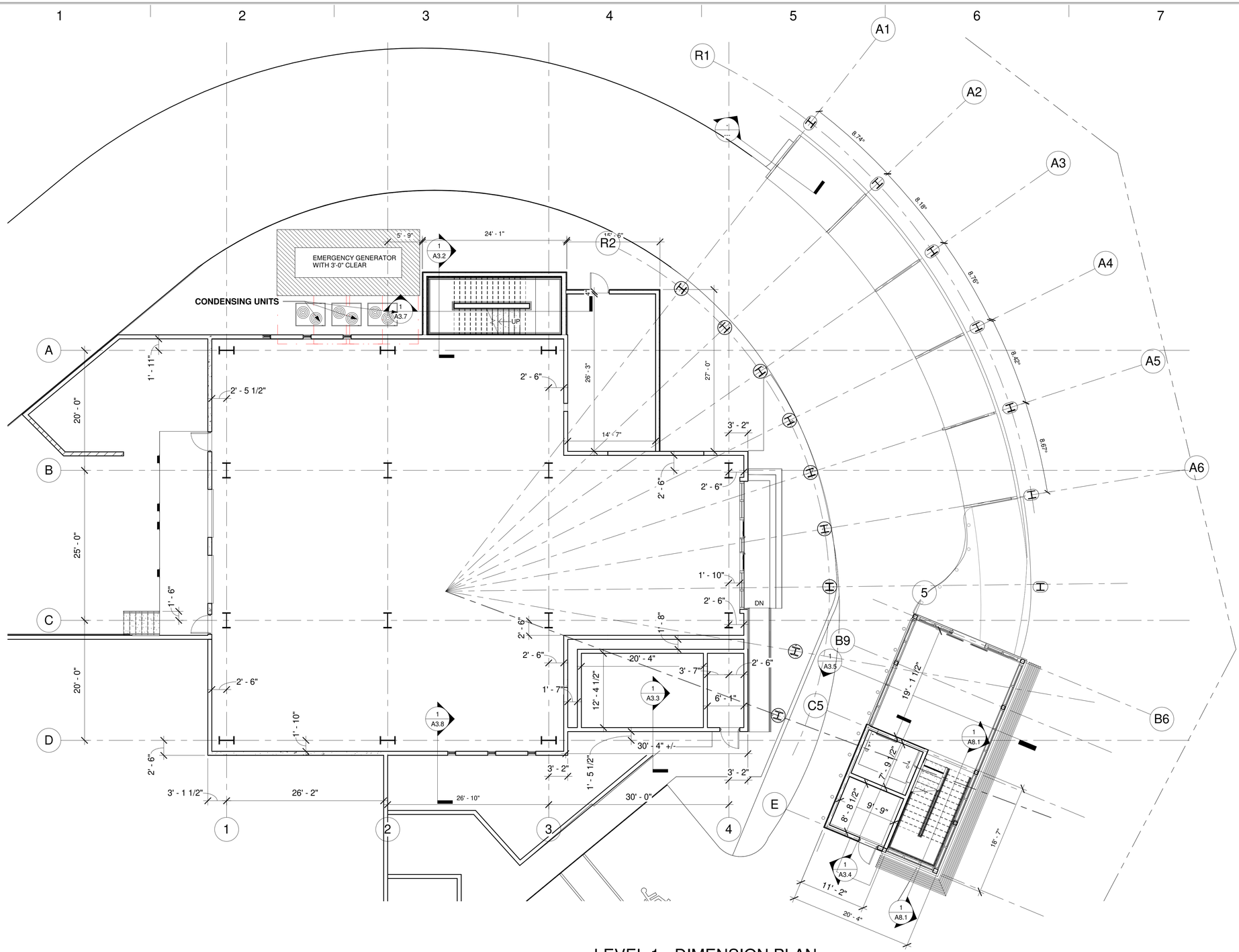
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A1.1

LEVEL 1 IDENTIFICATION PLAN

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



1 LEVEL 1 - DIMENSION PLAN

1/8" = 1'-0"

REVISIONS		
NO.	DATE	REMARKS

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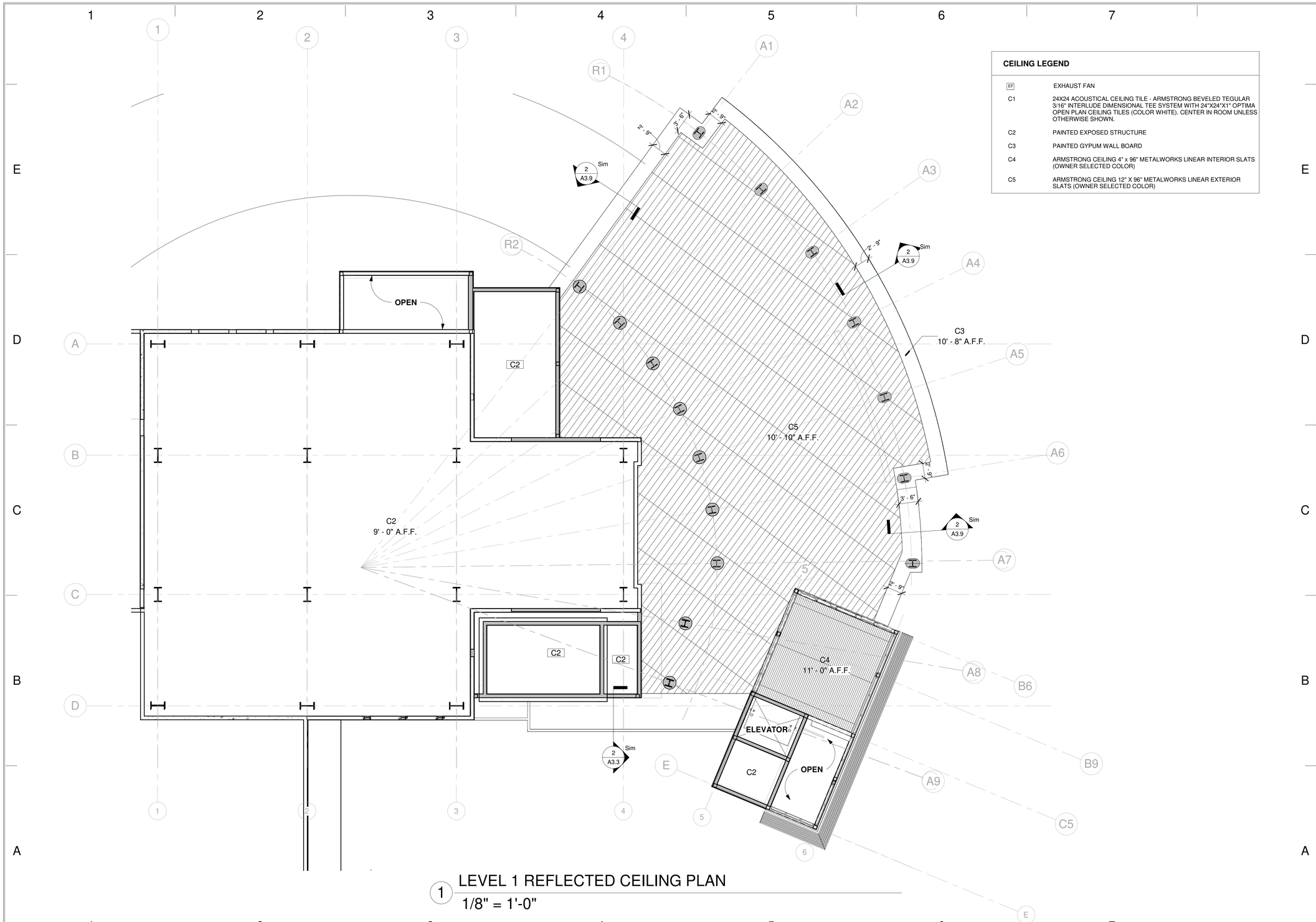
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A1.2

LEVEL 1 DIMENSION PLAN

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



1 LEVEL 1 REFLECTED CEILING PLAN

1/8" = 1'-0"

CEILING LEGEND

EF	EXHAUST FAN
C1	24X24 ACOUSTICAL CEILING TILE - ARMSTRONG BEVELED TEGULAR 3/16" INTERLUDE DIMENSIONAL TEE SYSTEM WITH 24"X24"X1" OPTIMA OPEN PLAN CEILING TILES (COLOR WHITE). CENTER IN ROOM UNLESS OTHERWISE SHOWN.
C2	PAINTED EXPOSED STRUCTURE
C3	PAINTED GYPUM WALL BOARD
C4	ARMSTRONG CEILING 4" x 96" METALWORKS LINEAR INTERIOR SLATS (OWNER SELECTED COLOR)
C5	ARMSTRONG CEILING 12" X 96" METALWORKS LINEAR EXTERIOR SLATS (OWNER SELECTED COLOR)

REVISIONS

NO.	DATE	REMARKS

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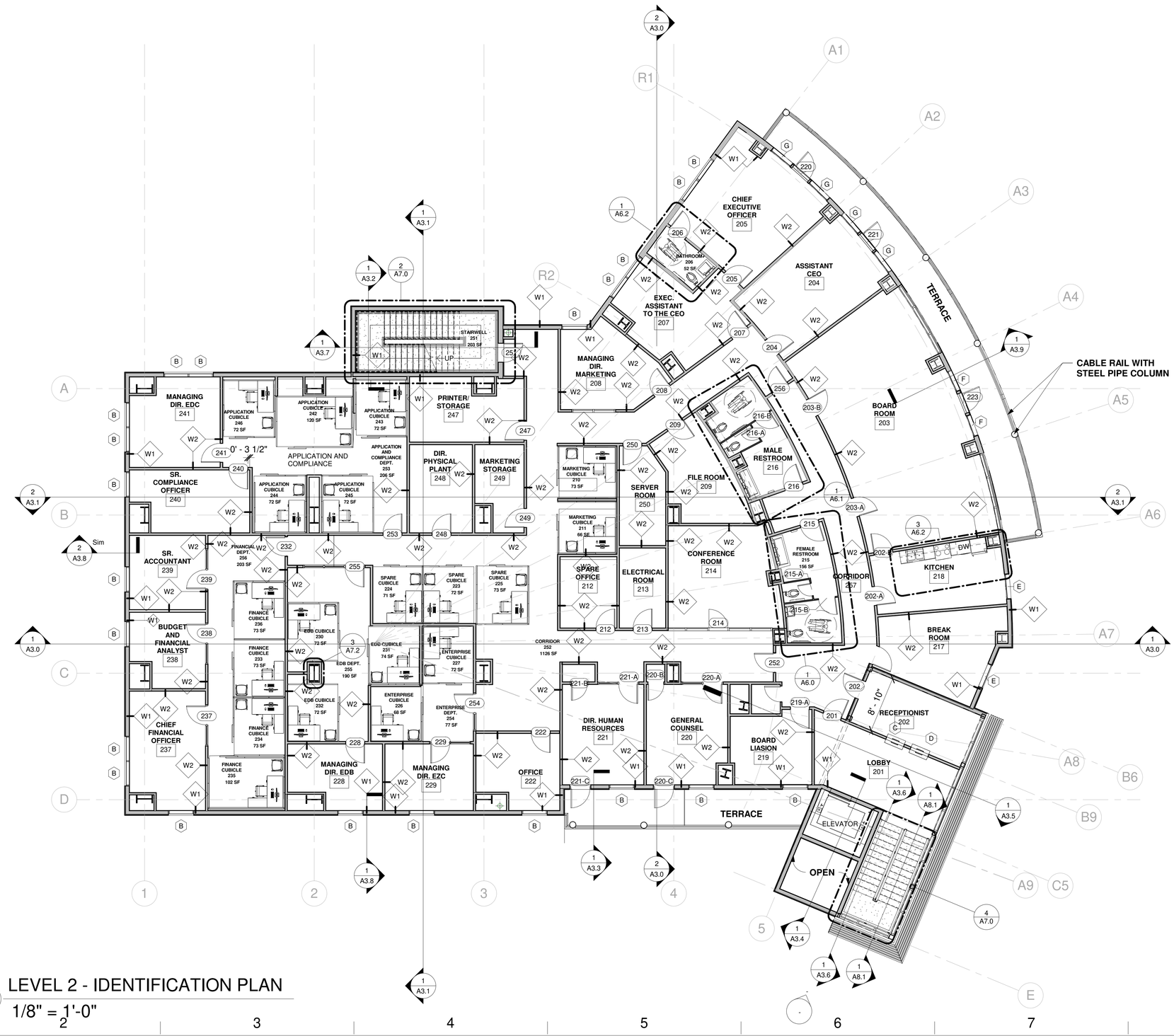
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A1.3

LEVEL 1 REFLECTED CEILING PLAN

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



1 LEVEL 2 - IDENTIFICATION PLAN
1/8" = 1'-0"

REVISIONS		
NO.	DATE	REMARKS
1		50% PROGRESS SET

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ST. THOMAS, USVI

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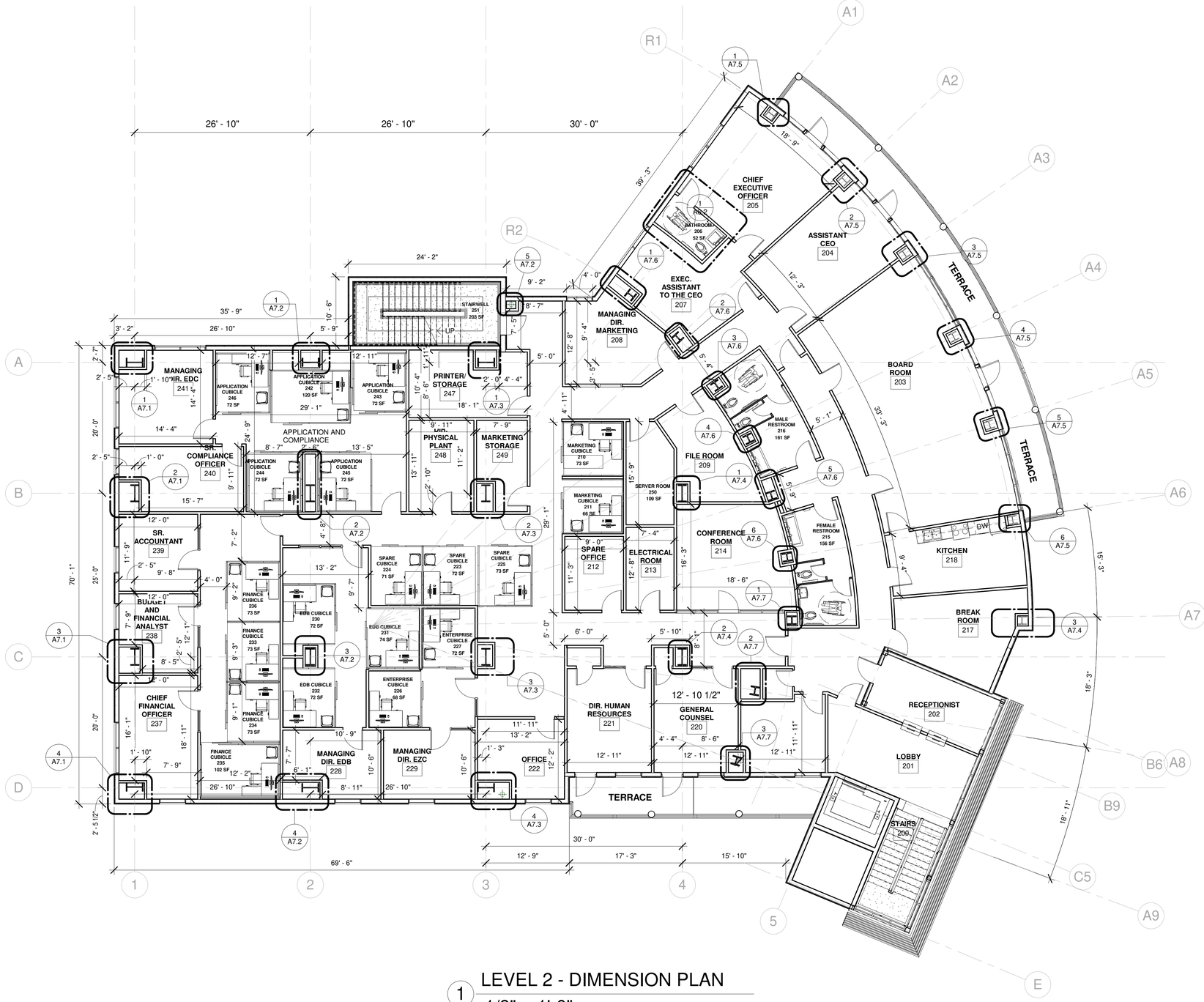
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A1.4

LEVEL 2 IDENTIFICATION PLAN

REGISTERED ARCHITECT J.P. WARD U.S. VIRGIN ISLANDS



1 LEVEL 2 - DIMENSION PLAN

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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ST. THOMAS, USVI

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DRAWN BY:	R.O.P
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A1.5

LEVEL 2 DIMENSION PLAN

LICENSED ARCHITECT USA VIRGIN ISLANDS

CEILING LEGEND

EF	EXHAUST FAN
C1	24X24 ACOUSTICAL CEILING TILE - ARMSTRONG BEVELED TEGULAR 3/16" INTERLUDE DIMENSIONAL TEE SYSTEM WITH 24"X24"X1" OPTIMA OPEN PLAN CEILING TILES (COLOR WHITE). CENTER IN ROOM UNLESS OTHERWISE SHOWN.
C2	PAINTED EXPOSED STRUCTURE
C3	PAINTED GYPUM WALL BOARD
C4	ARMSTRONG CEILING 4" x 96" METALWORKS LINEAR INTERIOR SLATS (OWNER SELECTED COLOR)
C5	ARMSTRONG CEILING 12" X 96" METALWORKS LINEAR EXTERIOR SLATS (OWNER SELECTED COLOR)

C1 9' - 0" A.F.F.
C2
C3 9' - 0" A.F.F.
C3 10' - 0" A.F.F.
C3 11' - 3" A.F.F.
C4 9' - 0" A.F.F.
C4 10' - 8" A.F.F.
C1 10' - 8" A.F.F.

LIGHTING TROUGH
SUNSCREEN ASSEMBLY
LIGHT ALCOVE
SUNSCREEN ASSEMBLY

1 LEVEL 2 REFLECTED CEILING PLAN
1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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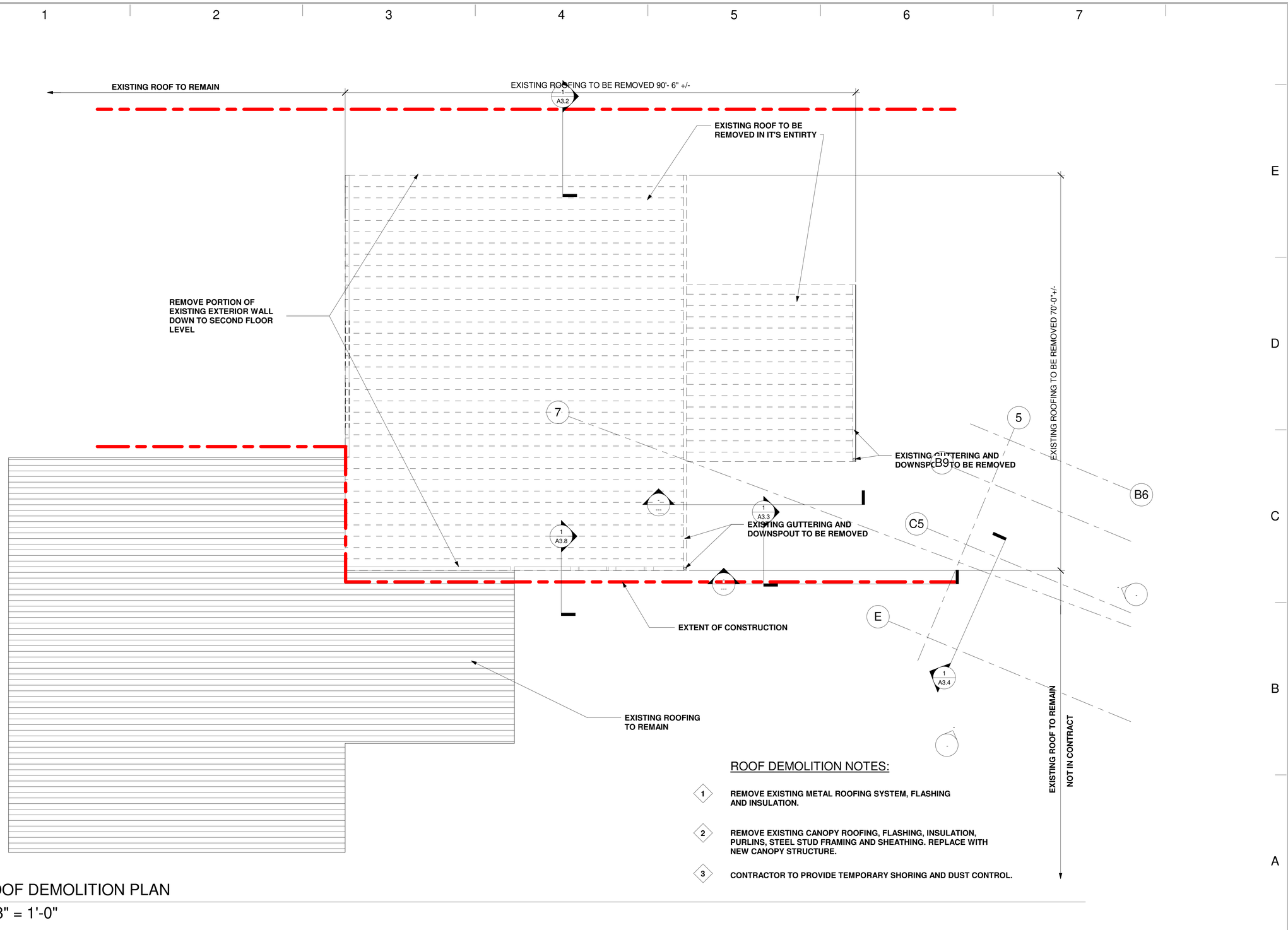
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A1.6

LEVEL 2 REFLECTED CEILING PLAN

LICENSED ARCHITECT
USA
ST. THOMAS, U.S. VIRGIN ISLANDS



ROOF DEMOLITION NOTES:

1. REMOVE EXISTING METAL ROOFING SYSTEM, FLASHING AND INSULATION.
2. REMOVE EXISTING CANOPY ROOFING, FLASHING, INSULATION, PURLINS, STEEL STUD FRAMING AND SHEATHING. REPLACE WITH NEW CANOPY STRUCTURE.
3. CONTRACTOR TO PROVIDE TEMPORARY SHORING AND DUST CONTROL.

1 ROOF DEMOLITION PLAN

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
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APPROVED BY:	Approver
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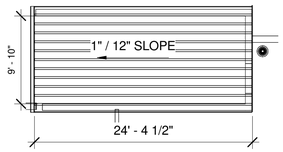
CONTRACT NO.:	-
SHEET NO.:	A1.7

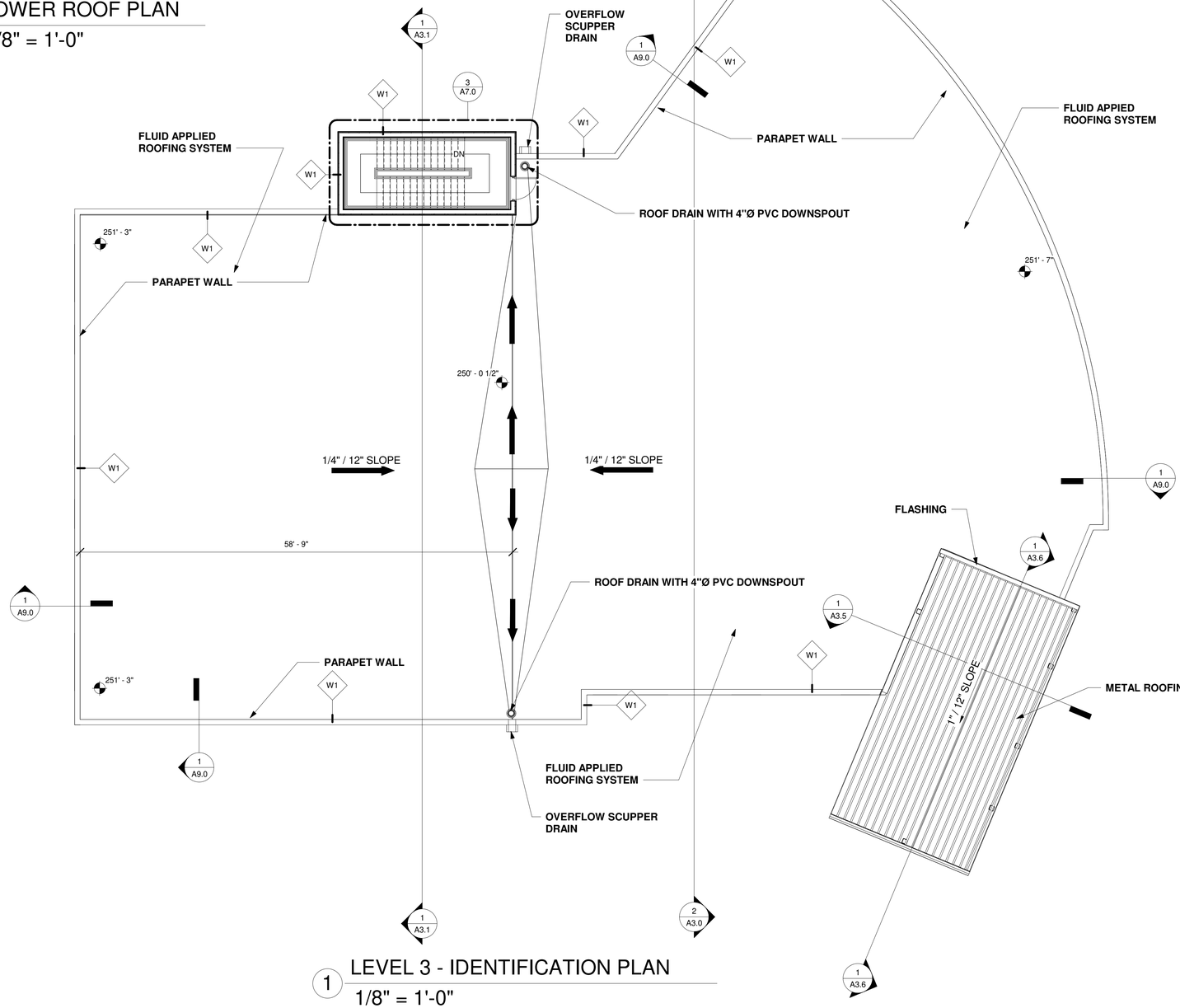
DEMOLITION ROOF PLAN

LICENSED ARCHITECT U.S. VIRGIN ISLANDS



LEVEL 3 - ENLARGED STAIR TOWER ROOF PLAN

2 1/8" = 1'-0"



LEVEL 3 - IDENTIFICATION PLAN

1 1/8" = 1'-0"

REVISIONS		
NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

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APPROVED BY:	Approver
PROJECT NO.:	21-003.C
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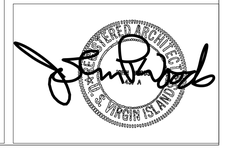
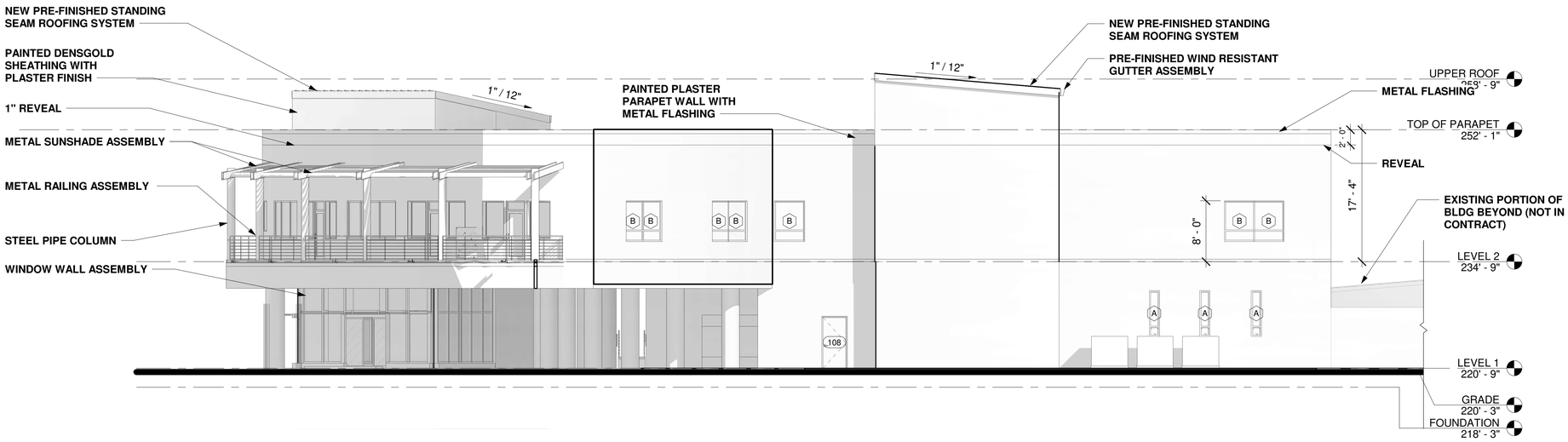
SHEET NO.:	A1.8

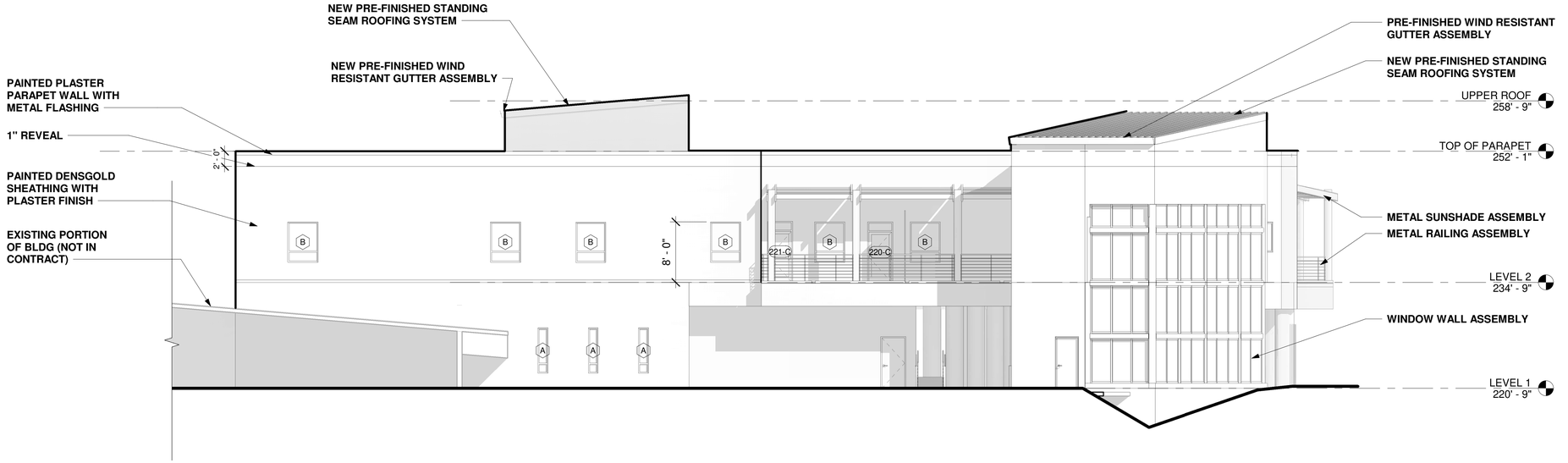
LEVEL 3 IDENTIFICATION PLAN





1 NORTH ELEVATION

1/8" = 1'-0"



2 SOUTH ELEVATION

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS
1		50% PROGRESS SET

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

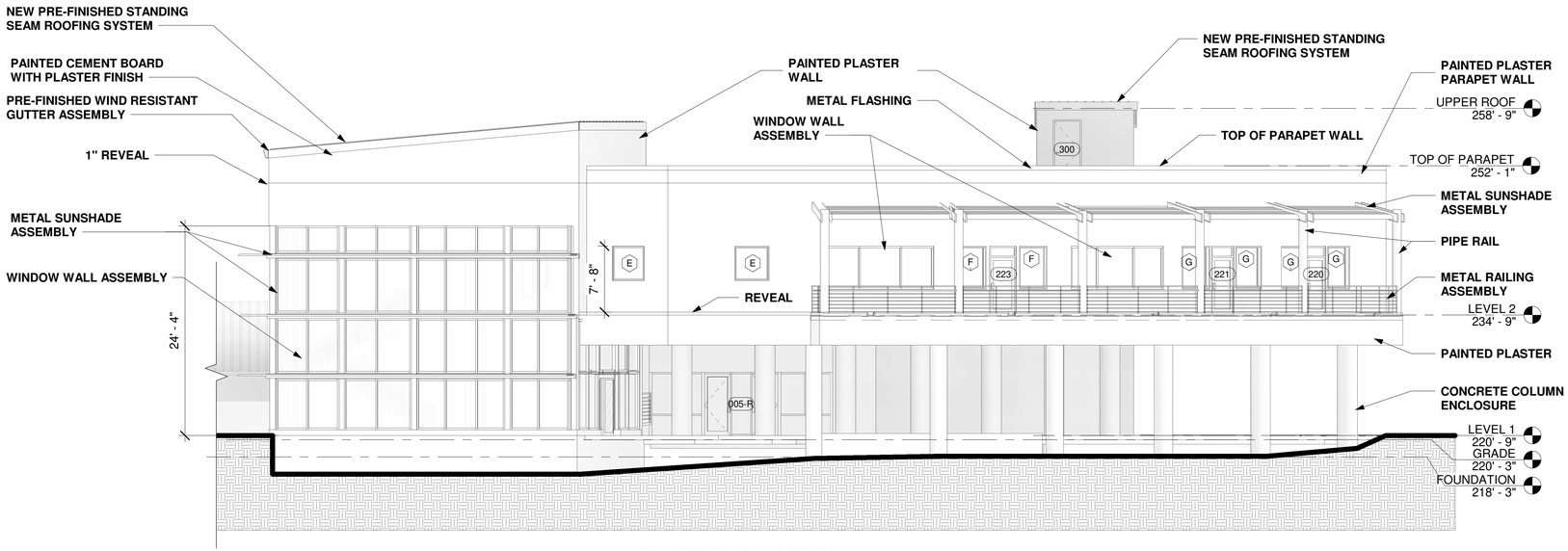
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A2.0

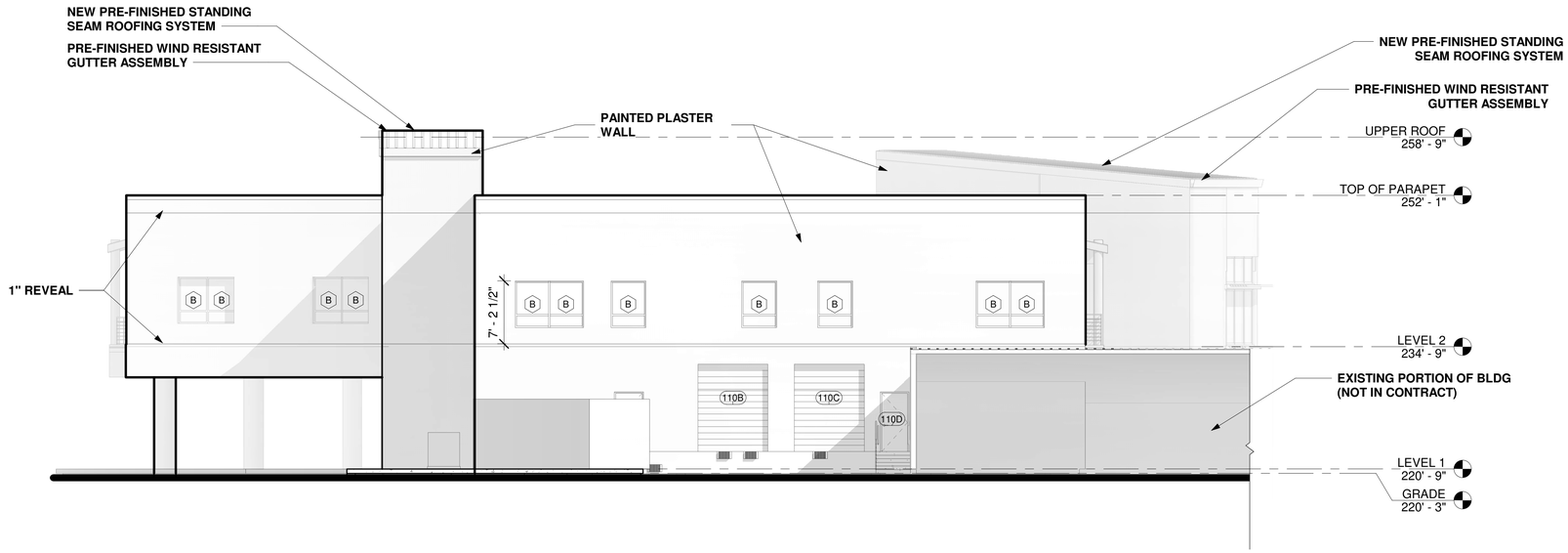
BUILDING ELEVATIONS

LICENSED ARCHITECT U.S. VIRGIN ISLANDS



1 EAST ELEVATION

1/8" = 1'-0"



2 WEST ELEVATION

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: B.J.S

CHECKED BY: J.P.W

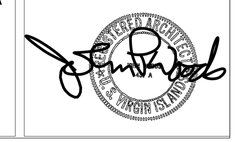
APPROVED BY: J.P.W.

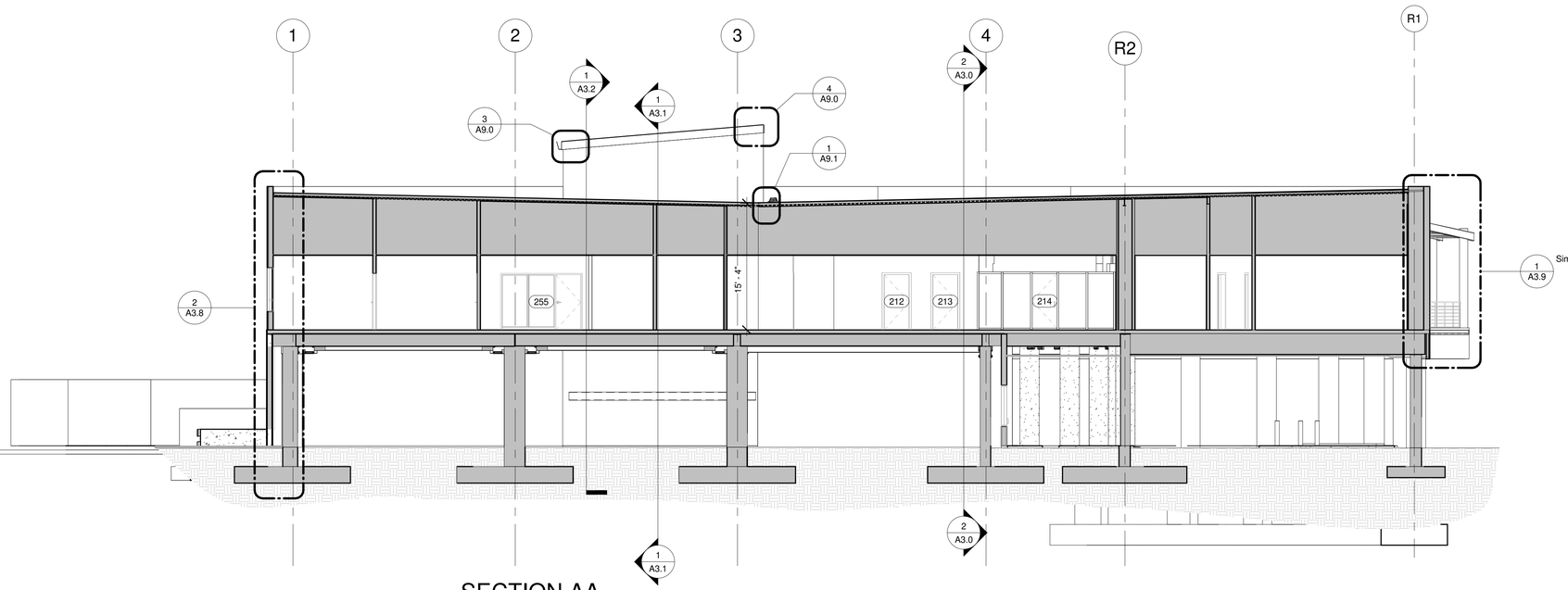
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A2.1

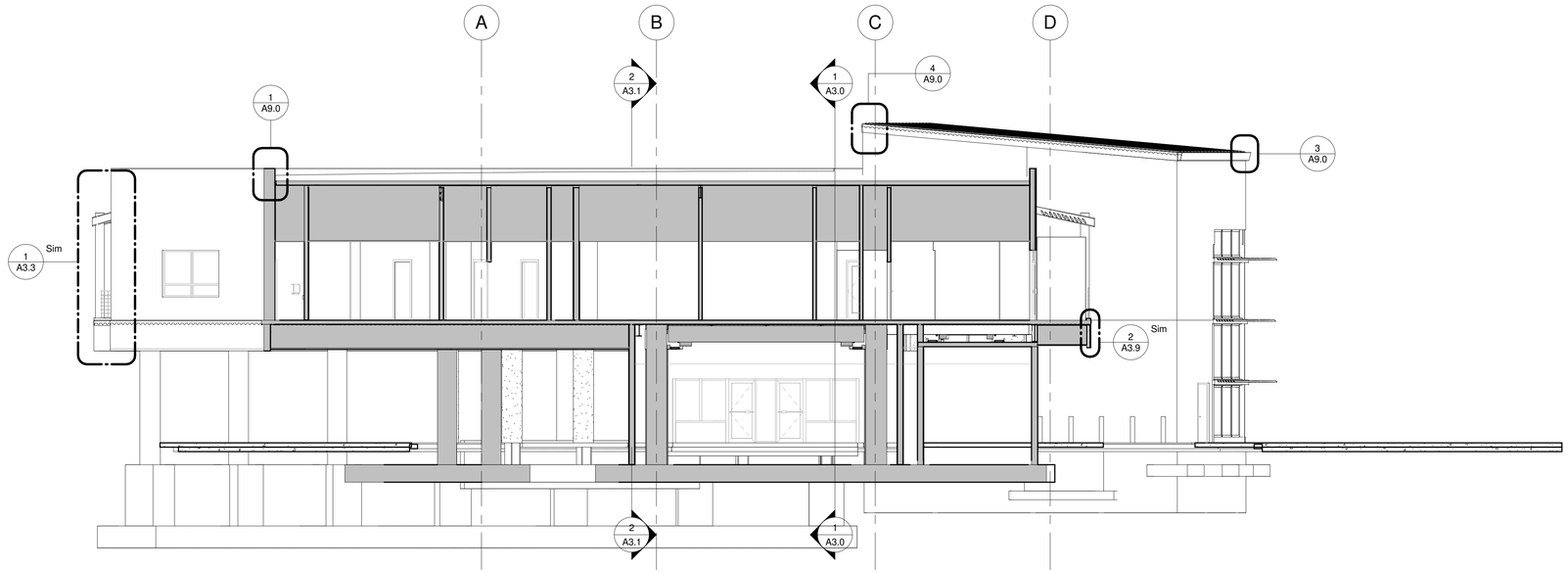
BUILDING ELEVATIONS





1 SECTION AA

1/8" = 1'-0"



2 SECTION BB

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: B.J.S

CHECKED BY: J.P.W

APPROVED BY: J.P.W.

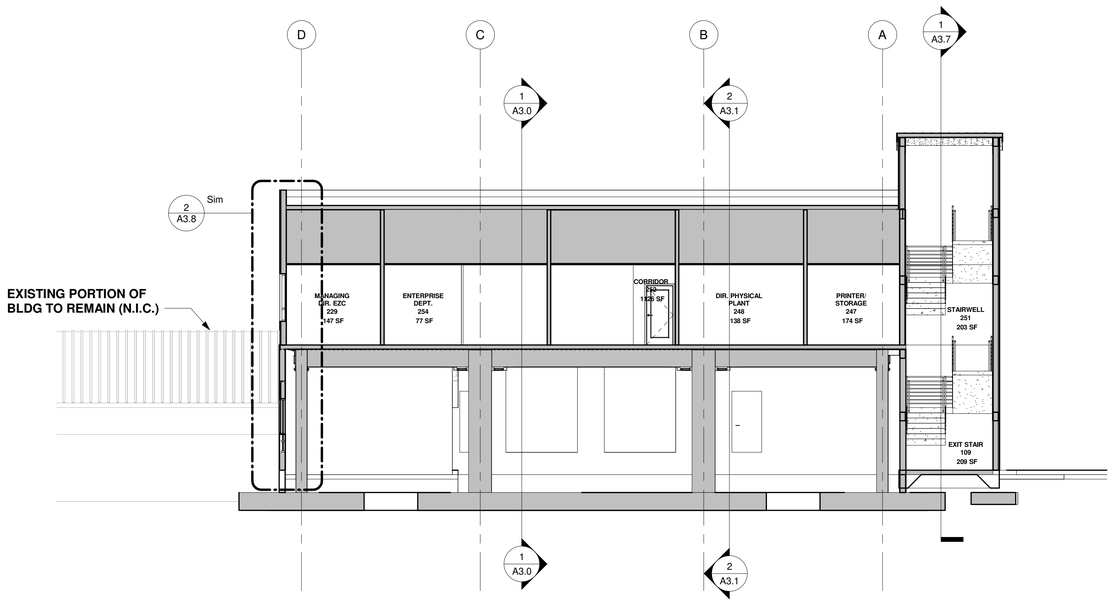
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A3.0

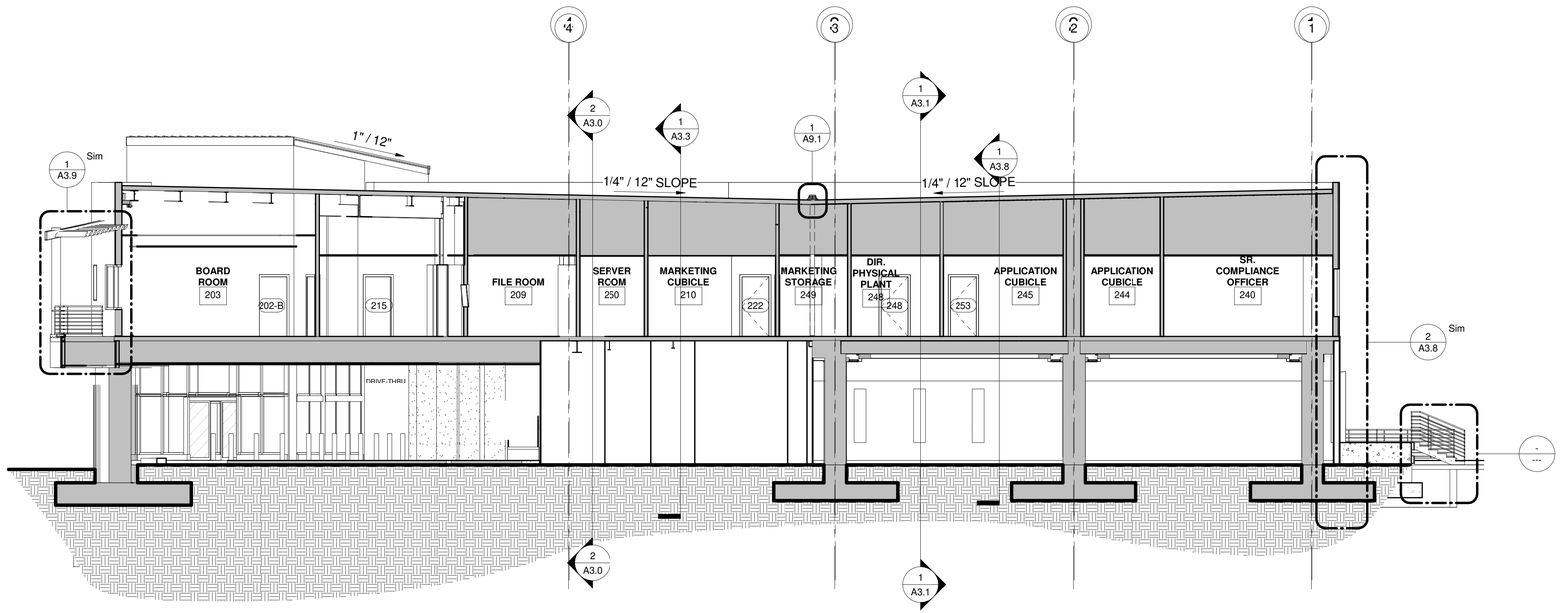
BUILDING SECTIONS

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



1 SECTION CC

1/8" = 1'-0"



2 SECTION DD

1/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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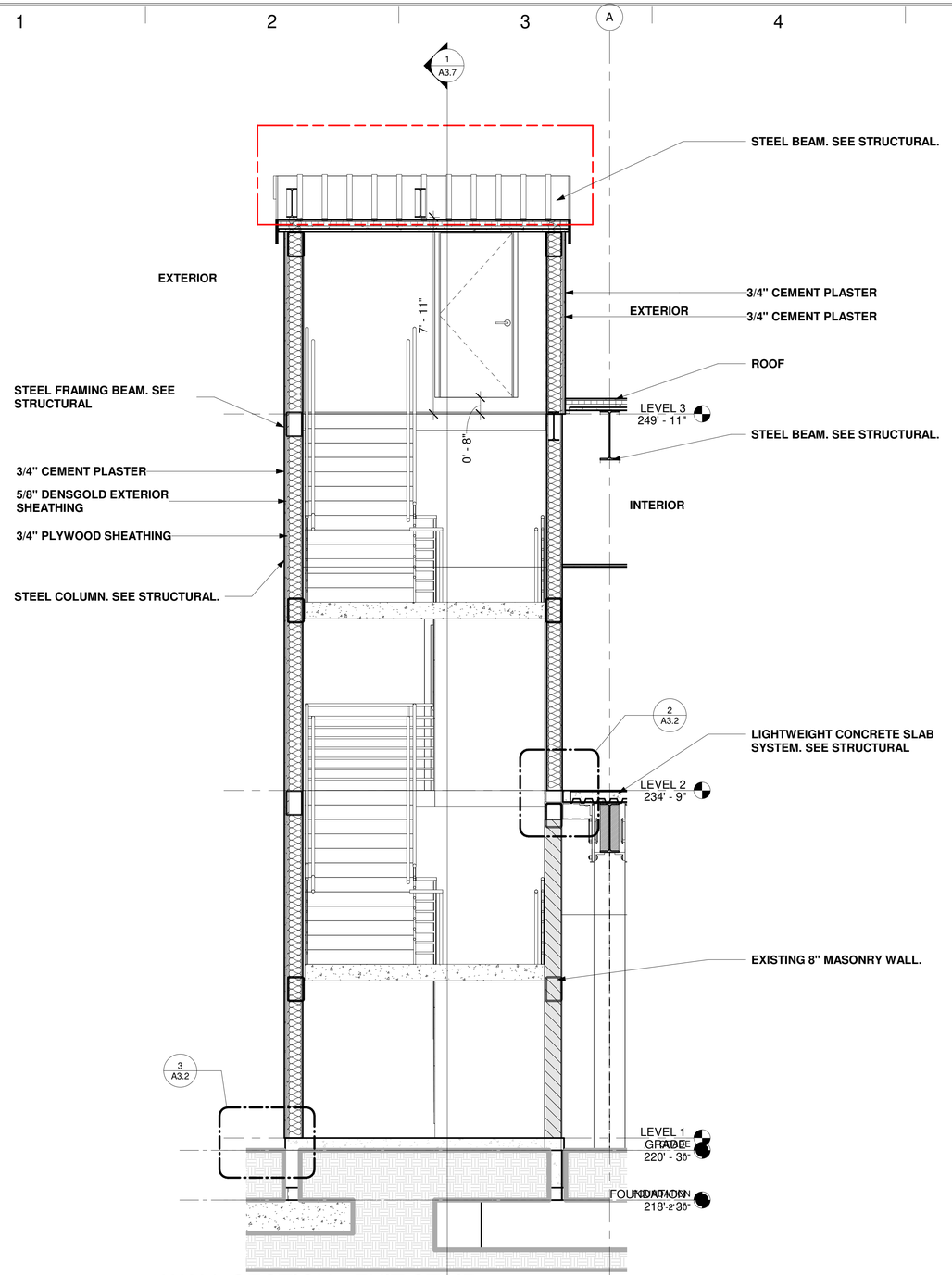
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A3.1

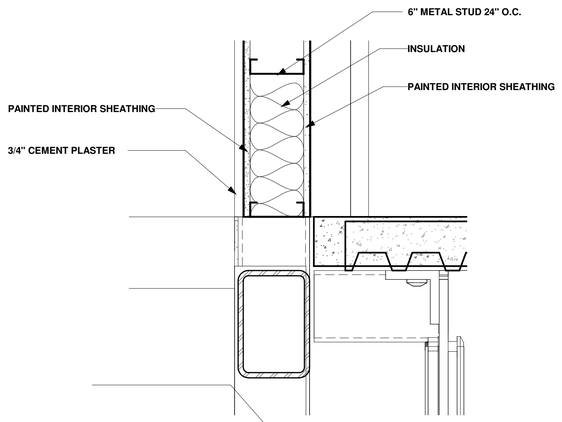
BUILDING SECTIONS

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



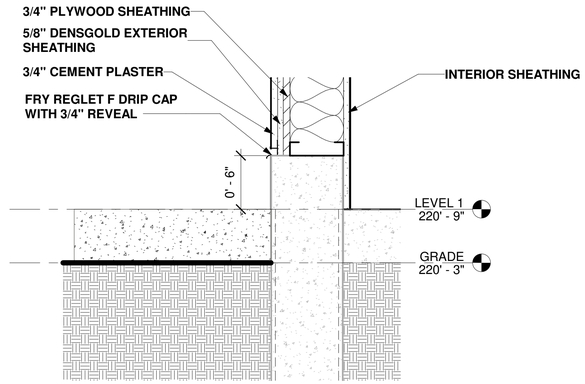
WALL SECTION - EXIT STAIR TOWER

1 3/8" = 1'-0"



WALL SECTION - EXIT STAIR TOWER - Callout 1

2 1 1/2" = 1'-0"



EXTERIOR WALL TRANSITION SLAB TO GRADE DETAIL

3 1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
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DATE:	02.14.2025
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CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

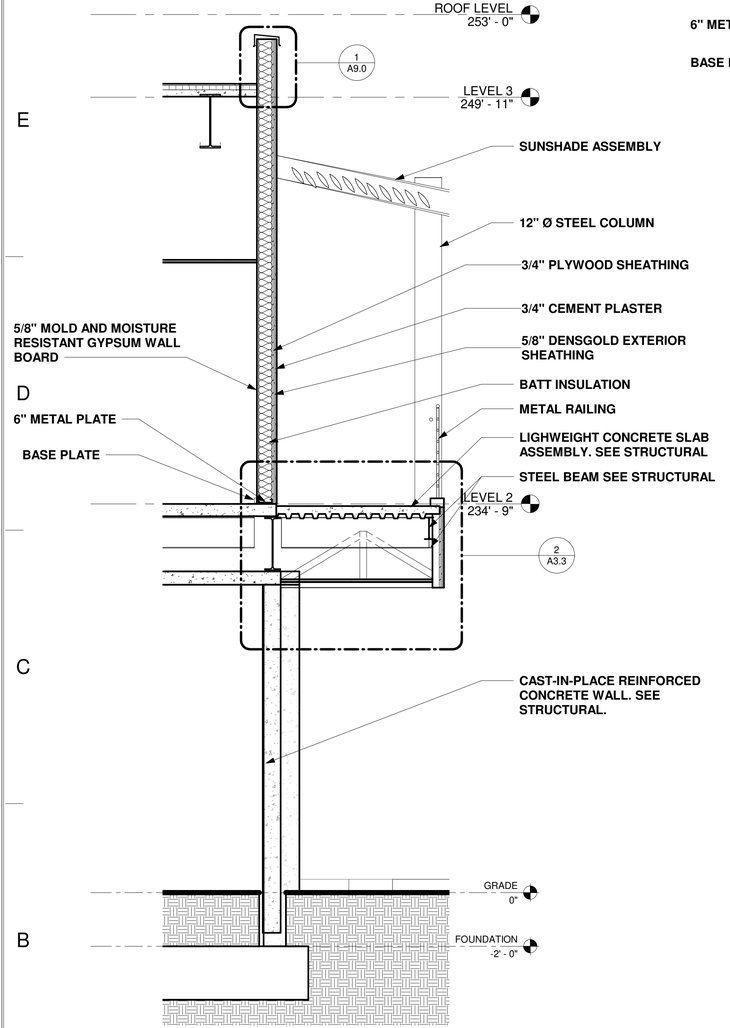
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A3.2

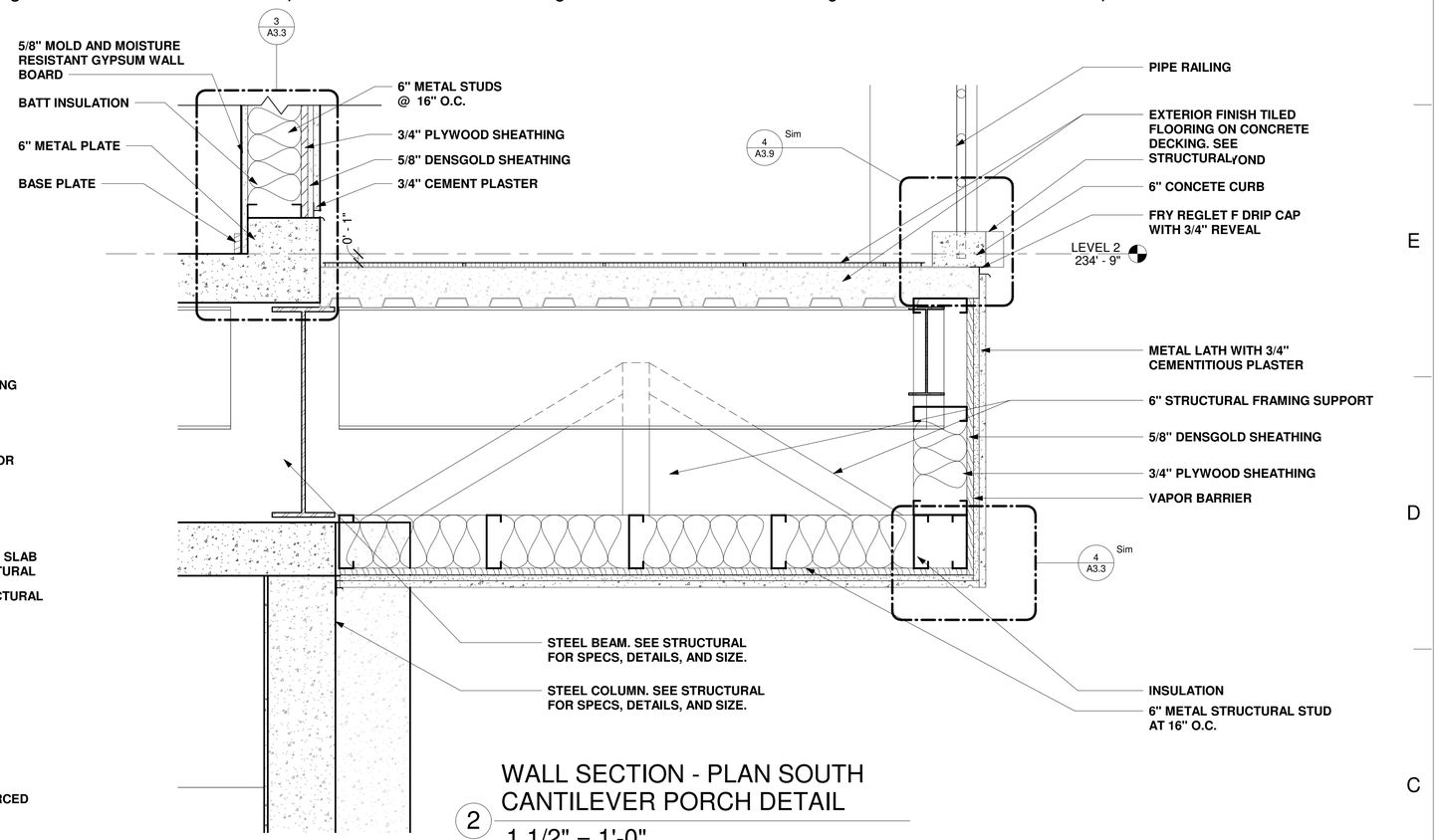
WALL SECTIONS

LICENSED ARCHITECT — U.S. VIRGIN ISLANDS — USA



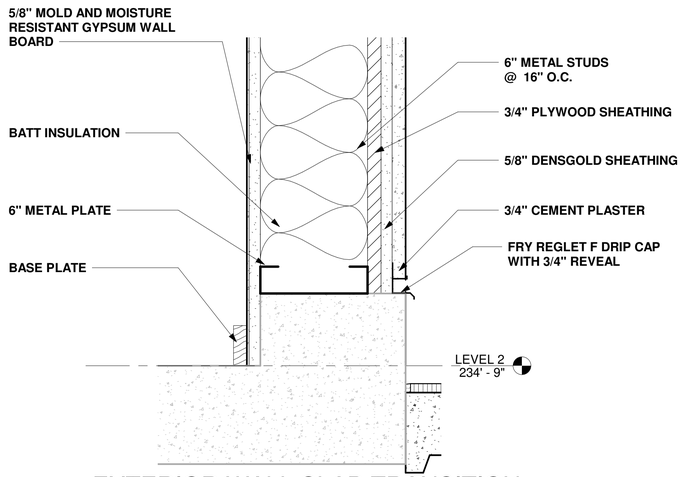
WALL SECTION - PLAN SOUTH PORCH

1 — 3/8" = 1'-0"



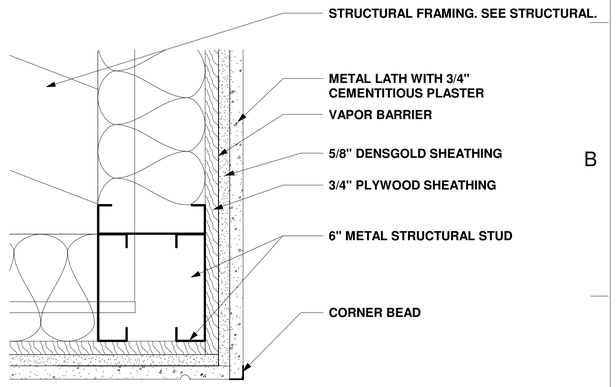
WALL SECTION - PLAN SOUTH CANTILEVER PORCH DETAIL

2 — 1 1/2" = 1'-0"



EXTERIOR WALL SLAB TRANSITION DETAIL

3 — 3" = 1'-0"



PORCH BULKHEAD CORNER DETAIL

4 — 3" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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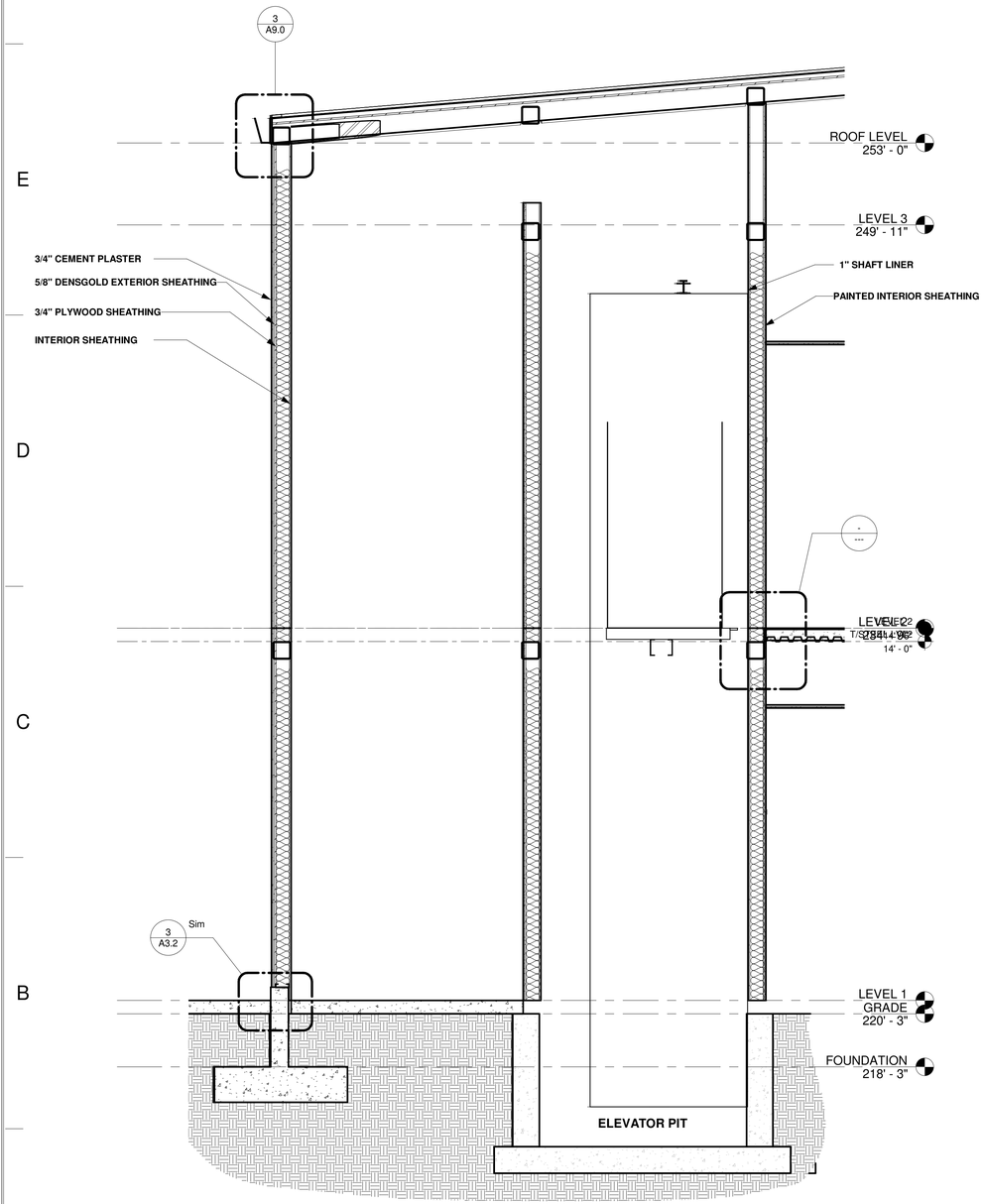
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A3.3

WALL SECTIONS

REGISTERED ARCHITECT — U.S. VIRGIN ISLANDS



1 WALL SECTION - ELEVATOR SHAFT

3/8" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: B.J.S

CHECKED BY: J.P.W

APPROVED BY: J.P.W.

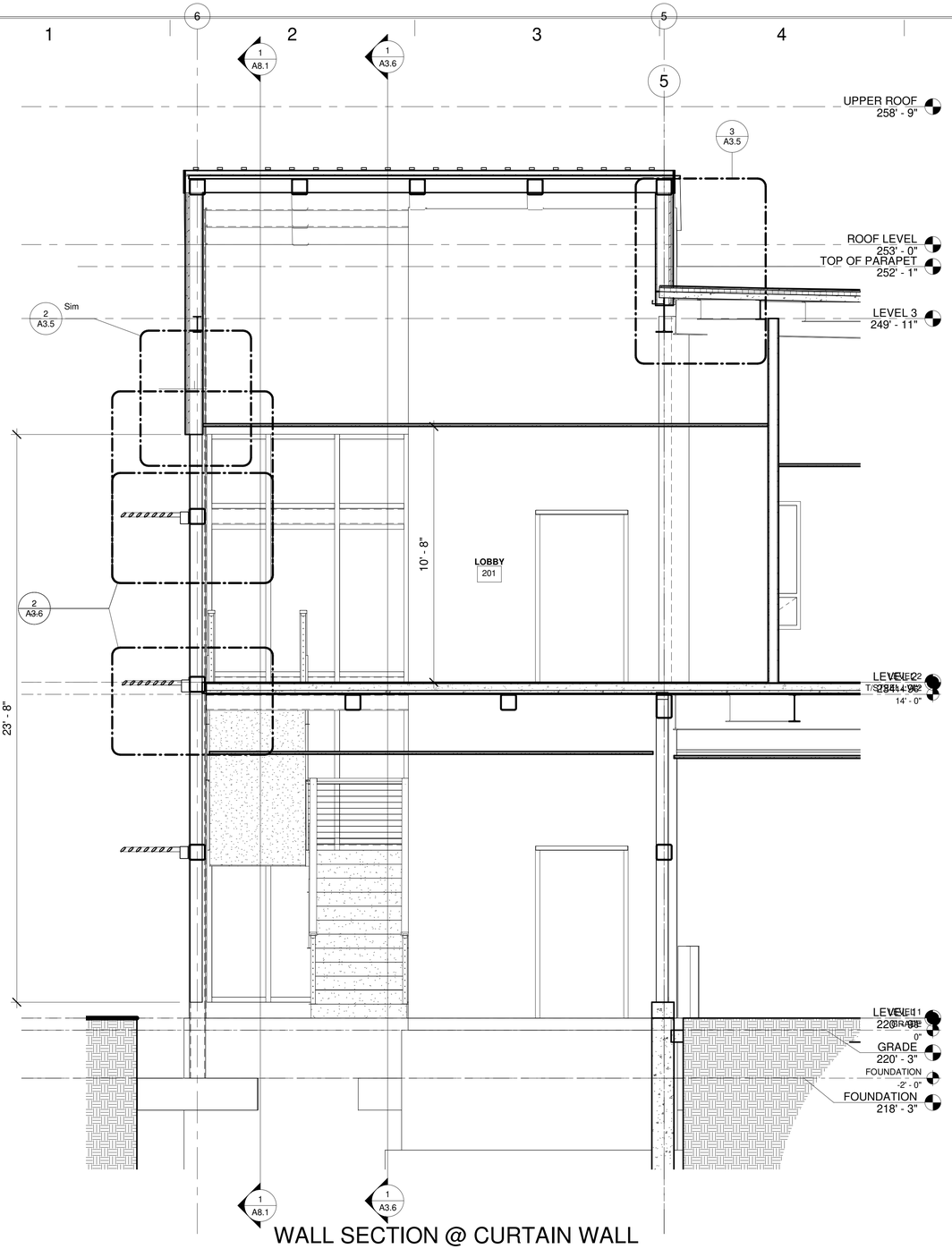
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A3.4

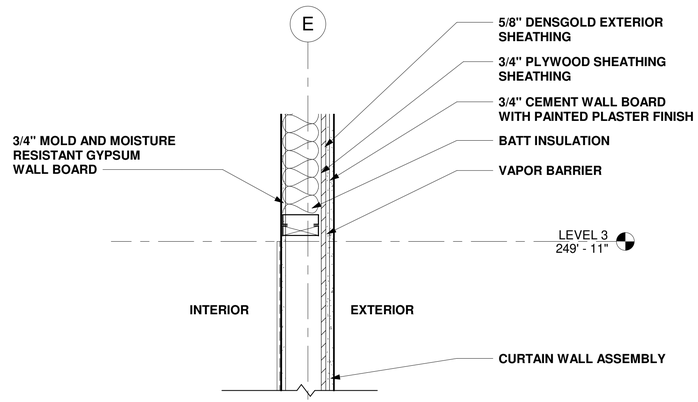
WALL SECTIONS

LICENSED ARCHITECT U.S. VIRGIN ISLANDS



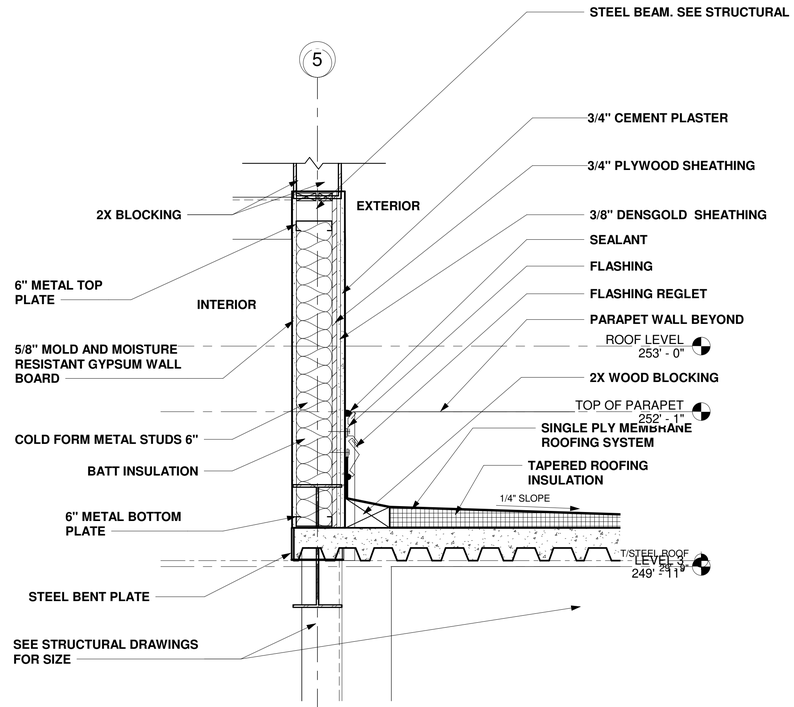
WALL SECTION @ CURTAIN WALL ASSEMBLY

1 — 3/8" = 1'-0"



EXTERIOR STUD WALL CURTAIN WALL CONNECTION

2 — 1" = 1'-0"



WALL SECTION AT HEADER

3 — 1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SUB BASE, CROWN BAY
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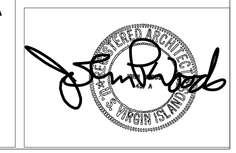
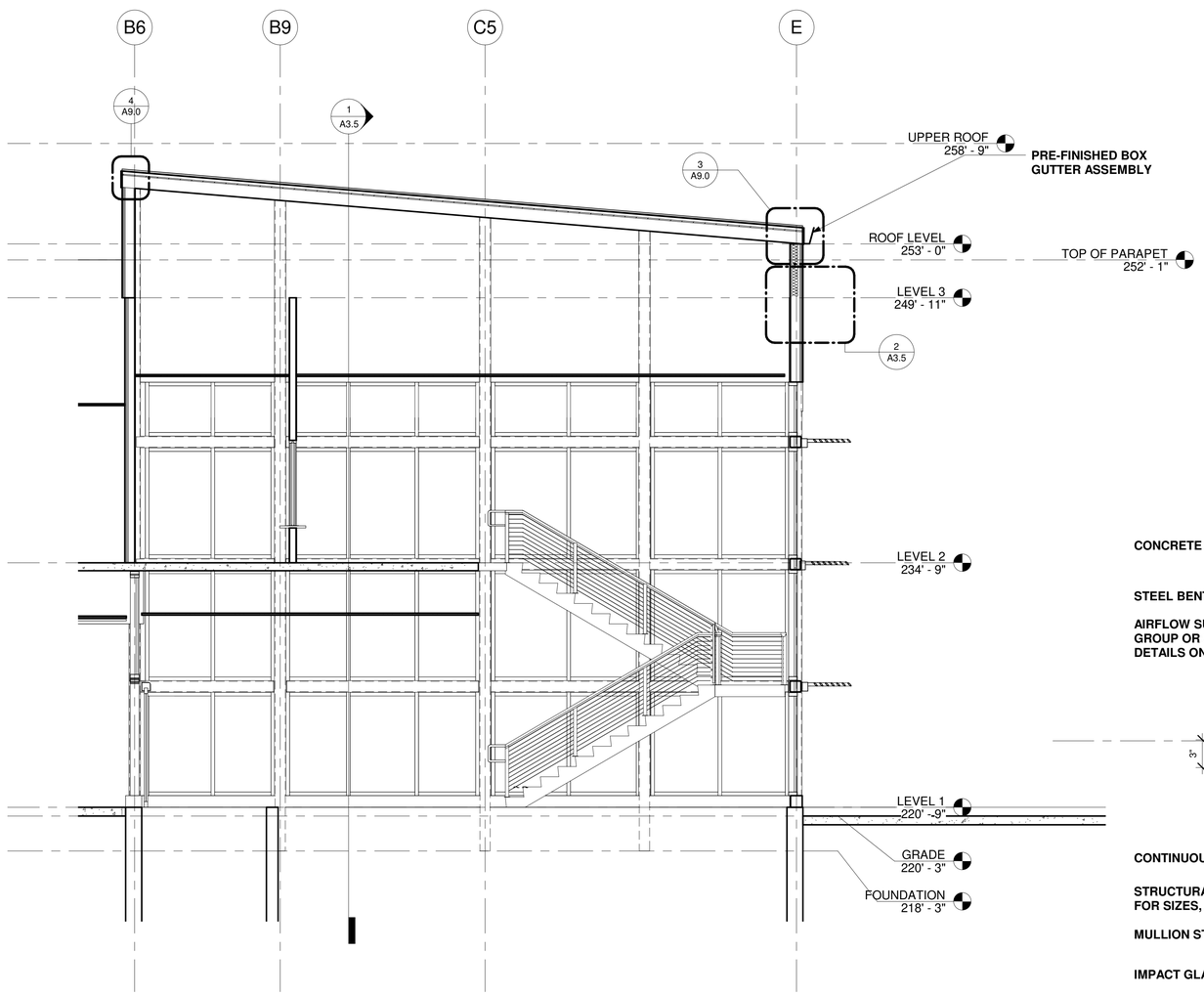
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A3.5

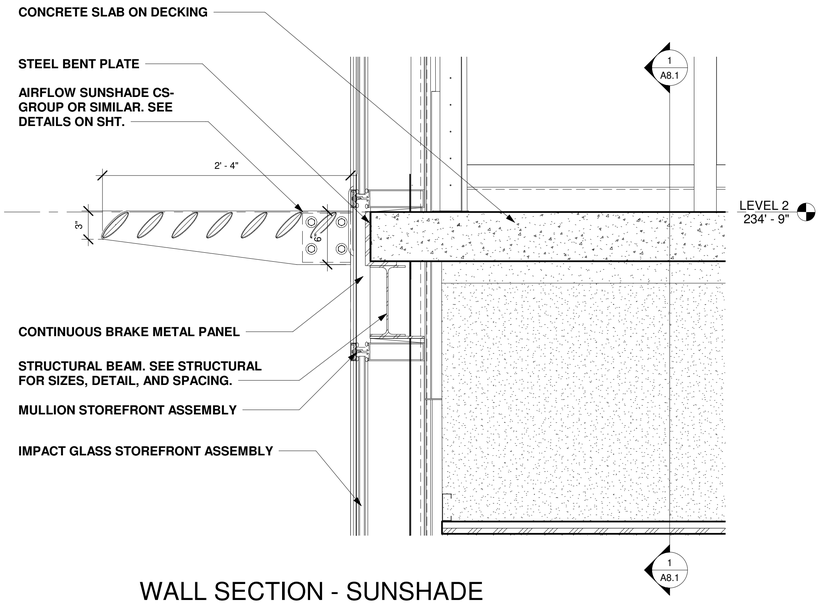
WALL SECTIONS





1 STAIR SECTION @ CURTAIN WALL ASSEMBLY

1/4" = 1'-0"



2 WALL SECTION - SUNSHADE ASSEMBLY CONNECTION TO SLAB

1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

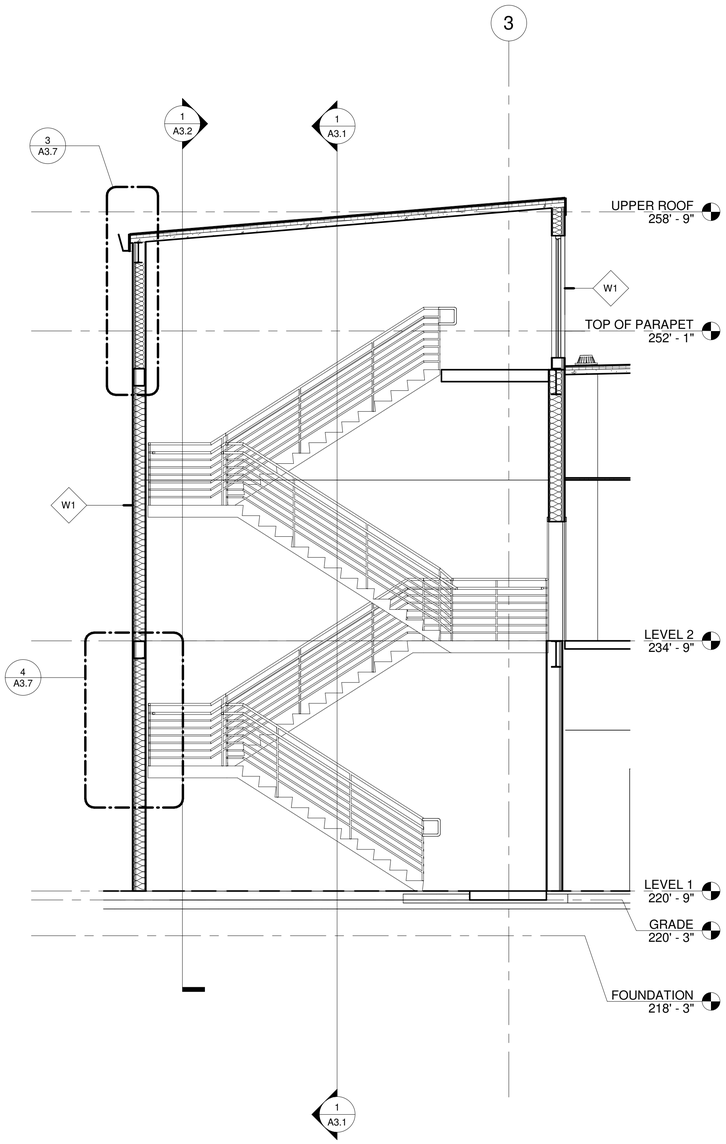
PROJECT NO.: 21-003.C

CONTRACT NO.: -

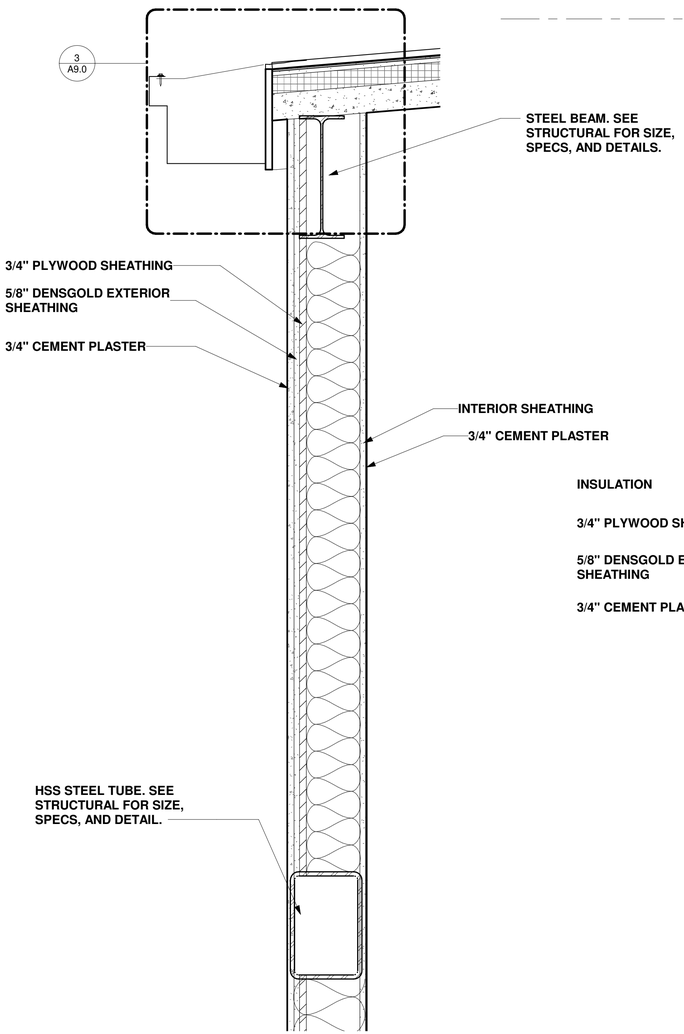
SHEET NO.: A3.6

WALL SECTIONS

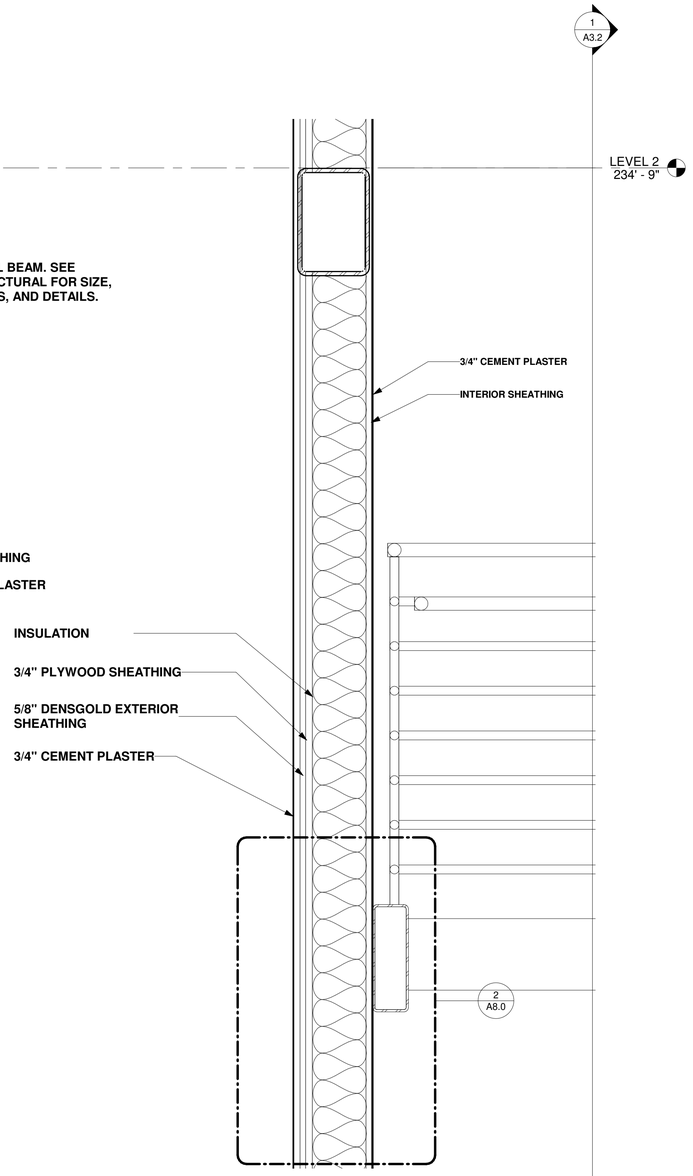
LICENSED ARCHITECT — U.S. VIRGIN ISLANDS



1 EXIT TOWER STAIR SECTION
1/4" = 1'-0"



3 PARTIAL SECTION AT EXIT STAIR TOWER TO EAVE
1 1/2" = 1'-0"



4 PARTIAL SECTION AT EXIT STAIR TOWER
1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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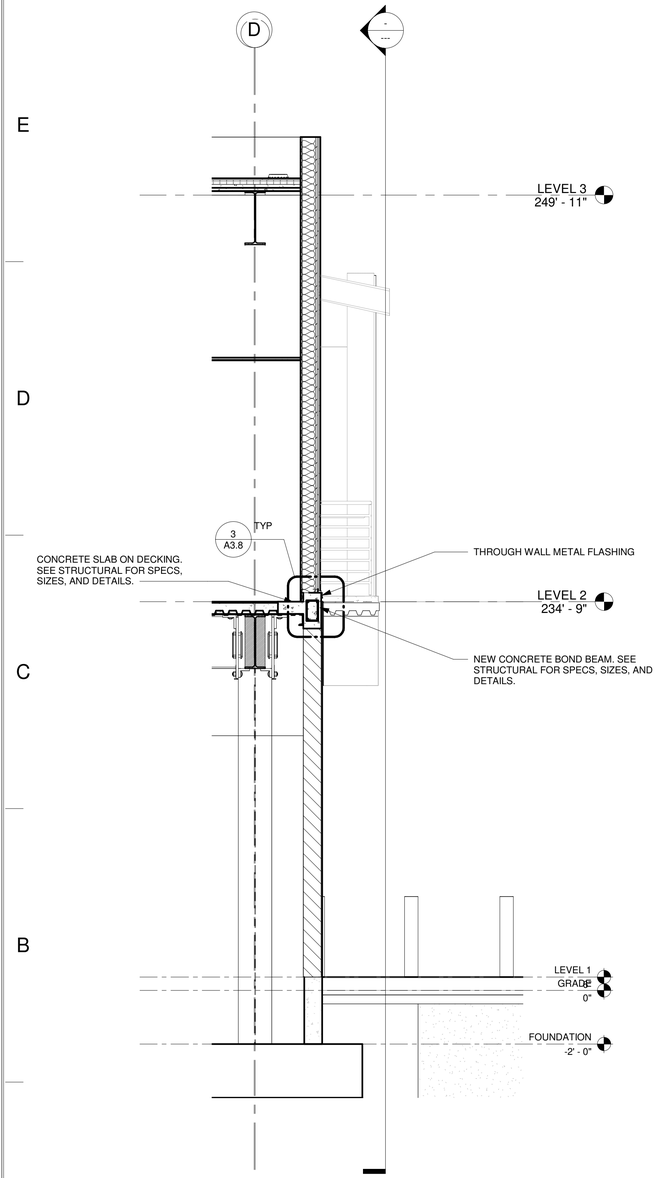
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A3.7

WALL SECTIONS

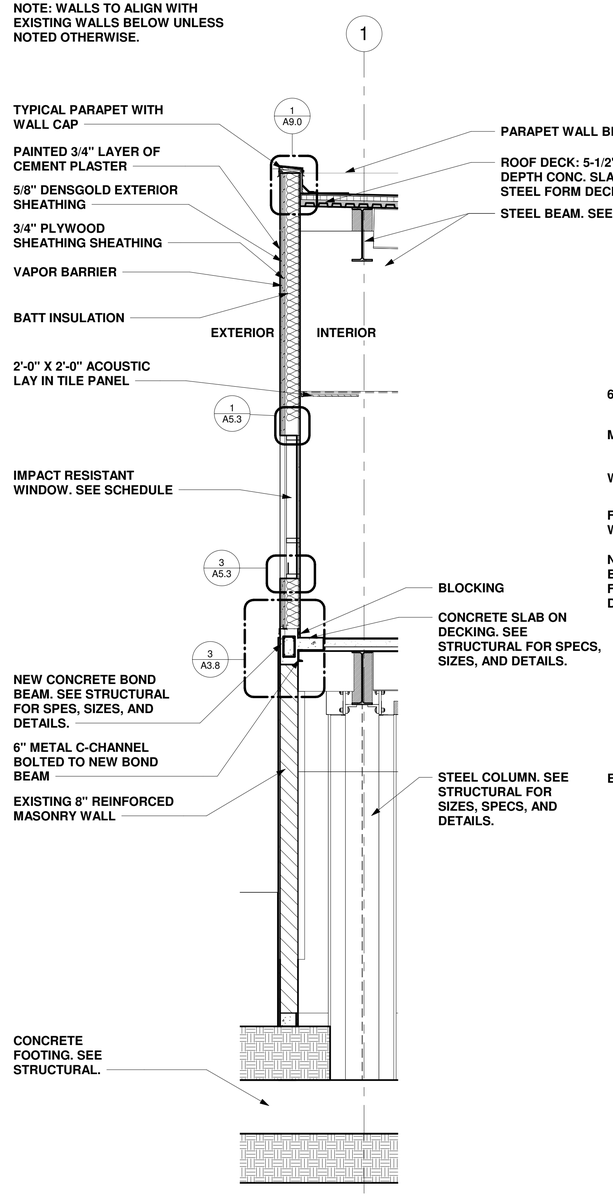
LICENSED ARCHITECT U.S. VIRGIN ISLANDS



WALL SECTION - MANAGING DIR. EDB OFFICE

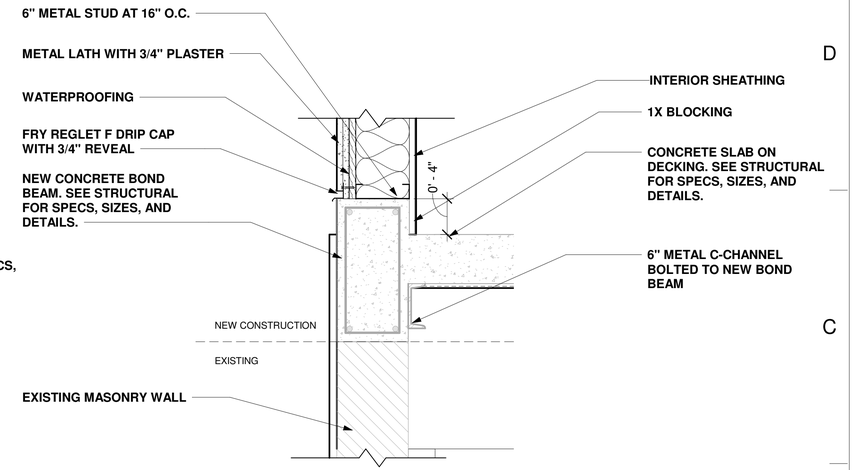
1 — 3/8" = 1'-0"

NOTE: WALLS TO ALIGN WITH EXISTING WALLS BELOW UNLESS NOTED OTHERWISE.



WALL SECTION - THROUGH LOADING DOCK

2 — 3/8" = 1'-0"



TRANSITION FROM EXISTING WALL TO NEW STUD WALL

3 — 1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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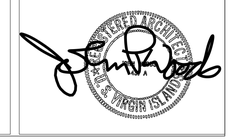
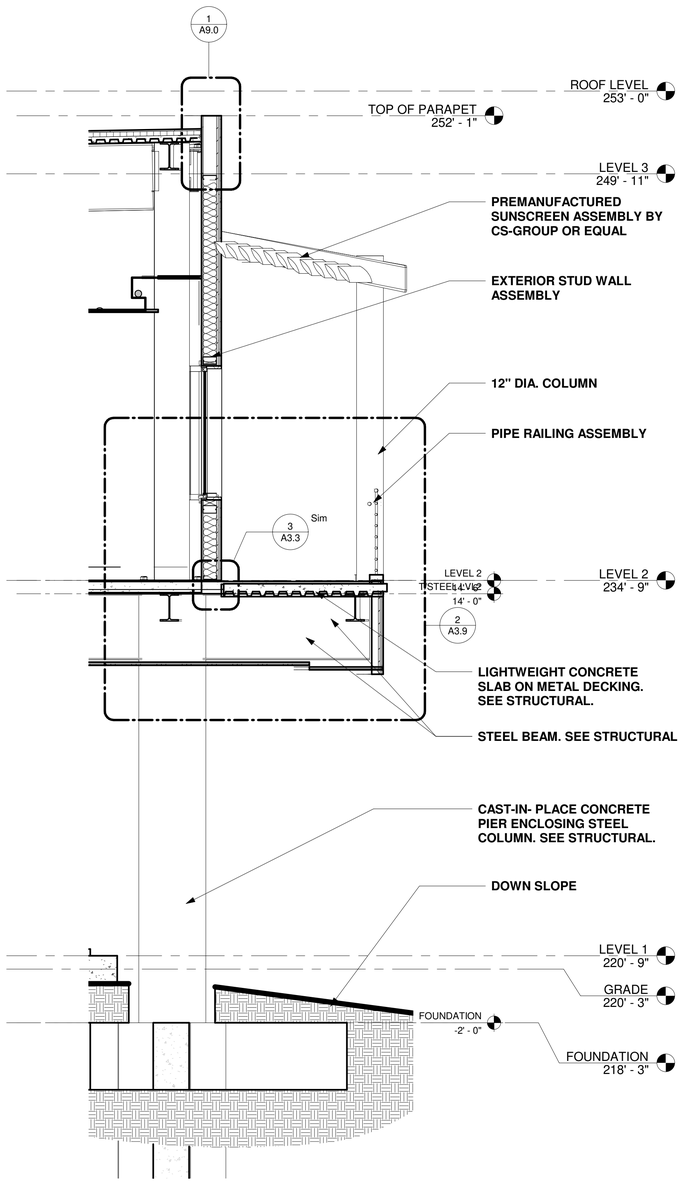
CONTRACT NO.:	-
SHEET NO.:	A3.8

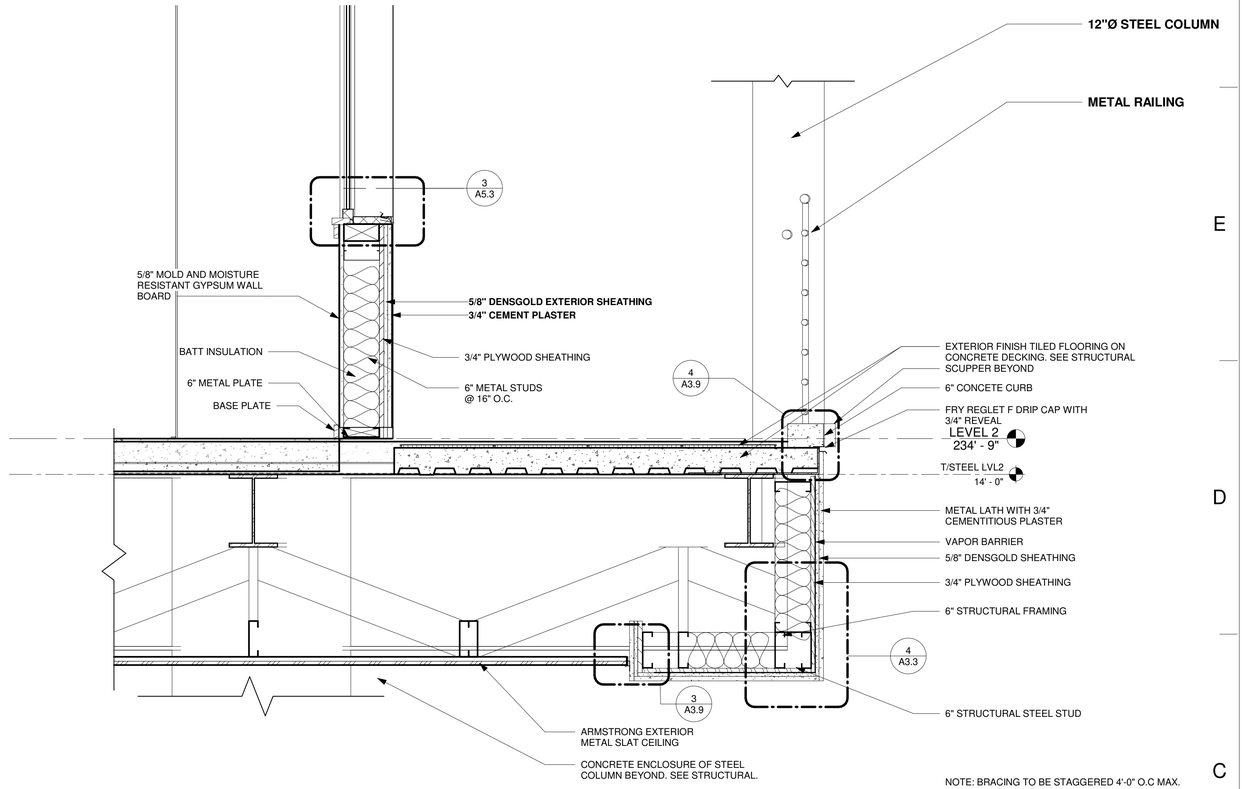
WALL SECTIONS





1 WALL SECTION - 2ND LEVEL BOARD ROOM

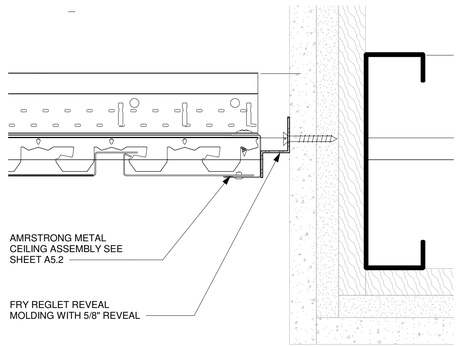
3/8" = 1'-0"



2 WALL SECTION - CANTILEVER PORCH DETAIL

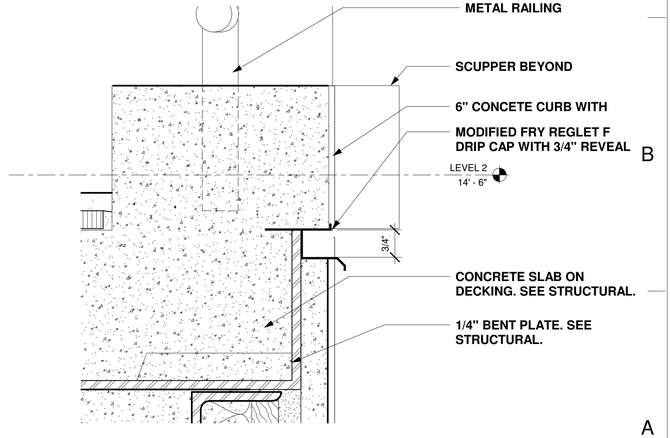
1" = 1'-0"

NOTE: BRACING TO BE STAGGERED 4'-0" O.C MAX.



3 F REVEAL STUCCO MOLDING DETAIL

6" = 1'-0"



4 CANTILEVER PORCH CURB DETAIL

6" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A3.9

WALL SECTIONS

LICENSED ARCHITECT — U.S.V.I. — VIRGIN ISLANDS

ROOM FINISH SCHEDULE										
ROOM NO.	ROOM NAME	FLOOR	WALL				CEILING		REMARKS	Area
		FIN	NORTH	SOUTH	EAST	WEST	FIN	HEIGHT		
LEVEL 1										
101	LOBBY	F1	G1	G1	P1	G1	C4	11' - 0"		359 SF
102	ENTRY STAIR	F1	G1	G1	G1		C2			146 SF
103	ELEV.						C2			74 SF
104	MACHINE ROOM	T	P1	P1	P1	P1	C3			81 SF
105	PUMP RM.	T	P2	P2	P2	P2	C2	11' - 0"		75 SF
106	CISTERN		P2	P2	P2	P2	C2			251 SF

ROOM FINISH SCHEDULE										
ROOM NO.	ROOM NAME	FLOOR	WALL				CEILING		REMARKS	Area
		FIN	NORTH	SOUTH	EAST	WEST	FIN	HEIGHT		
107	LEASED AREA		P1	P1	P1	P1		9' - 0"	FUTURE LEESEE RESPONSIBILITY (NIC)	4913 SF
108	MECHANICAL ROOM	T	P2	P2	P2	P2	C2	9' - 0"		389 SF
109	EXIT STAIR		P1	P1	P1	P1	C2			209 SF
LEVEL 2										
200	STAIRS	F1	P1	P1	P1	P1	C1	10' - 8"		159 SF
201	LOBBY	F1	P1	G1	P1	P1	C1	9' - 0"		239 SF
202	RECEPTIONIST	F1	P1	G1	P1	P1	C1	9' - 0"		172 SF
203	BOARD ROOM	F1	P1	P1	P1	P1	C3	10'-0"/ 11'-3"		805 SF
204	ASSISTANT CEO	F1	P1	P1	P1	P1	C1	9' - 0"		283 SF
205	CHIEF EXECUTIVE OFFICER	F1	P1	P1	P1	P1	C1	9' - 0"		331 SF
206	BATHROOM	F1	P1	P1	P1	P1	C3	9' - 0"		52 SF
207	EXEC. ASSISTANT TO THE CEO	F1	P1	P1	P1	P1	C1	9' - 0"		221 SF
208	MANAGING DIR. MARKETING	F1	P1	P1	P1	P1	C1	9' - 0"		183 SF
209	FILE ROOM	F1	P1	P1	P1	P1	C1	9' - 0"		165 SF
212	SPARE OFFICE	F1	P1	P1	P1	P1	C1	9' - 0"		97 SF
213	ELECTRICAL ROOM	F1	P1	P1	P1	P1	C2	9' - 0"		92 SF
214	CONFERENCE ROOM	F1	P1	P1	P1	G2	C1	9' - 0"		275 SF
215	FEMALE RESTROOM	F1	P1	P1	P1	P1	C3	9' - 0"		156 SF
216	MALE RESTROOM	F1	P1	P1	P1	P1	C3	9' - 0"		161 SF
217	BREAK ROOM	F1	P1	P1	P1	P1	C1	9' - 0"		223 SF
218	KITCHEN	F1	P1	P1	P1	P1	C1	9' - 0"		209 SF
219	BOARD LIASION	F1	P1	P1	P1	P1	C1	9' - 0"		141 SF
220	GENERAL COUNSEL	F1	P1	P1	P1	P1	C1	9' - 0"		197 SF
221	DIR. HUMAN RESOURCES	F1	P1	P1	P1	P1	C1	9' - 0"		204 SF
222	OFFICE	F1	P1	P1	P1	P1	C1	9' - 0"		147 SF
228	MANAGING DIR. EDB	F1	P1	P1	P1	P1	C1	9' - 0"		140 SF
229	MANAGING DIR. EZC	F1	P1	P1	P1	P1	C1	9' - 0"		147 SF
237	CHIEF FINANCIAL OFFICER	F1	P1	P1	P1	P1	C1	9' - 0"		215 SF
238	BUDGET AND FINANCIAL ANALYST	F1	P1	P1	P1	P1	C1	9' - 0"		130 SF
239	SR. ACCOUNTANT	F1	P1	P1	P1	P1	C1	9' - 0"		141 SF
240	SR. COMPLIANCE OFFICER	F1	P1	P1	P1	P1	C1	9' - 0"		173 SF
241	MANAGING DIR. EDC	F1	P1	P1	P1	P1	C1	9' - 0"		193 SF
247	PRINTER/ STORAGE	F1	P1	P1	P1	P1	C1	9' - 0"		174 SF
248	DIR. PHYSICAL PLANT	F1	P1	P1	P1	P1	C1	9' - 0"		138 SF
249	MARKETING STORAGE	F1	P1	P1	P1	P1	C1	9' - 0"		95 SF
250	SERVER ROOM	F1	P1	P1	P1	P1	C2			109 SF
251	STAIRWELL	F1	P1	P1	P1	P1	C1			203 SF
252	CORRIDOR	F1	P1	P1	P1	P1	C4	9' - 0"		1126 SF
253	APPLICATION AND COMPLIANCE DEPT.	F1	P1	P1	P1	P1	C1	9' - 0"		206 SF
254	ENTERPRISE DEPT.	F1	P1	P1	P1	P1	C1	9' - 0"		77 SF
255	EDB DEPT.	F1	P1	P1	P1	P1	C1	9' - 0"		190 SF
256	FINANCIAL DEPT.	F1	P1	P1	P1	P1	C1	9' - 0"		203 SF
257	CORRIDOR	F1	P1	P1	P1	P1	C4	9' - 0"		406 SF
Grand total: 48										15074 SF

INTERIOR FINISH SCHEDULE KEY

FLOOR

F1	IMPERVIOUS TILE FLOORING
F2	EPOXY RESIN SEAMLESS FIELD APPLIED FLOORING

WALL BASE

B1	VINYL COVE BASE

WALL FINISH

P1	PAINTED GYPSUM WALL BOARD
P2	PLASTER FINISH
G1	EXTERIOR GLASS WALL ASSEMBLY
G2	INTERIOR GLASS WALL ASSEMBLY

CEILING

C1	24X24 ACOUSTICAL CEILING TILE - ARMSTRONG BEVELED TEGULAR 3/16" INTERLUDE DIMENSIONAL TEE SYSTEM WITH 24"X24"X1" OPTIMA OPEN PLAN CEILING TILES (COLOR WHITE)
C2	PAINTED EXPOSED STRUCTURE
C3	PAINTED GYPUM WALL BOARD
C4	ARMSTRONG METALWORKS LINEAR CEILING (OWNER SELECTED COLOR)

WALL MATERIAL

T	TILE
CMU	CONCRETE MASONRY UNIT
CWB	CEMENT WALL BOARD
GWB	MOISTURE & MILDEW RESISTANT GYPSUM WALL BOARD
PNT	PAINTED FINISH
CONC.	CONCRETE
CWA	CURTAIN WALL ASSEMBLY

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NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: Author
CHECKED BY: Checker
APPROVED BY: Approver

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A4.0

SCHEDULES

LICENSED ARCHITECT USA VIRGIN ISLANDS

DOOR SCHEDULE

DOOR MARK	DESCRIPTION	Type Mark	WIDTH	HEIGHT	THICKNESS	FUNCTION	COMMENTS
LEVEL 1							
101	AUTOMATIC SLIDING GLASS DOOR 6'-0" X 7'-0"	09	6' - 0"	6' - 8"		Exterior	
104	METAL SINGLE FLUSH	01	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
105	METAL SINGLE FLUSH	01	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
107	STOREFRONT DOOR ASSEMBLY WITH SIDELITES	11	5' - 6"	7' - 4"	0' - 1 1/2"	Exterior	
108	METAL SINGLE FLUSH WITH LOUVERS	03	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
110A	METAL SINGLE FLUSH	01	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
110B	OVERHEAD COILING DOOR	10	8' - 0"	10' - 0"	0' - 3"	Exterior	
110C	OVERHEAD COILING DOOR	10	8' - 0"	10' - 0"	0' - 3"	Exterior	
110D	METAL SINGLE FLUSH	01	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
LEVEL 2							
201	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
202	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
202-A	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
202-B	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
203-A	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
203-B	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
204	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
205	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
206	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
207	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
208	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
209	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
212	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
213	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
214	GLASS SINGLE FLUSH	04	3' - 0"	6' - 7"		Interior	
215	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
215-O	SINGLE FLUSH	02	2' - 6"	6' - 8"	0' - 2"	Interior	
216	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
219-A	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
220	METAL SINGLE FLUSH WITH TRANSOM EXT	05	3' - 0"	6' - 8"		Interior	
220-A	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
220-B	SINGLE FLUSH	02	2' - 6"	6' - 8"	0' - 1 3/8"	Interior	
220-C	METAL SINGLE FLUSH WITH TRANSOM EXT	05	3' - 0"	6' - 8"		Interior	
221	METAL SINGLE FLUSH WITH TRANSOM EXT	05	3' - 0"	6' - 8"		Interior	
221-A	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
221-B	SINGLE FLUSH	02	2' - 6"	6' - 8"	0' - 2"	Interior	
221-C	METAL SINGLE FLUSH WITH TRANSOM EXT	05	3' - 0"	6' - 8"		Interior	
222	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
223	METAL SINGLE FLUSH WITH TRANSOM EXT	05	3' - 0"	6' - 8"		Interior	
224	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
228	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
229	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
232	SINGLE FLUSH WITH FULL LITE	06	3' - 0"	6' - 8"	0' - 1 3/4"	Exterior	
237	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
238	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
239	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
240	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
241	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
247	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
248	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
249	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
250	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
251	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
252	SINGLE FLUSH WITH FULL LITE	06	3' - 0"	6' - 8"	0' - 1 3/4"	Exterior	
253	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
254	SINGLE FLUSH WITH SIDE LITE	07	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
255	SINGLE FLUSH WITH (2) SIDE LITES	08	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	
256	SINGLE FLUSH 36" X 80" INT	02	3' - 0"	6' - 8"	0' - 1 3/8"	Interior	
LEVEL 3							
300	METAL SINGLE FLUSH	01	3' - 0"	6' - 8"	0' - 1 3/4"	Interior	

DOOR SCHEDULE NOTES

1. CONTRACTOR SHALL VERIFY DOOR SIZES IN FIELD PRIOR TO ORDERING ANY MATERIALS.
2. ALL DOORS TO RECEIVE A STAINED FINISH TO MATCH EXSITING DOORS.

WINDOW SCHEDULE

TYPE	DESCRIPTION	SIZE		HEADER HEIGHT A.F.F.	MATERIAL	REMARKS
		WIDTH	HEIGHT			
A	HOPPER WINDOW	4' - 0"	6' - 0"			IMPACT RESISTANT GLASS
B	HOPPER WINDOW	4' - 0"	2' - 0"			IMPACT RESISTANT GLASS
C	TRANSACTION WINDOW	3' - 0"	4' - 0"			
D	TRANSACTION WINDOW	3' - 0"	5' - 0"			TRANSACTION WINDOW ADA HEIGHT REQUIRMENT
E	FIXED WINDOW	4' - 0"	4' - 0"			IMPACT RESISTANT GLASS
F	FIXED WINDOW	3' - 6"	5' - 0"			IMPACT RESISTANT GLASS
G	FIXED WINDOW	4' - 0"	5' - 0"			IMPACT RESISTANT GLASS

WINDOW SCHEDULE NOTES:

1. CONTRACTOR SHALL VERIFY NEW ROUGH OPENING DIMENSIONS PRIOR TO ORDERING WINDOWS AND SHUTTERS.
2. CONTRACTOR SHALL PROVIDE INSECT SCREENS FOR ALL OPENINGS.

REVISIONS

NO.	DATE	REMARKS

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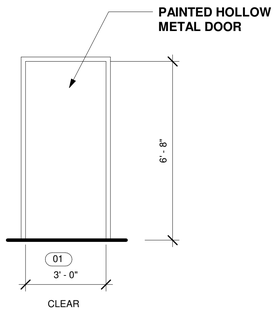
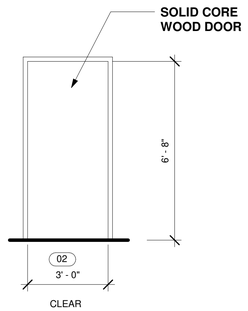
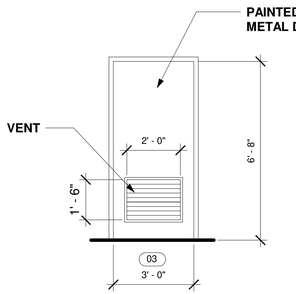
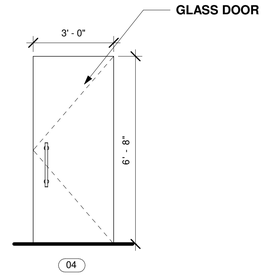
SCHEDULES

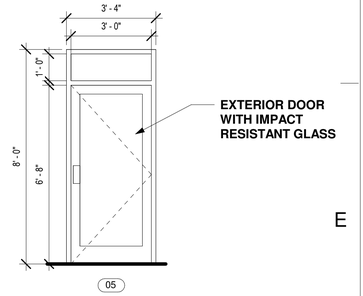
REGISTERED ARCHITECT U.S. VIRGIN ISLANDS

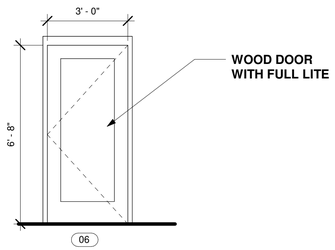


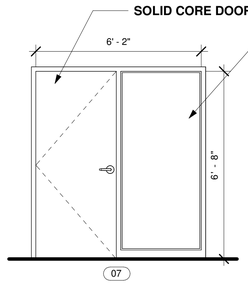


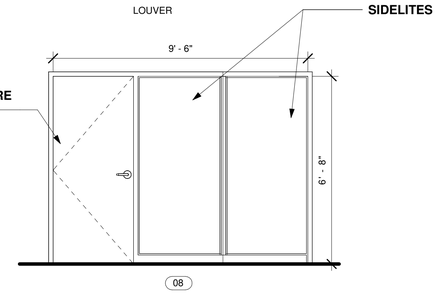


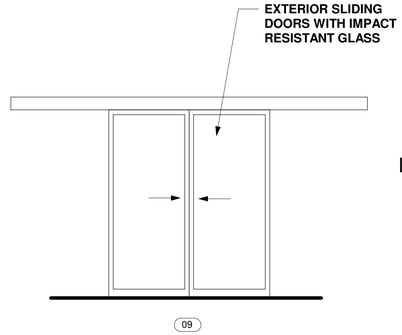


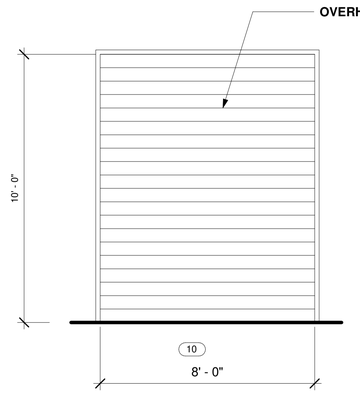


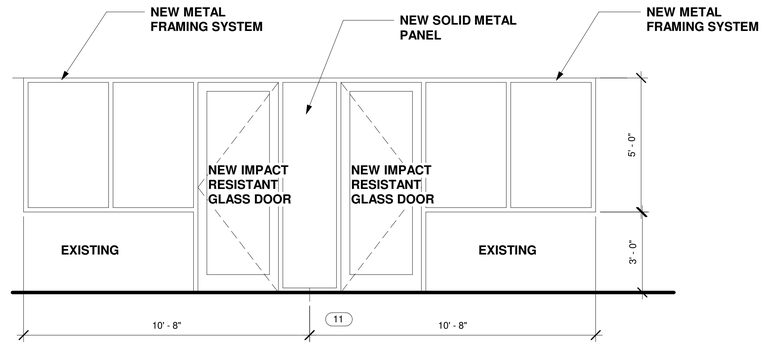




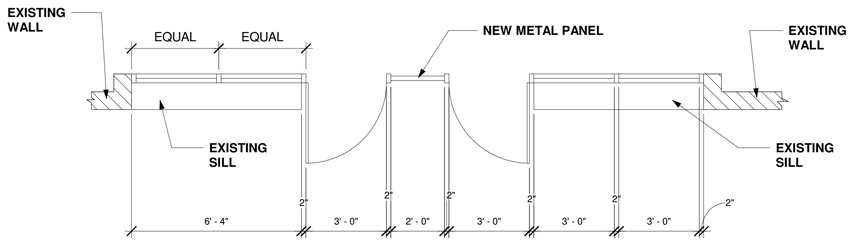








STORE FRONT ELEVATION



PLAN DETAIL

REVISIONS

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St. Thomas, U.S. Virgin Islands 00804-6218
telephone: 340.777.1600
e-mail: admin@jaredian.com
website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	Author
CHECKED BY:	Checker
APPROVED BY:	Approver

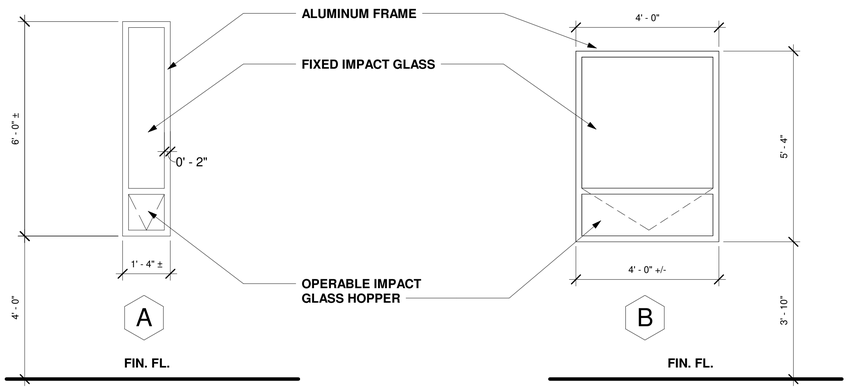
PROJECT NO.: 21-003.C

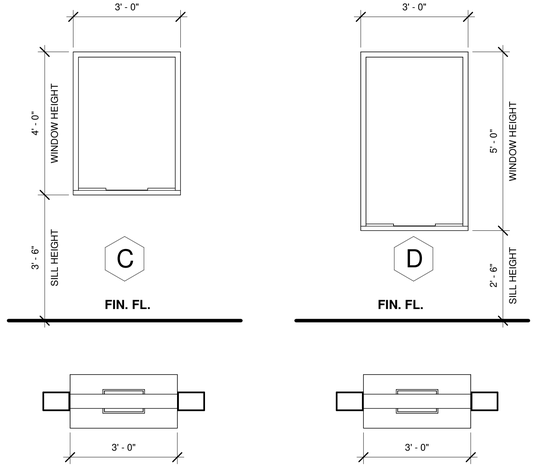
CONTRACT NO.: -

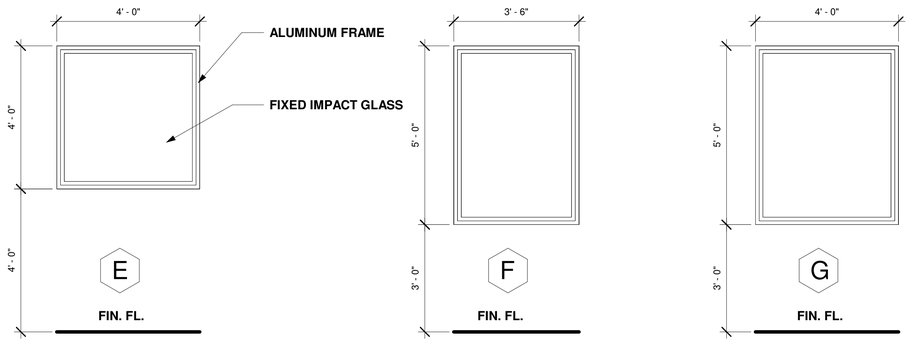
SHEET NO.: A4.2

DOOR ELEVATIONS

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS







REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: R.O.P

CHECKED BY: J.P.W

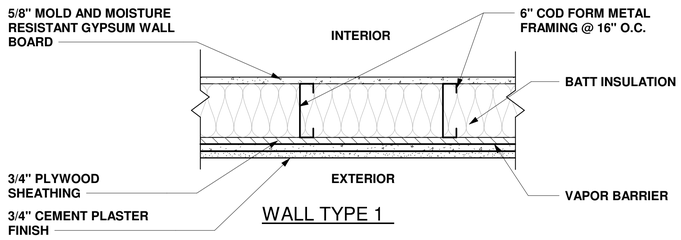
APPROVED BY: J.P.W.

PROJECT NO.: 21-003.C

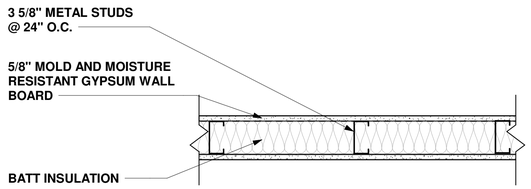
CONTRACT NO.: -

SHEET NO.: A4.3

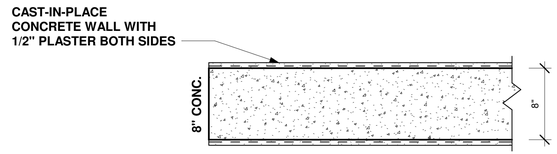
WINDOW ELEVATIONS



WALL TYPE 1



WALL TYPE 2



WALL TYPE 3

1 WALL DETAILS

1 1/2" = 1'-0"

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ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: R.O.P

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APPROVED BY: J.P.W.

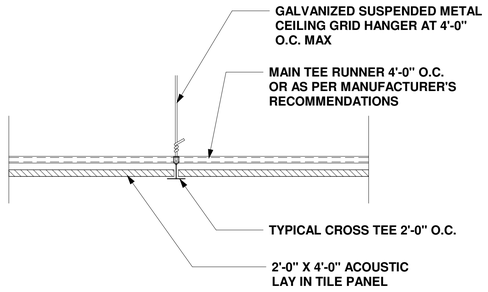
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A4.4

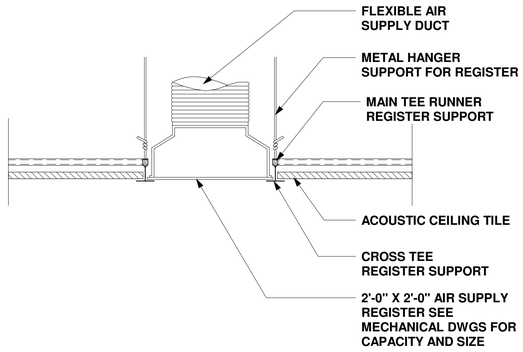
WALL DETAILS

LICENSED ARCHITECT U.S.V.I. U.S.A.



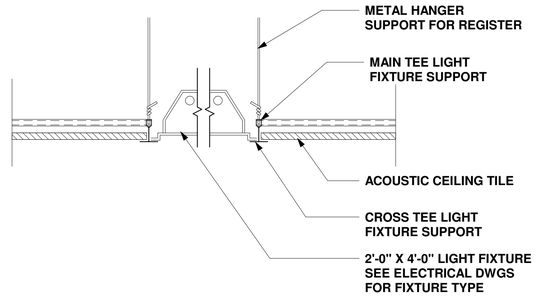
1 TYPICAL SUSPENDED CEILING DETAIL

1 1/2" = 1'-0"



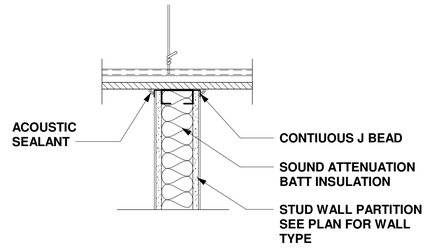
2 CEILING DETAIL AT REGISTER

1 1/2" = 1'-0"



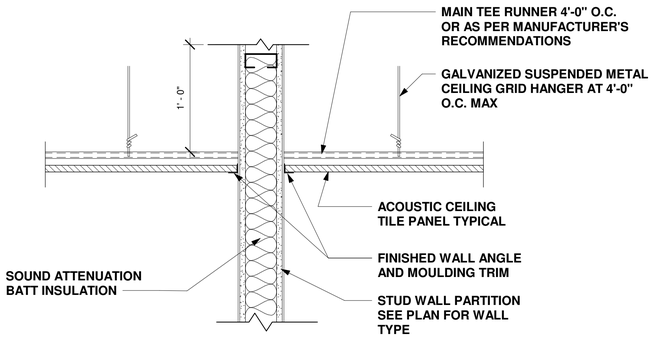
3 CEILING DETAIL AT LIGHT

1 1/2" = 1'-0"



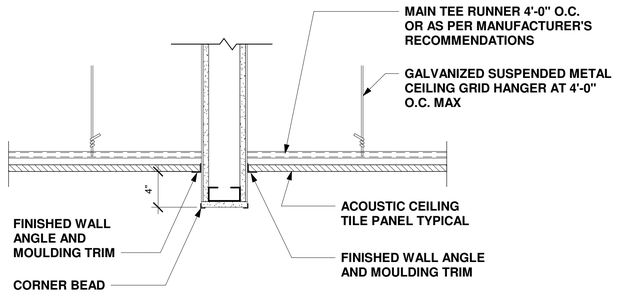
4 CEILING DETAIL 1

1 1/2" = 1'-0"



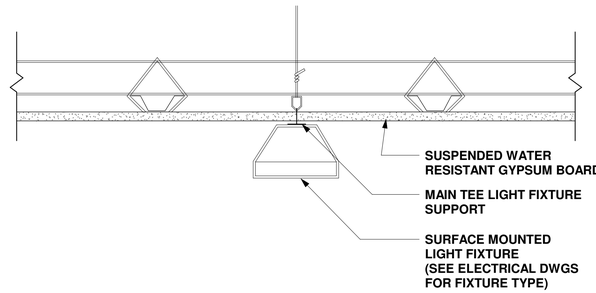
5 CEILING DETAIL 2

1 1/2" = 1'-0"



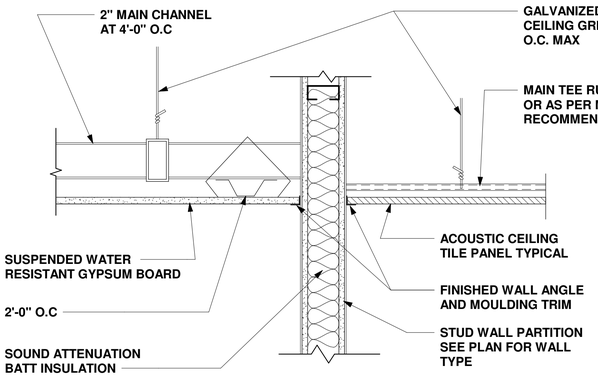
6 BULKHEAD DETAIL

1 1/2" = 1'-0"



7 CEILING DETAIL AT LIGHT FIXTURE

1 1/2" = 1'-0"



8 CEILING DETAIL 3

1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
DRAWN BY: Author
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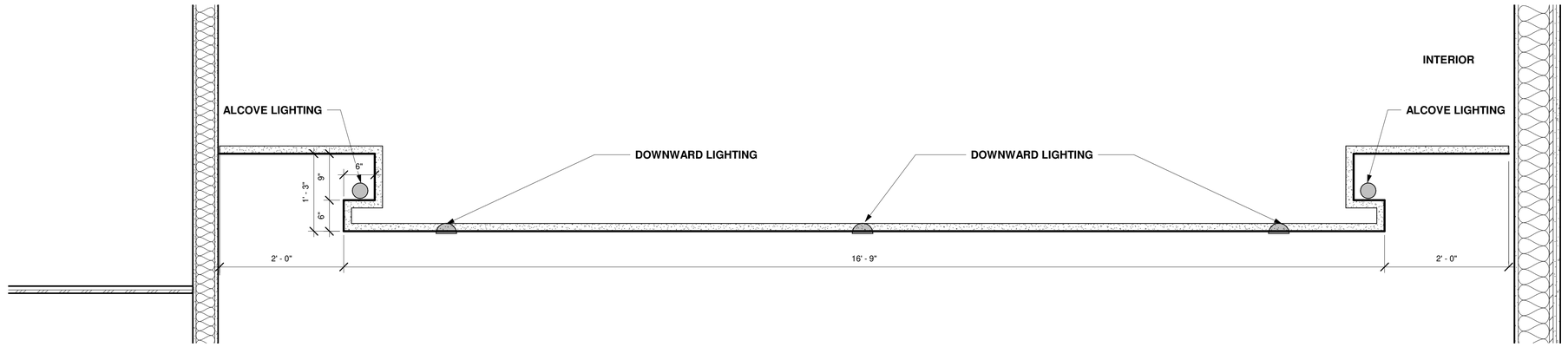
APPROVED BY: Approver

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A5.0

CEILING DETAILS



1 CONFERENCE ROOM CEILING

1" = 1'-0"

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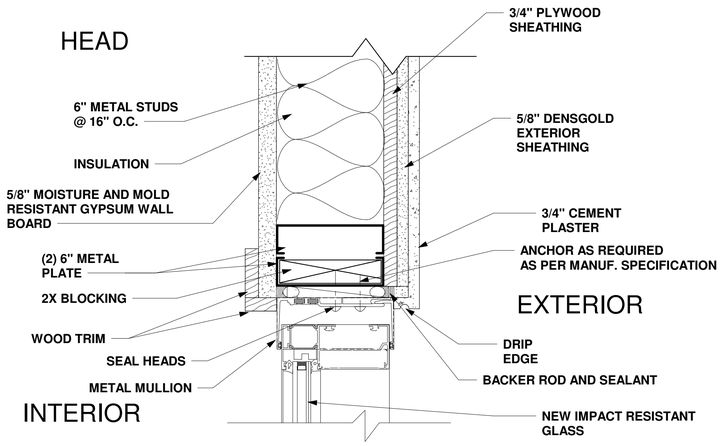
PROJECT NO.: 21-003.C

CONTRACT NO.: -

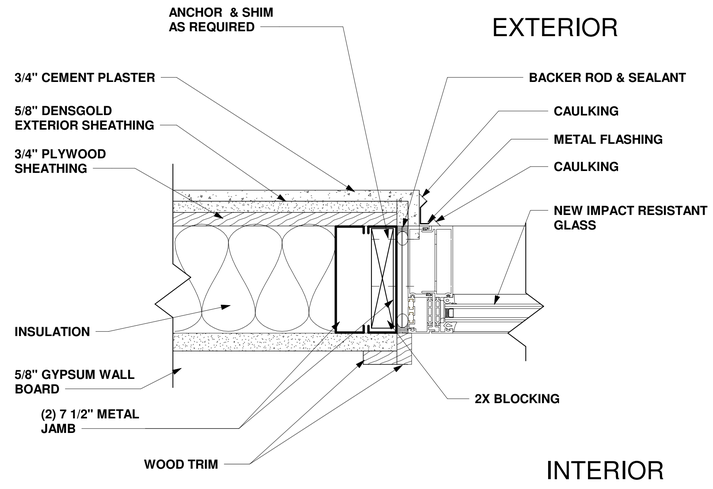
SHEET NO.: A5.1

CEILING DETAILS

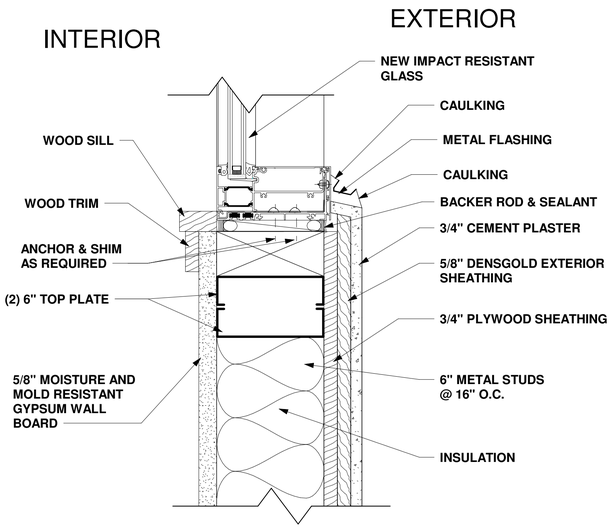
LICENSED ARCHITECT USA VIRGIN ISLANDS



1 WINDOW HEAD DETAIL
3" = 1'-0"



2 WINDOW JAMB DETAIL
3" = 1'-0"



3 WINDOW SILL DETAIL
3" = 1'-0"

REVISIONS

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CONSULTANTS

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ST. THOMAS, USVI

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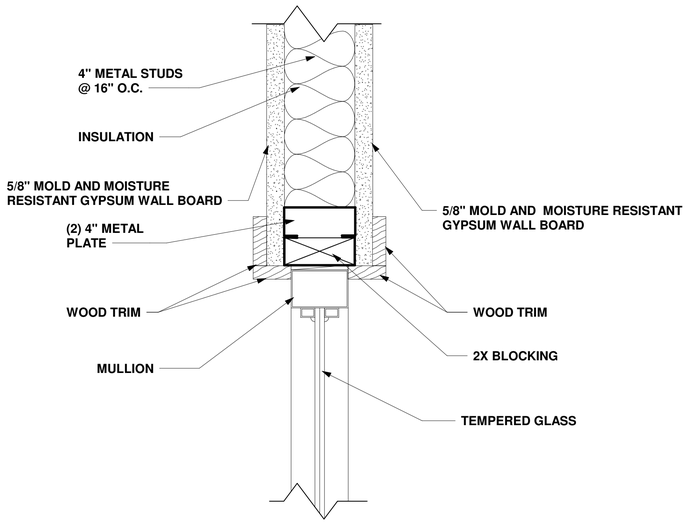
PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A5.3

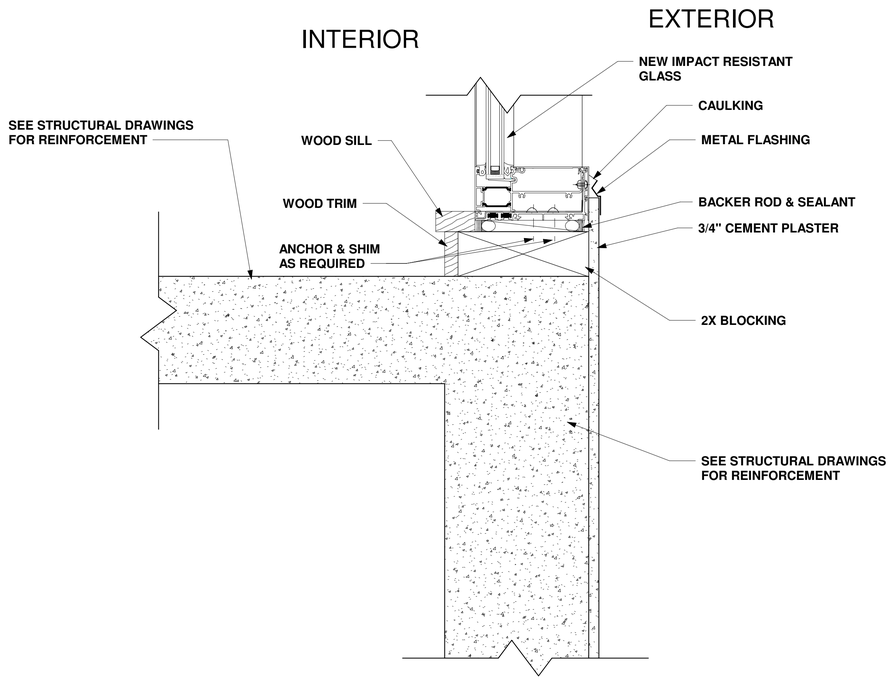
WINDOW DETAILS

REGISTERED ARCHITECT U.S. VIRGIN ISLANDS



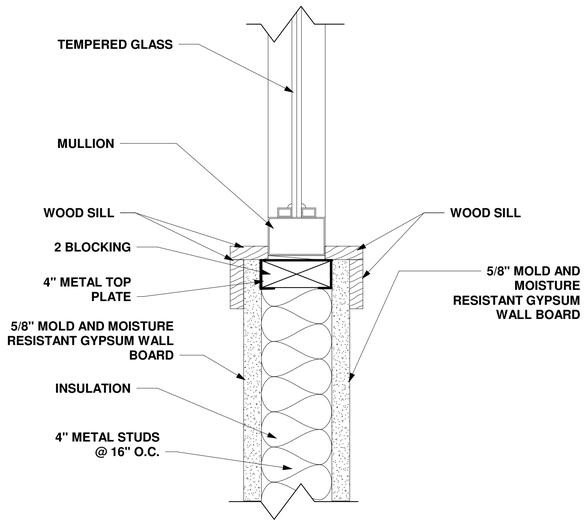
1 WINDOW HEAD DETAIL 3

3" = 1'-0"



3 CURTAIN WALL DETAIL4

3" = 1'-0"



2 WINDOW SILL DETAIL 3

3" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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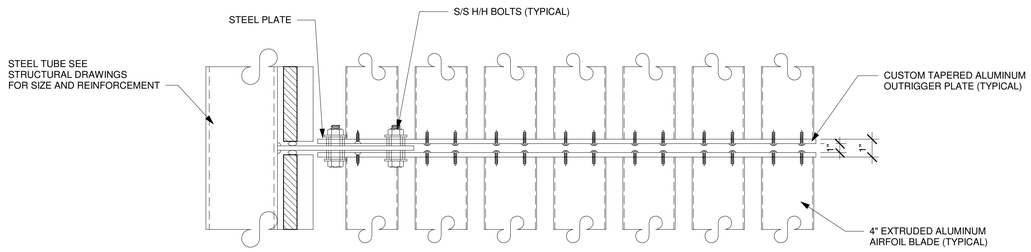
CONTRACT NO.: -

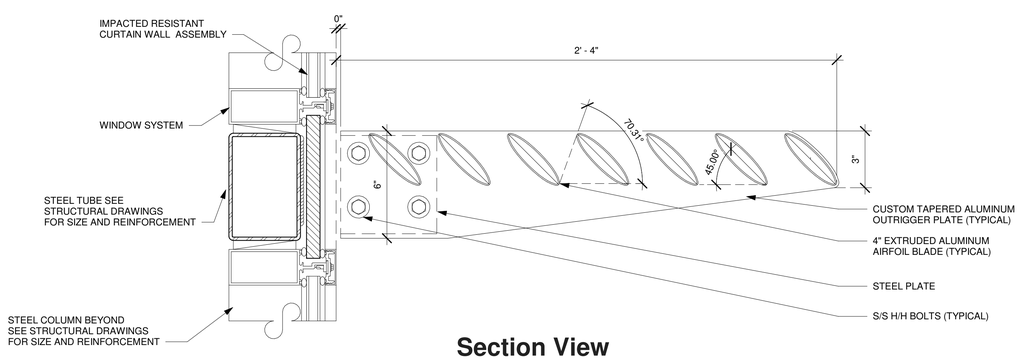
SHEET NO.: A5.4

WINDOW DETAILS

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



Plan View



Section View

Sunshade Design must meet local code requirements. Please contract CS Representative in your area for assistance.

1 AIRFLOW SUNSHADE DETAIL 2
NOT TO SCALE

CONSTRUCTION SPECIALTIES
49 MEEKER AVENUE
CRANFORD, N.J. 07016-3198

TITLE: CANTILEVER AIRFLOIL SUNSHADE SYSTEM
REF. 100-3C-32

CONTRACTOR CAN UTILIZE APPROVED SUBSTITUTION

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
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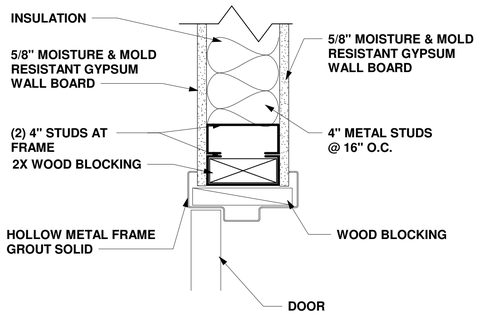
PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A5.5

WINDOW DETAILS

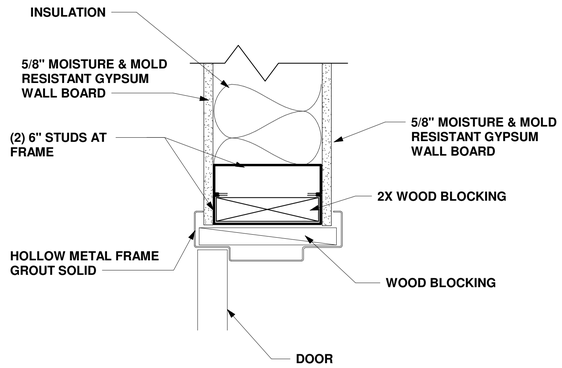
REGISTERED ARCHITECT U.S. VIRGIN ISLANDS



DOOR HEAD DETAIL J1
JAMB WITH 4" STUD WALL

1 DOOR HEAD DETAILS 2

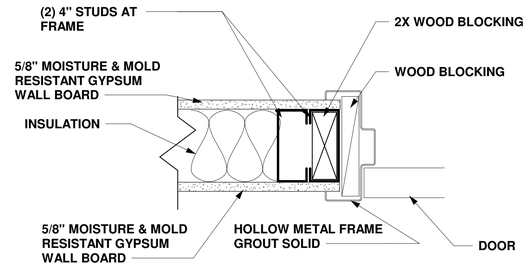
3" = 1'-0"



DOOR HEAD DETAIL J2
JAMB WITH 6" STUD WALL

2 DOOR HEAD DETAILS 3

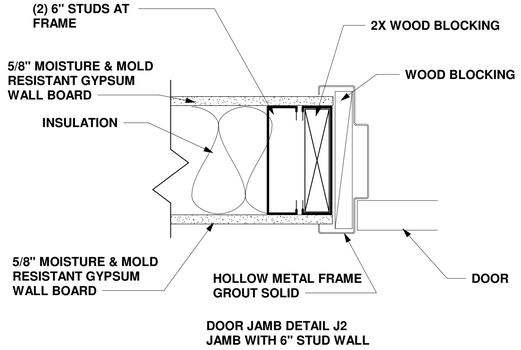
3" = 1'-0"



DOOR JAMB DETAIL J1
JAMB WITH 4" STUD WALL

3 DOOR JAMB DETAILS 2

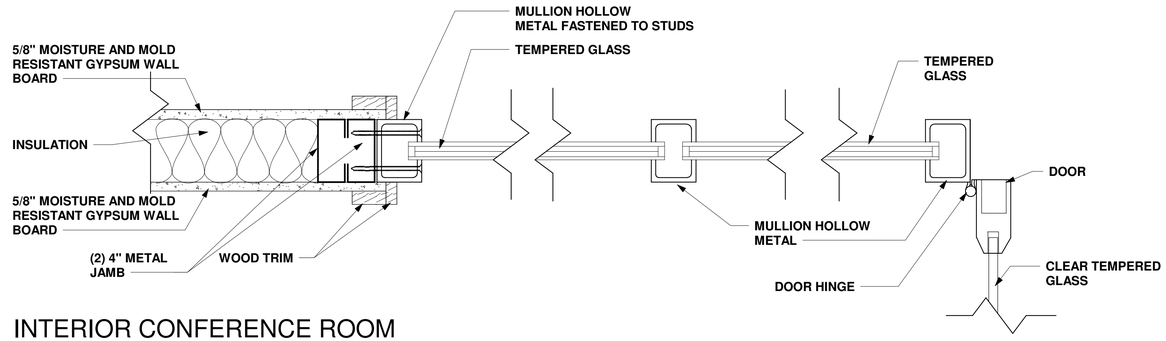
3" = 1'-0"



DOOR JAMB DETAIL J2
JAMB WITH 6" STUD WALL

4 DOOR JAMB DETAILS 3

3" = 1'-0"



INTERIOR CONFERENCE ROOM CURTAIN WALL ASSEMBLY

5

3" = 1'-0"

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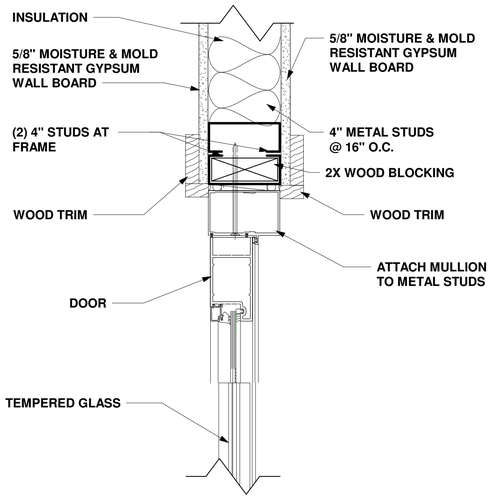
PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A5.6

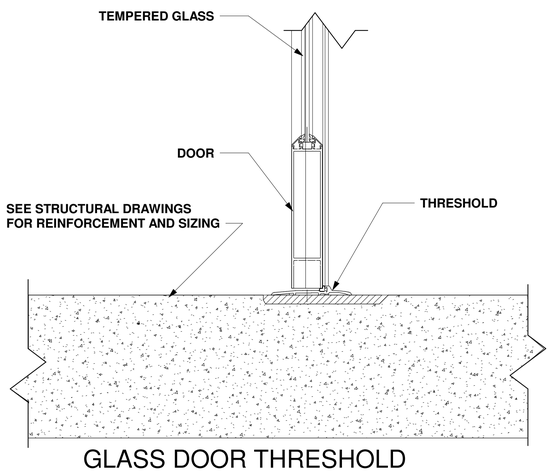
DOOR DETAILS

REGISTERED ARCHITECT
U.S. VIRGIN ISLANDS



1 GLASS DOOR HEAD DETAILS

3" = 1'-0"



2 GLASS DOOR THRESHOLD DETAILS

3" = 1'-0"

REVISIONS

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CONSULTANTS

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ST. THOMAS, USVI

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CONTRACT NO.: -

SHEET NO.: A5.7

DOOR DETAILS

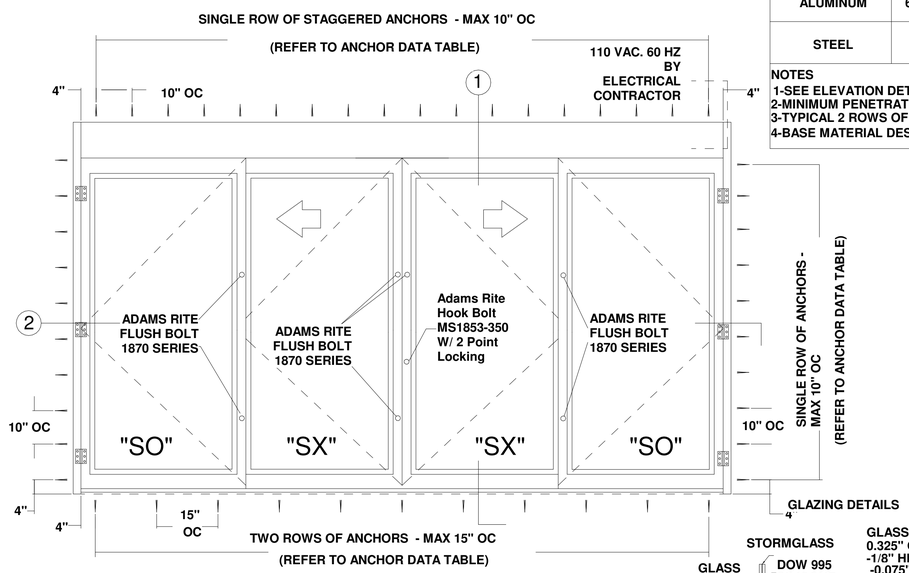
LICENSED ARCHITECT ST. THOMAS & ST. JOHN VIRGIN ISLANDS USA

REV	DESCRIPTION	BY	DATE
B	UPDATED PACKAGE WITH DATA	JESC	06/24/2004
C	ADDED STEEL TUBE TO STILES	JESC	04/22/2005
D	RELEASED	JESC	10/17/2005
E	UPDATED ADDRESS, ADDED NEW THRESHOLD, CHANGED DRAWING NUMBER FROM US80021	DSW	08/08/2011
F	NOA REF. # CHANGED	DSW	02/20/2013
G	NEW NOA#, REVISED ANCHOR AND GLASS SPEC.	DSW	05/02/2016
H	REPLACED NOA# WITH FL#	DSW	02/01/2019

ANCHOR DATA

SUBSTRATE	MINUMUM REQUIREMENTS	EXPLODE	PENETRATION	EDGE DISTANCE
CONCRETE	3000 PSI	HEADER JAMBS SILL	1 1/2" MIN	2 1/2" MIN
WOOD	SG >= 0.55	HEADER JAMBS SILL	1 1/2" MIN	3/4" MIN
ALUMINUM	6063-T5 - 1/8" THICK	HEADER JAMBS	N/A	3/4" MIN
STEEL	A36 - 1/8" THICK Fu = 58 ksI	HEADER JAMBS	N/A	3/4" MIN

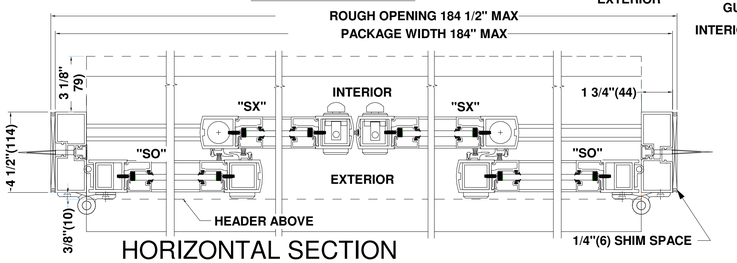
NOTES
1-SEE ELEVATION DETAILS FOR ANCHOR SPACING
2-MINIMUM PENETRATION IS BEYOND WALL DRESSING
3-TYPICAL 2 ROWS OF FASTENERS FOR THRESHOLD INSTALLATION
4-BASE MATERIAL DESIGNED BY OTHERS TO WITHSTAND IMPOSED LOAD



ELEVATION

NOTE:

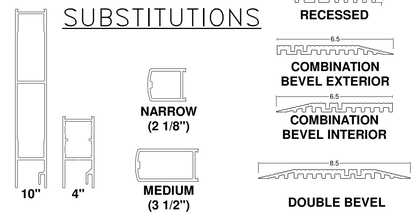
USE TORMAX USA INC. FOR AUTOMATIC SLIDING DOOR OR SIMILAR



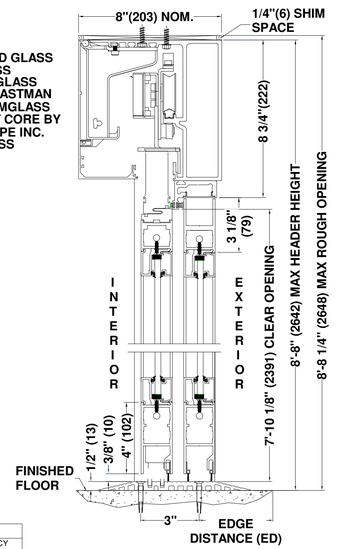
HORIZONTAL SECTION

GLASS SPECIFICATION
0.325" OLDCASTLE IMPACT RATED GLASS
-1/8" HEAT STRENGTHENED GLASS
-0.075" SAFLEX CP COMPOSITE GLASS INTERLAYER W/PET CORE BY EASTMAN CHEMICAL CO. OR 0.075" STORMGLASS SAFLEX CP INTERLAYER W/PET CORE BY OLDCASTLE BUILDING ENVELOPE INC.
-1/8" HEAT STRENGTHENED GLASS

AVAILABLE SUBSTITUTIONS



BOTTOM RAILS VERTICALS THRESHOLDS



VERTICAL SECTION

DOOR ALLOWABLE DESIGNED PRESSURES

+55*, -55 PSF

LARGE MISSILE IMPACT

ILLUSTRATION "B"

OVERHANG MUST BE GREATER OR EQUAL TO FLOOR TO CEILING DIM.

FLOOR TO CEILING DOOR HEIGHT

OVERHANG

* PRODUCT CAN BE USED ONLY IN AREAS WHERE THE DOOR DESIGN IS NOT LIMITED BY WATER INFILTRATION OR MEETING THE CRITERIA AS PER ILLUSTRATION "B"

PACKAGE HEIGHT INFORMATION

MAX PACKAGE HEIGHT	MASONRY OPENING	NOM. CLEAR DOOR OPENING
8'-8" (104")	8'-8 1/4" (104 1/4")	94 1/8"

PACKAGE WIDTH INFORMATION

MAX PACKAGE WIDTH	MASONRY OPENING	NOM. CLEAR DOOR OPENING	SLIDING DOOR PANEL NOM. WIDTH	EMERGENCY BREAKOUT NOM. WIDTH
15'-4" (184")	15'-4 1/2" (184 1/2")	77 3/4"	46 7/8"	171"

DESIGN INTENT

TORMAX INC.
12859 WETMORE RD.
SANTONIO TEXAS 78247

TITLE: 9300 IMPACT, BI-PART
REF. FL# 17648.3

CONTRACTOR CAN UTILIZE APPROVED SUBSTITUTION

1 SLIDING GLASS DOOR DETAILS
NOT TO SCALE

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

Jaredian Design Group
Architects, Engineers and Construction Managers
5143 Palm Passage,
Second Floor - Suites B24-25 St. Thomas,
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website: www.jaredian.com

PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

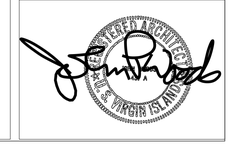
NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

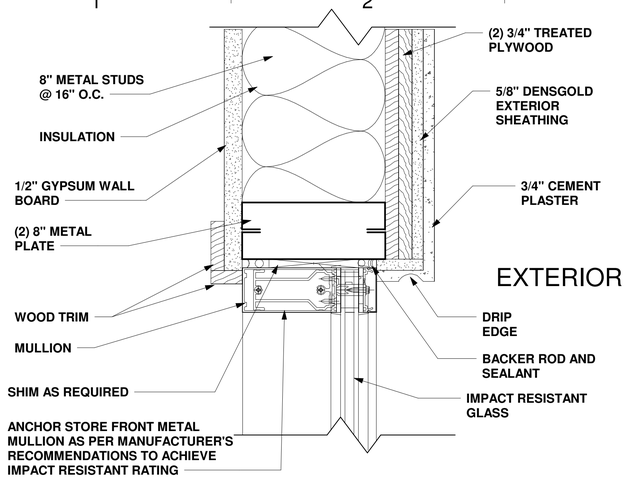
DATE: 02.14.2025
DRAWN BY: Author
CHECKED BY: Checker
APPROVED BY: Approver

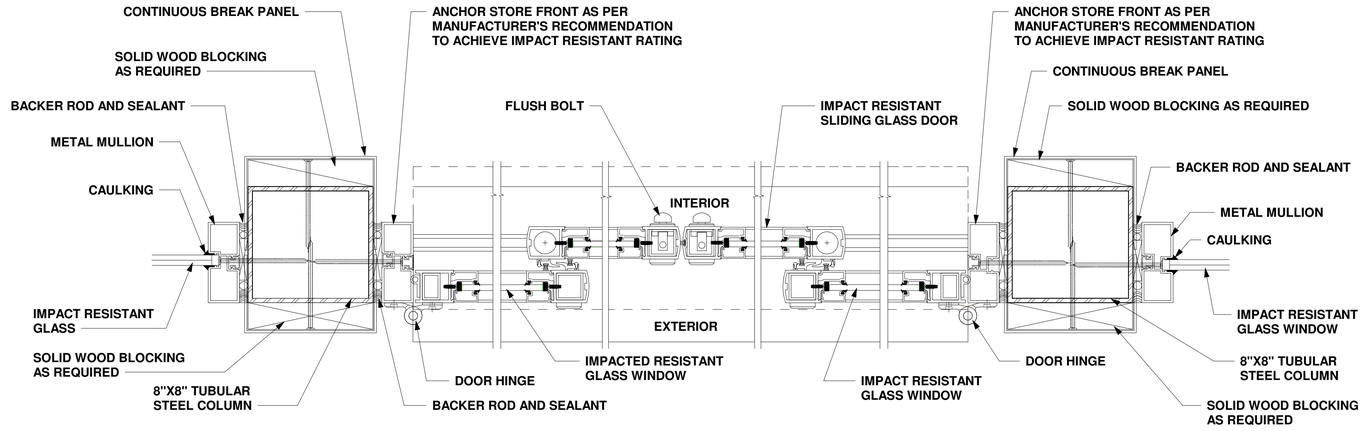
PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A5.8

DOOR DETAILS

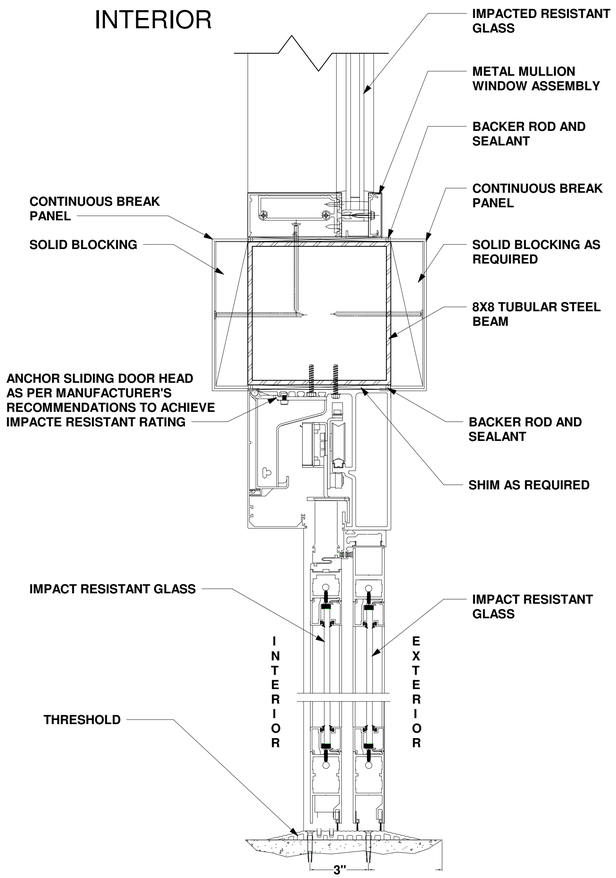






SLIDING GLASS DOOR JAMB DETAILS

2 NOT TO SCALE



SLIDING GLASS DOOR HEAD DETAILS

1 NOT TO SCALE

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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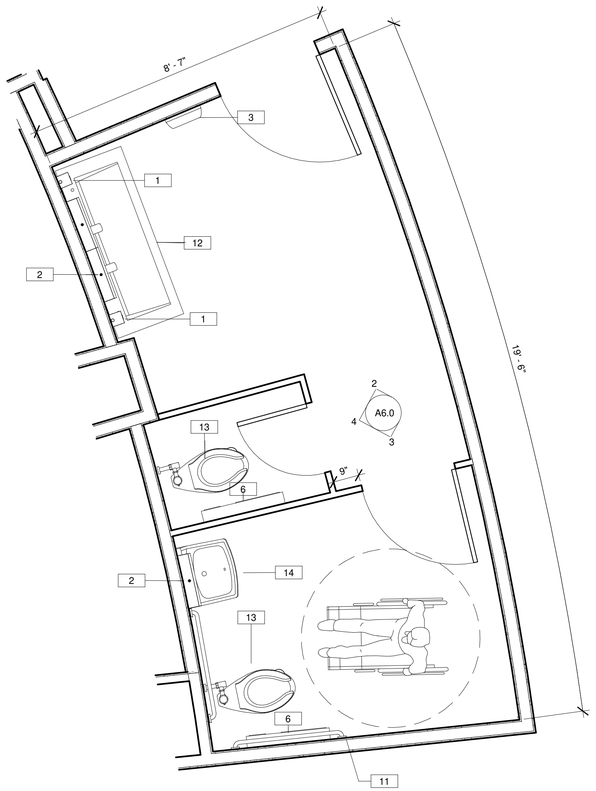
PROJECT NO.: 21-003.C

CONTRACT NO.: -

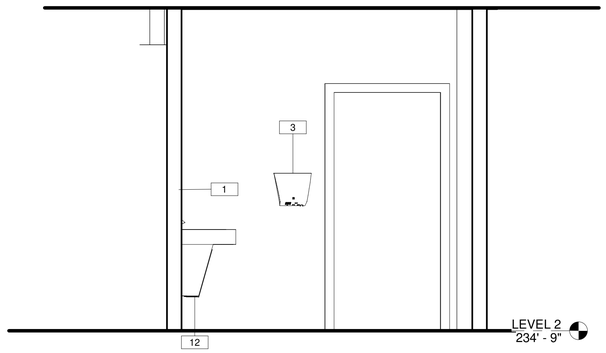
SHEET NO.: A5.9

DOOR DETAILS

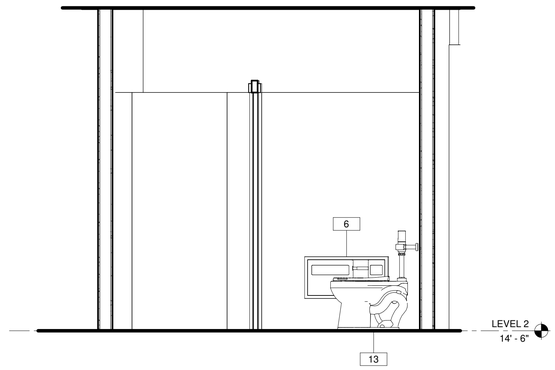
LICENSED ARCHITECT USA VIRGIN ISLANDS



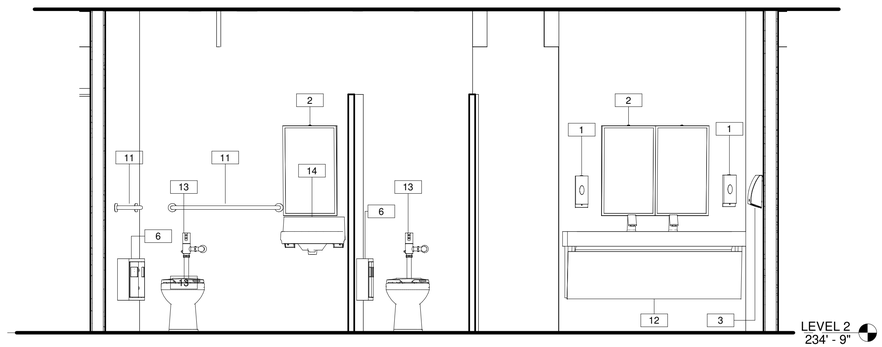
1 LEVEL 2 - ENLARGED FEMALE RESTROOM
1/2" = 1'-0"



2 FEMALE RESTROOM ELEVATION -1
1/2" = 1'-0"



3 FEMALE RESTROOM ELEVATION - 2
1/2" = 1'-0"



4 FEMALE RESTOOM ELEVATION - 3
1/2" = 1'-0"

RESTROOM ACCESSORIES LEGEND

1	SOAP DISPENSER
2	TILTED MIRROR
3	HAND DRYER
6	COMBINATION TOILET SEAT COVER DISPENSER, TOILET PAPER DISPENSER, WASTE RECEPTACLE
11	ABA COMPLIANT GRAB BAR
12	LAVATORY ABA COMPLIANT LAVATORY
13	ABA COMPLIANT FLOOR MOUNTED TOILET
14	ABA COMPLIANT WALL HUNG SINK

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

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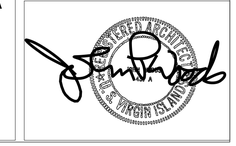
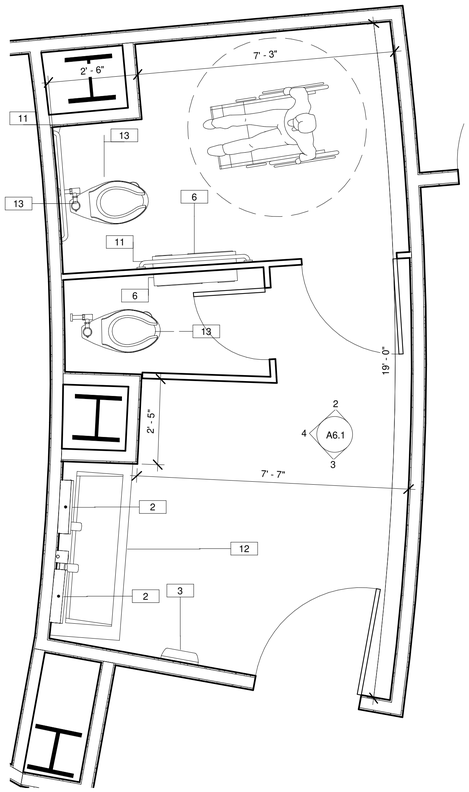
CONTRACT NO.: -

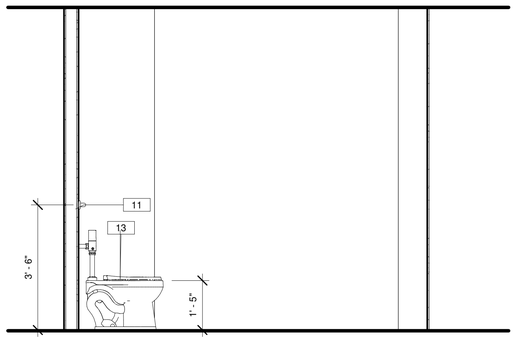
SHEET NO.: A6.0

ENLARGED RESTROOM PLANS

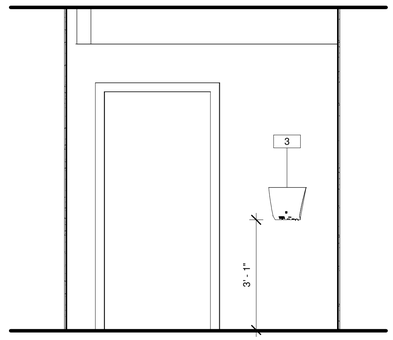




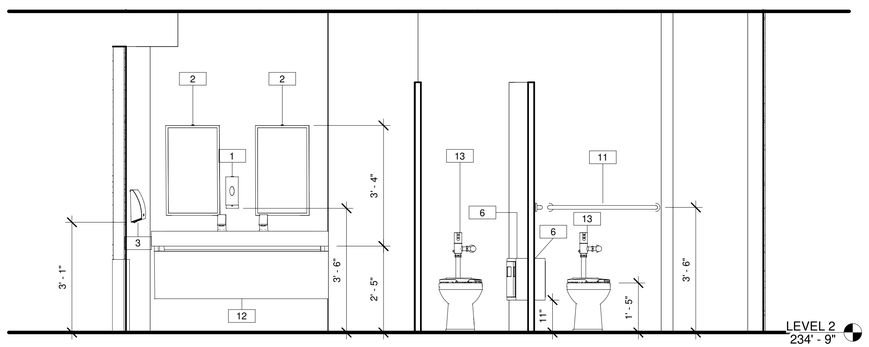
1 LEVEL 2 - ENLARGED MALE RESTROOM
1/2" = 1'-0"



2 MALE RESTROOM ELEVATION - 1
1/2" = 1'-0"



3 MALE RESTROOM ELEVATION -2
1/2" = 1'-0"



4 MALE RESTROOM ELEVATION - 3
1/2" = 1'-0"

RESTROOM ACCESSORIES LEGEND

1	SOAP DISPENSER
2	TILTED MIRROR
3	HAND DRYER
6	COMBINATION TOILET SEAT COVER DISPENSER, TOILET PAPER DISPENSER, WASTE RECEPTACLE
11	ABA COMPLIANT GRAB BAR
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REVISIONS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025
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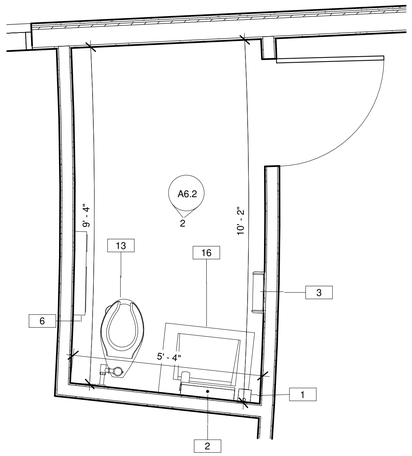
APPROVED BY: Approver

PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A6.1

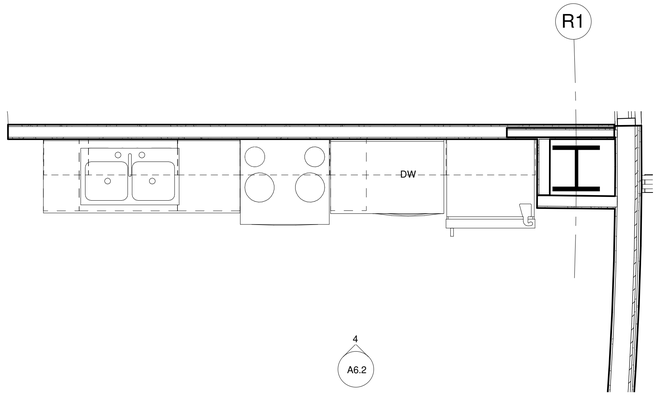
ENLARGED RESTROOM PLANS

LICENSED ARCHITECT USA VIRGIN ISLANDS



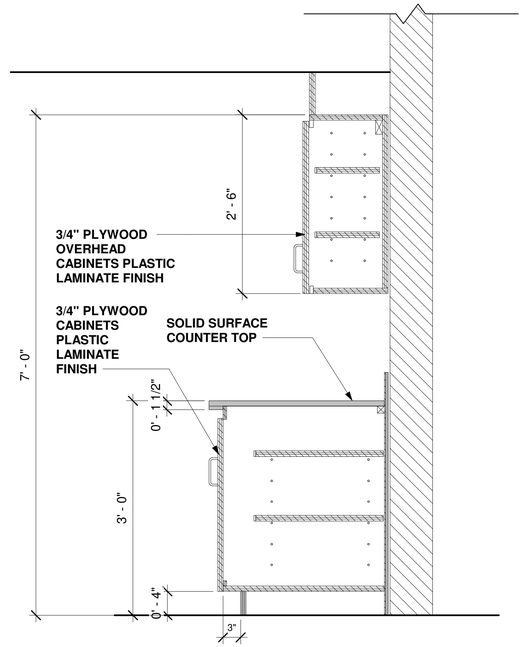
LEVEL 2 - ENLARGED CEO RESTROOM

1 1/2" = 1'-0"



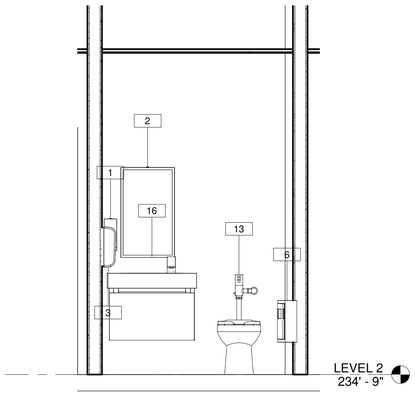
LEVEL 2 - ENLARGED KITCHEN PLAN

3 1/2" = 1'-0"



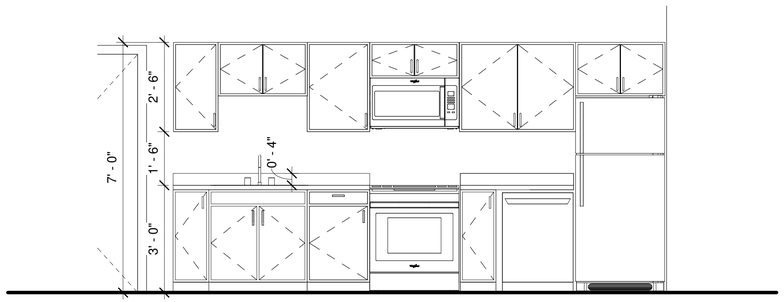
CASEWORK SECTION

5 1" = 1'-0"



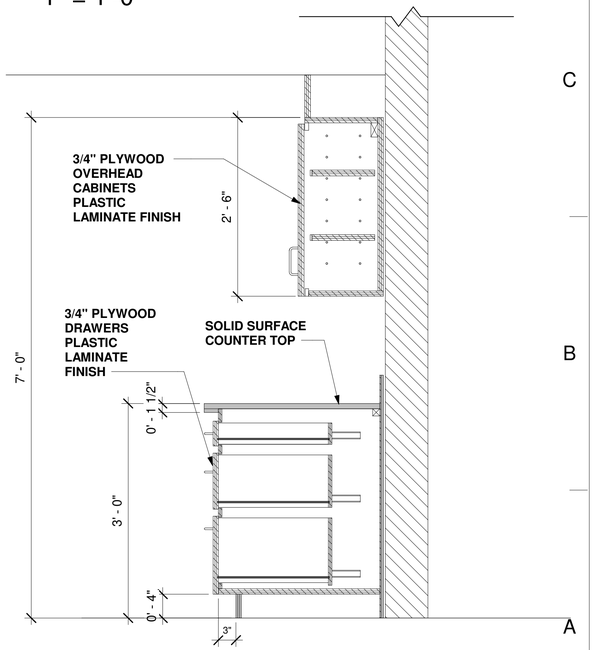
CEO RESTROOM ELEVATION

2 1/2" = 1'-0"



KITCHEN ELEVATION

4 1/2" = 1'-0"



CASEWORK SECTION -DRAWERS

6 1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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Second Floor - Suites B24-25 St. Thomas,
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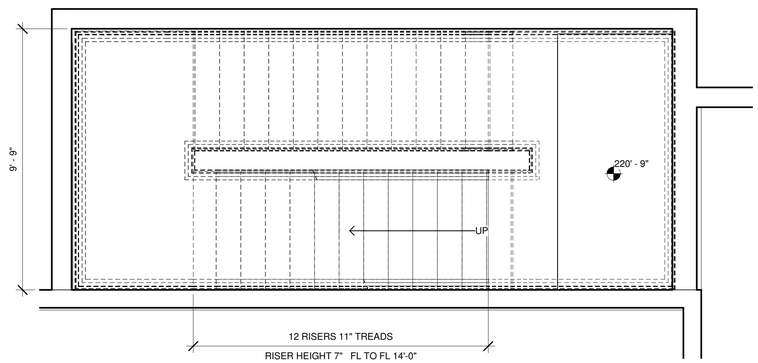
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY
INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	Author
CHECKED BY:	Checker
APPROVED BY:	Approver
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A6.2

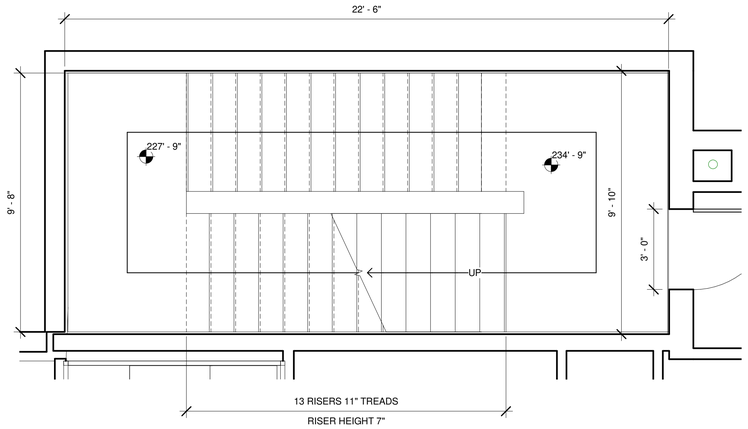
ENLARGED PLANS AND ELEVATIONS

REGISTERED ARCHITECT U.S. VIRGIN ISLANDS



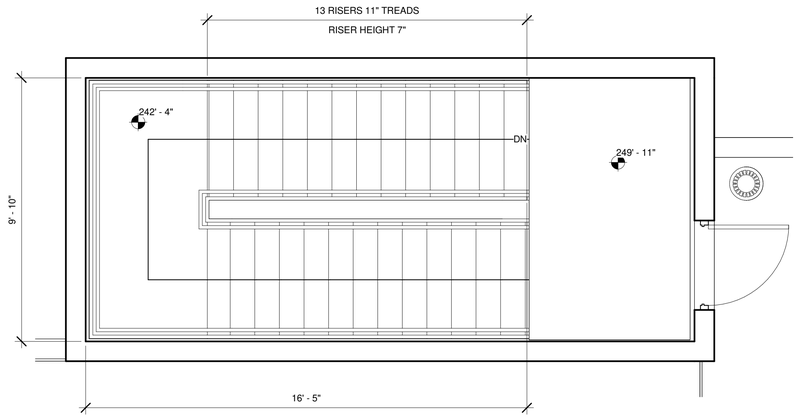
LEVEL 1 - ENLARGED STAIR TOWER

1 — 3/8" = 1'-0"



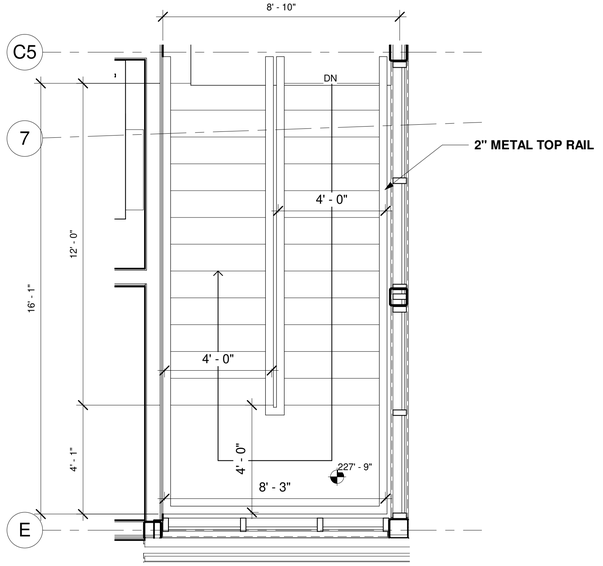
LEVEL 2 - ENLARGED STAIR TOWER

2 — 3/8" = 1'-0"



LEVEL 3 - ENLARGED STAIR TOWER

3 — 3/8" = 1'-0"



LEVEL 2 - ENLARGED MAIN STAIR

4 — 3/8" = 1'-0"

REVISIONS

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CONSULTANTS

Jaredian Design Group
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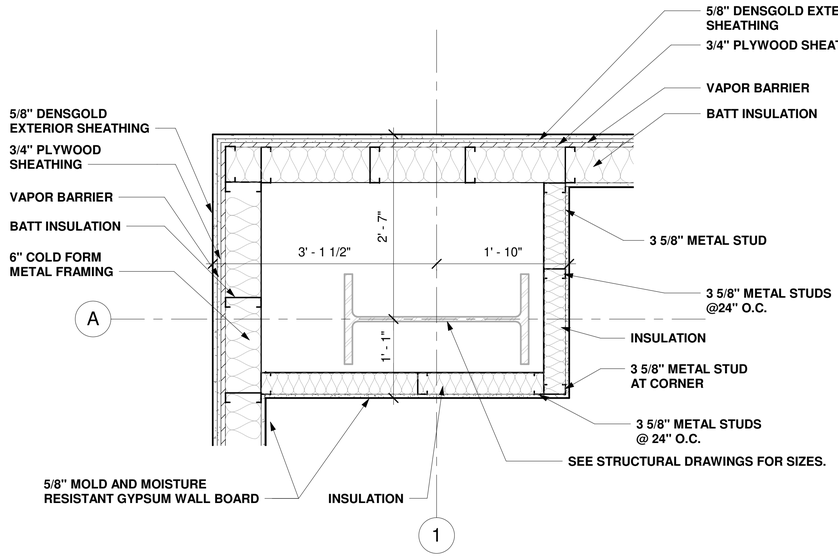
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	R.O.P
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.0

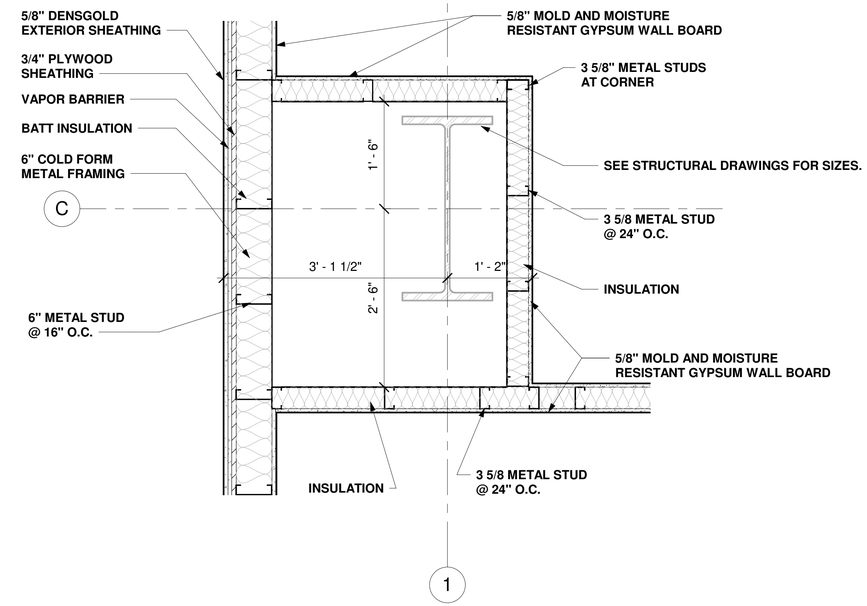
ENLARGED STAIR PLAN

REGISTERED ARCHITECT — U.S. VIRGIN ISLANDS



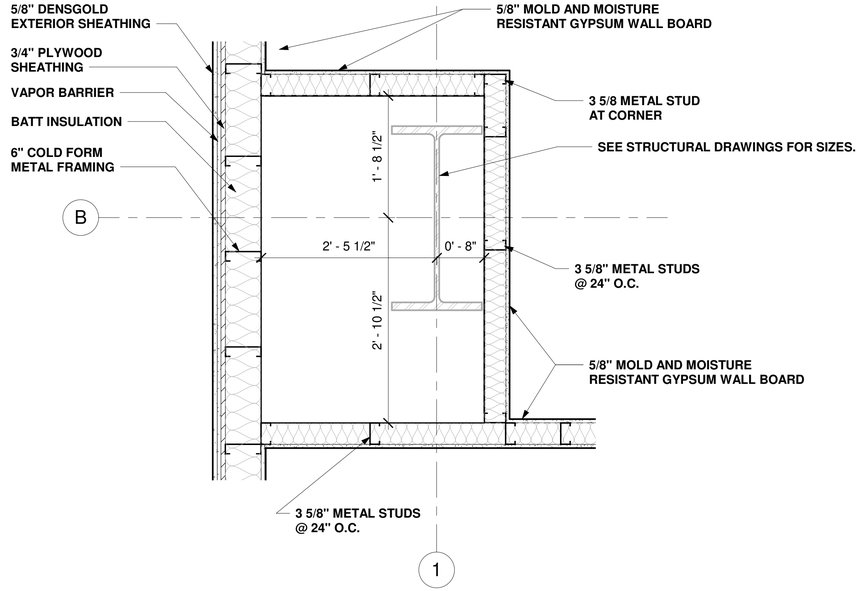
1 LEVEL 2 - PLAN DETAIL - A AND 1

1" = 1'-0"



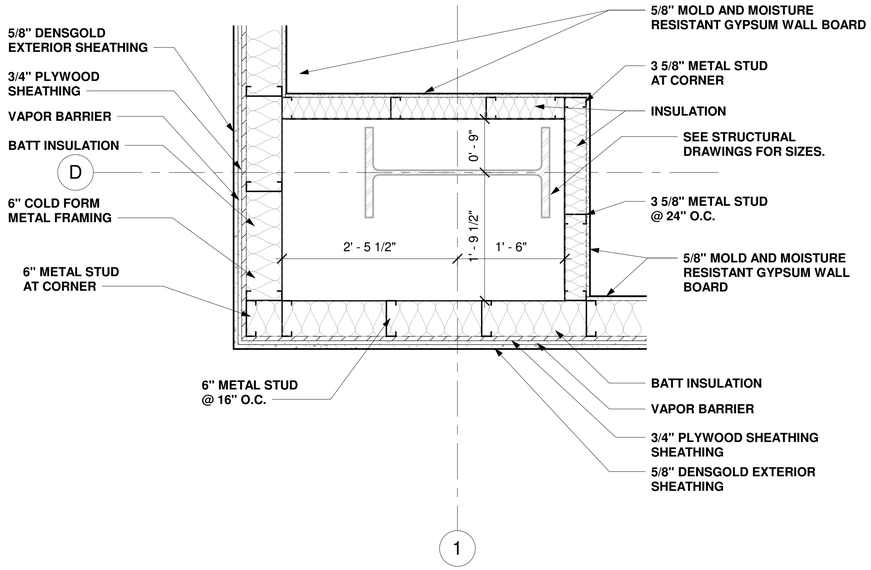
3 LEVEL 2 - PLAN DETAIL - C AND 1

1" = 1'-0"



2 LEVEL 2 - PLAN DETAIL - B AND 1

1" ₸ 1'-0"



4 LEVEL 2 - PLAN DETAIL - D AND 1

1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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DATE:	02.14.2025
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CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.

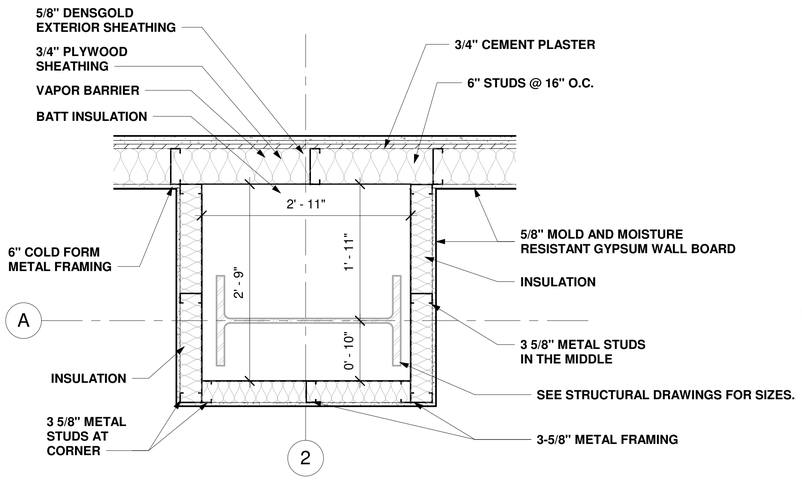
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A7.1

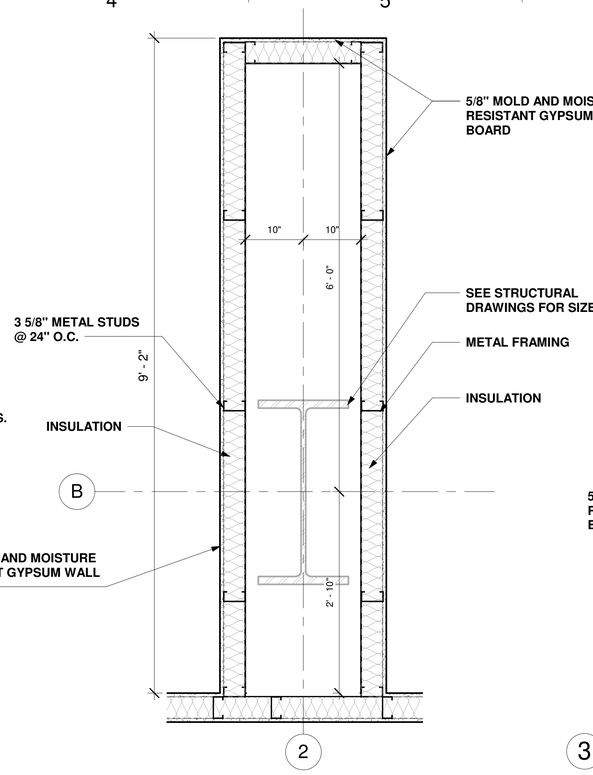
ENLARGED COLUMN ENCLOSURES

LICENSED ARCHITECT J.P. WARD USA VIRGIN ISLANDS



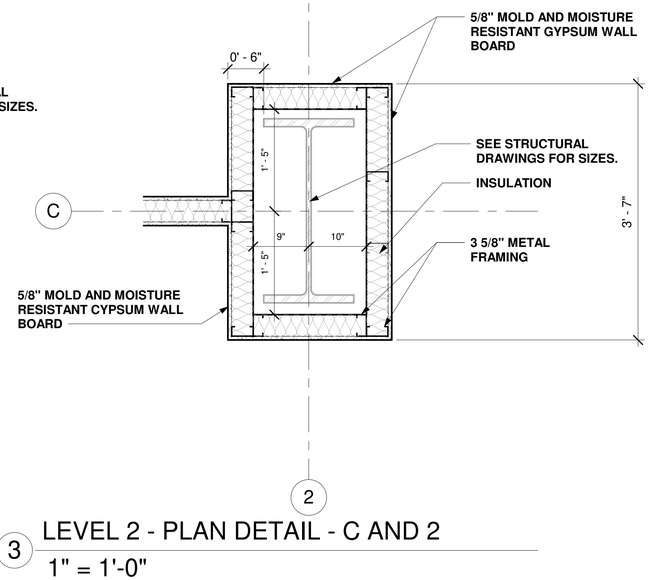
1 LEVEL 2 - PLAN DETAIL - A AND 2

1" = 1'-0"



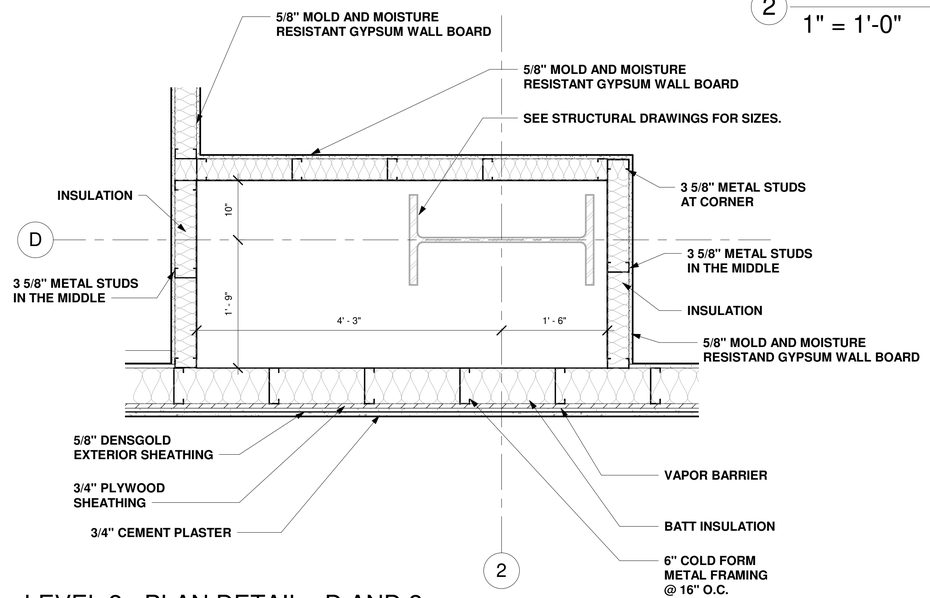
2 LEVEL 2 - PLAN DETAIL - B AND 2

1" = 1'-0"



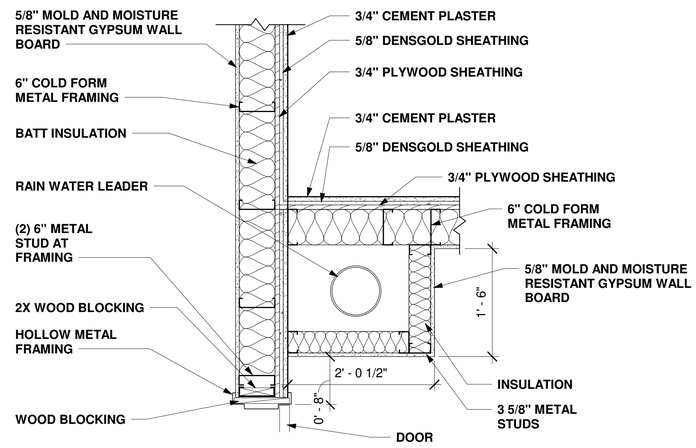
3 LEVEL 2 - PLAN DETAIL - C AND 2

1" = 1'-0"



4 LEVEL 2 - PLAN DETAIL - D AND 2

1" = 1'-0"



5 RAIN LEADER COVER

1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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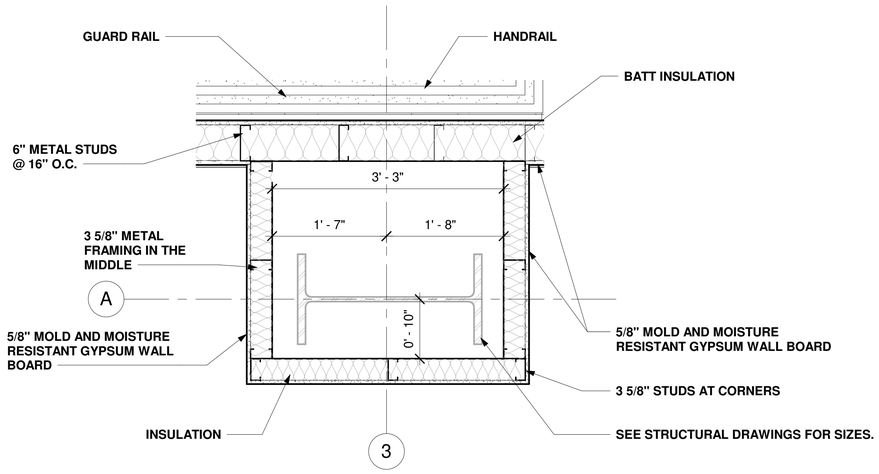
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.2

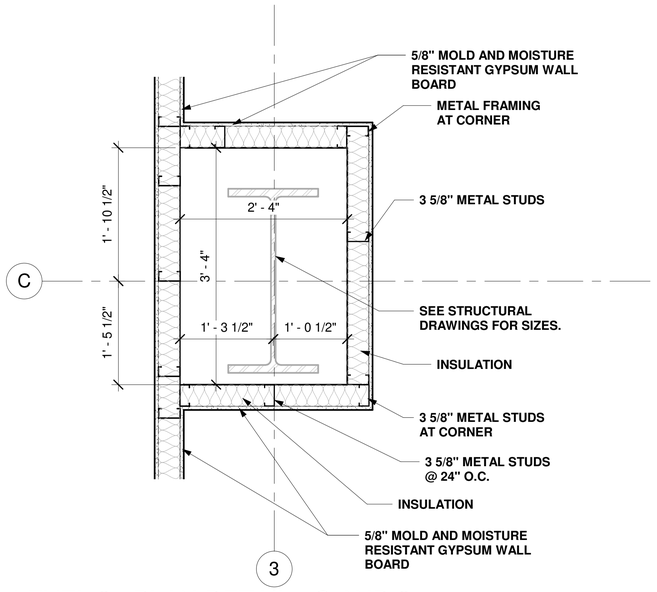
ENLARGED COLUMN ENCLOSURES

LICENSED ARCHITECT U.S.V.I. U.S.A.



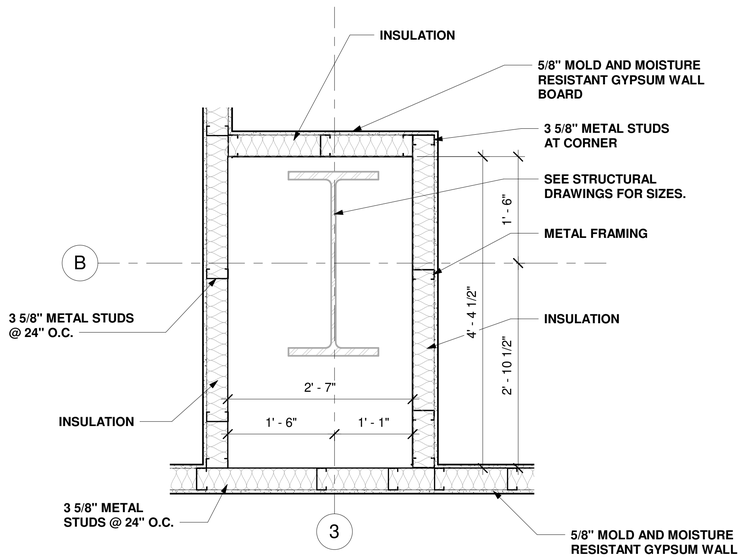
1 LEVEL 2 - PLAN DETAIL - A AND 3

1" = 1'-0"



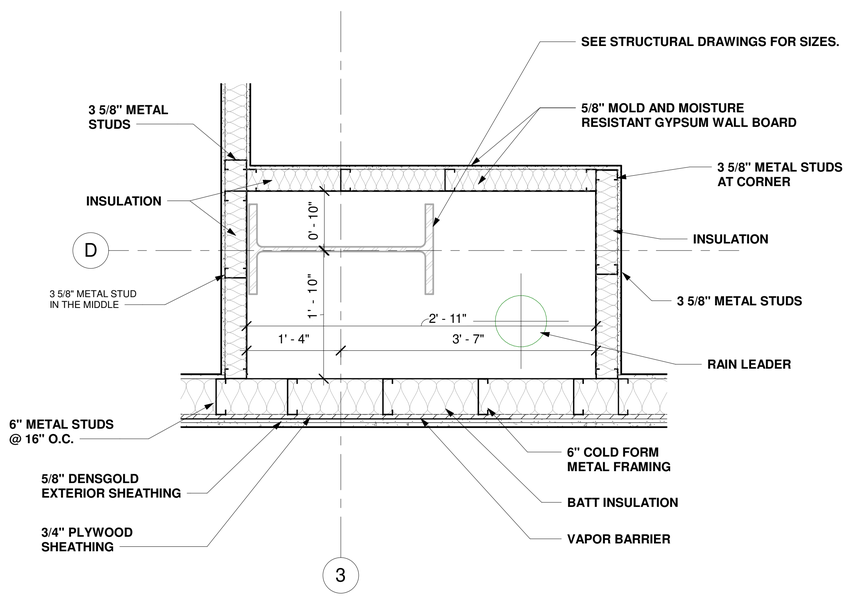
3 LEVEL 2 - PLAN DETAIL - C AND 3

1" = 1'-0"



2 LEVEL 2 - PLAN DETAIL - B AND 3

1" = 1'-0"



4 LEVEL 2 - PLAN DETAIL - D AND 3

1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

Jaredian Design Group
Architects, Engineers and Construction Managers

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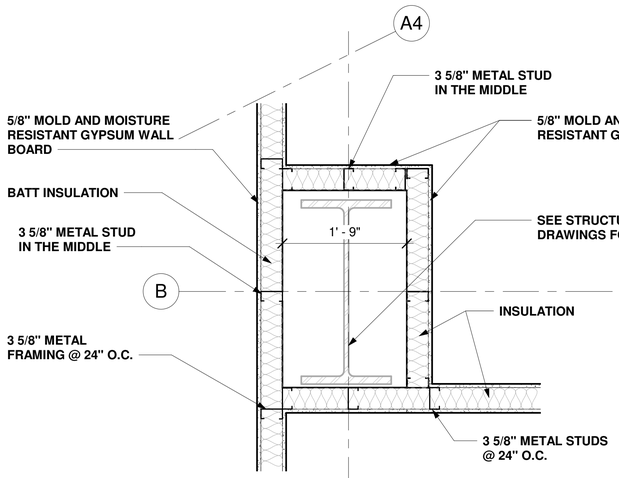
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.3

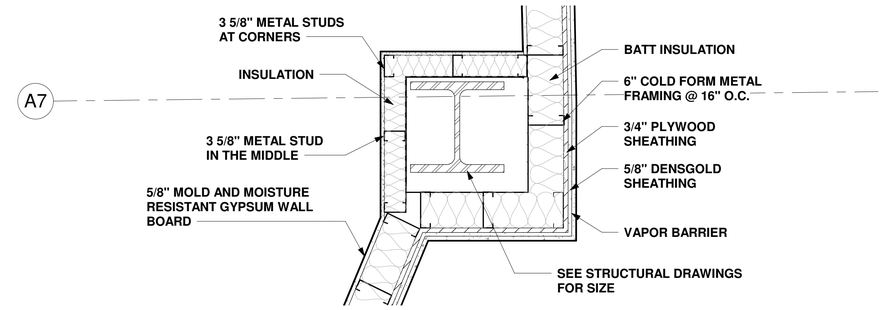
ENLARGED COLUMN ENCLOSURES

REGISTERED ARCHITECT U.S. VIRGIN ISLANDS



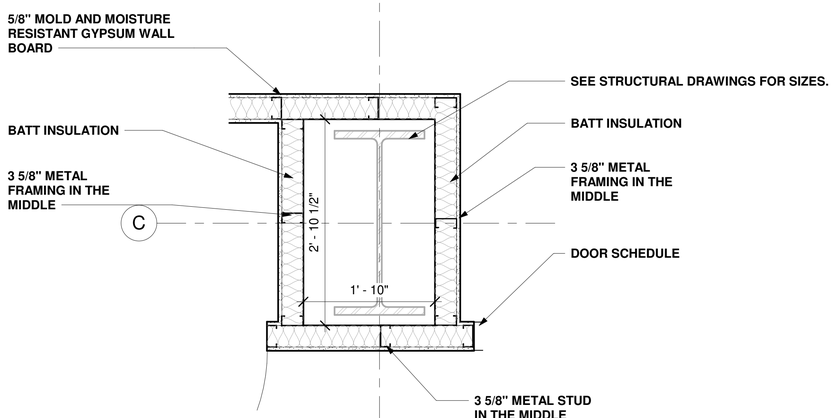
1 LEVEL 2 - PLAN DETAIL - B AND 4

1" = 1'-0"



3 LEVEL 2 - WALL CONNECTION PLAN

1" = 1'-0"



2 LEVEL 2 - PLAN DETAIL - C AND 4

1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

Jaredian Design Group
Architects, Engineers and Construction Managers

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY
INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 02.14.2025

DRAWN BY: B.J.S

CHECKED BY: J.P.W

APPROVED BY: J.P.W.

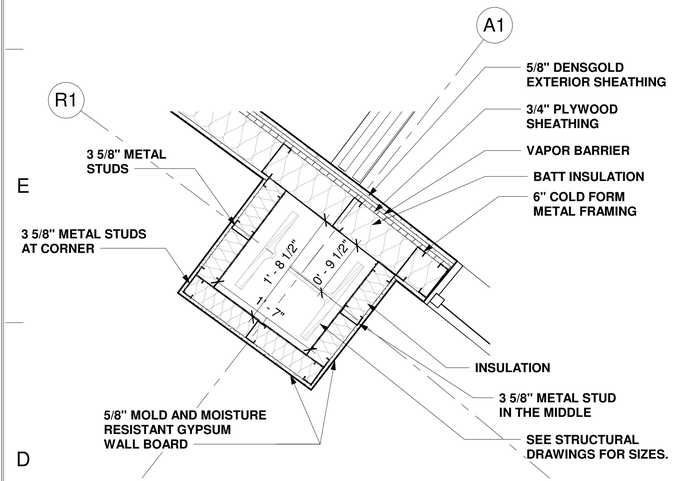
PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A7.4

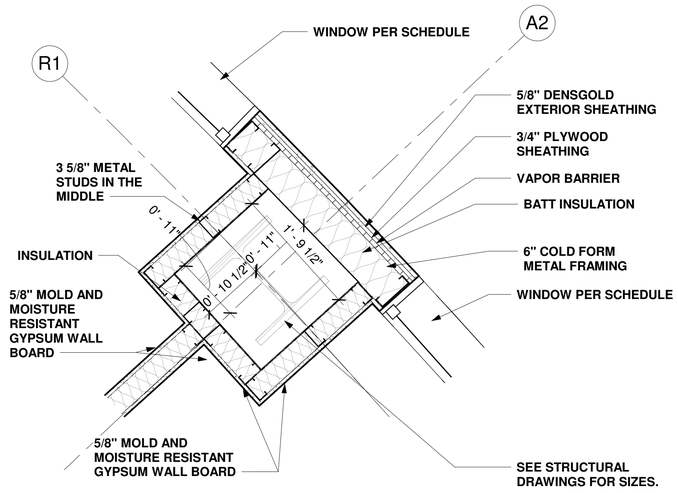
ENLARGED COLUMN ENCLOSURES

REGISTERED ARCHITECT U.S. VIRGIN ISLANDS



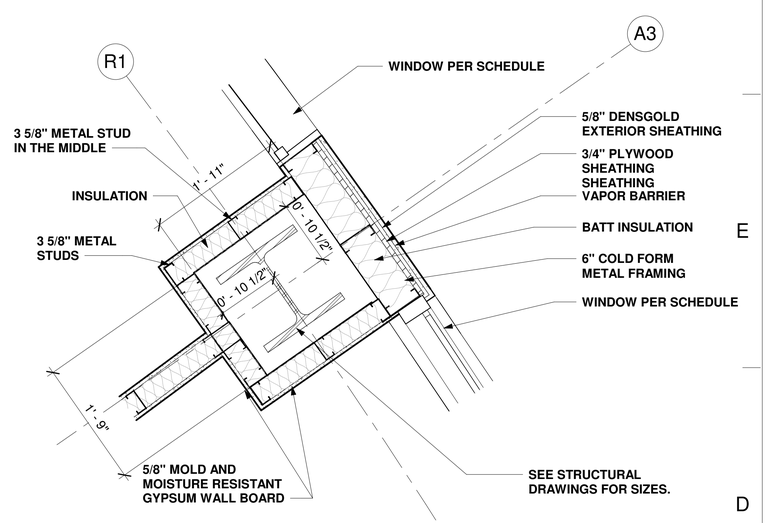
1 LEVEL 2 - PLAN DETAIL - R1 AND A1

1" = 1'-0"



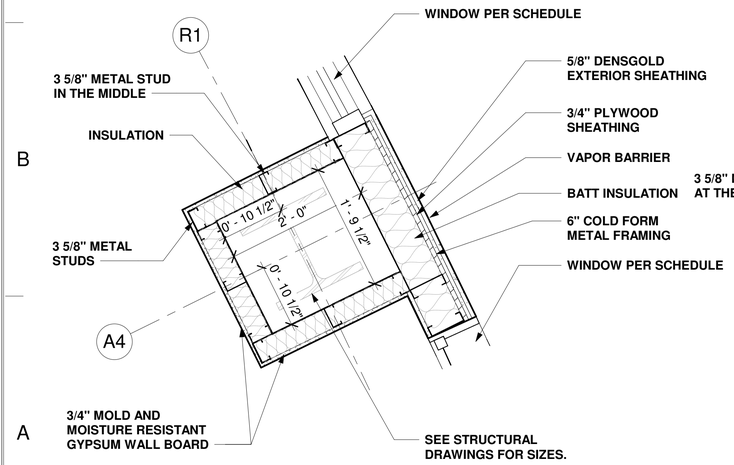
2 LEVEL 2 - PLAN DETAIL - R1 AND A2

1" = 1'-0"



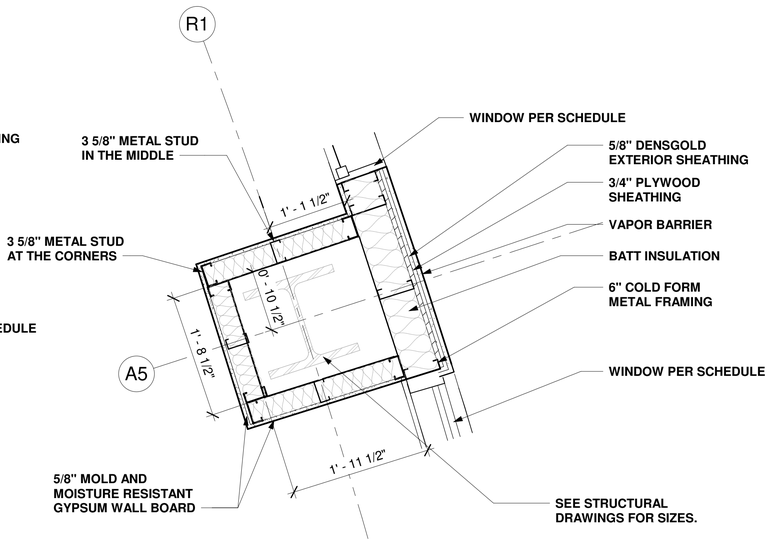
3 LEVEL 2 - PLAN DETAIL - R1 AND A3

1" = 1'-0"



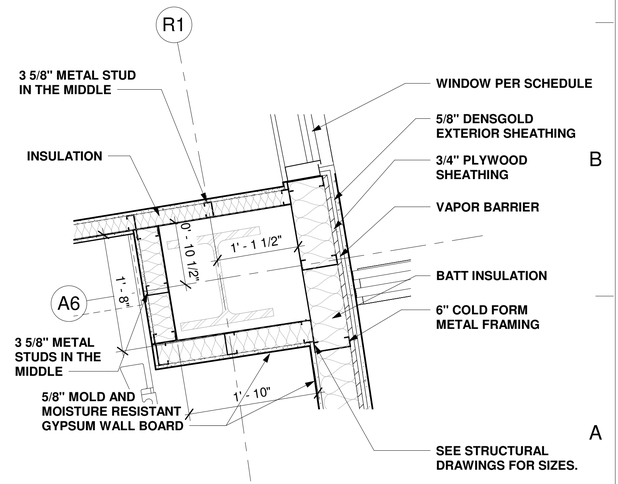
4 LEVEL 2 - PLAN DETAIL - R1 AND A4

1" = 1'-0"



5 LEVEL 2 - PLAN DETAIL - R1 AND A5

1" = 1'-0"



6 LEVEL 2 - PLAN DETAIL - R1 AND A6

1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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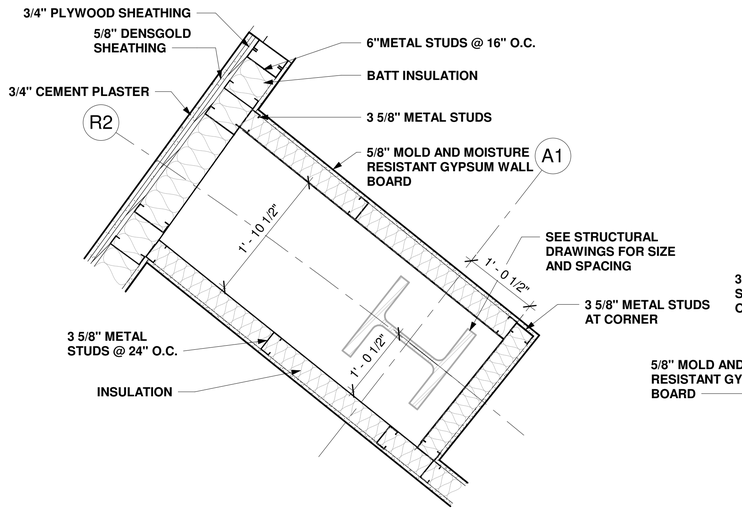
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

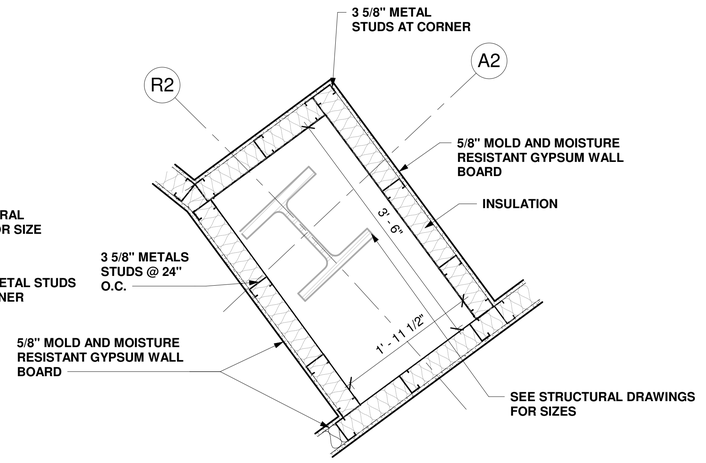
DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.5

ENLARGED COLUMN ENCLOSURES

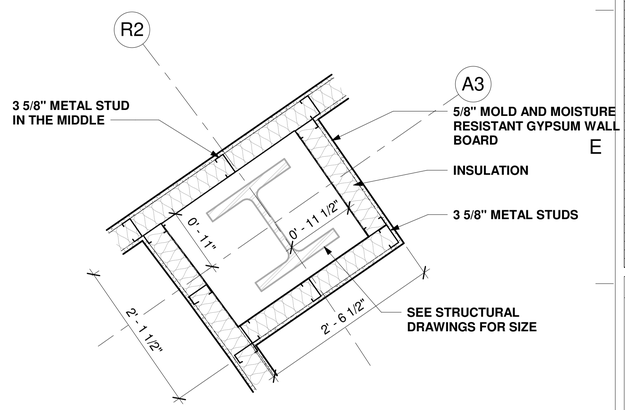
LICENSED ARCHITECT — U.S.V.I.



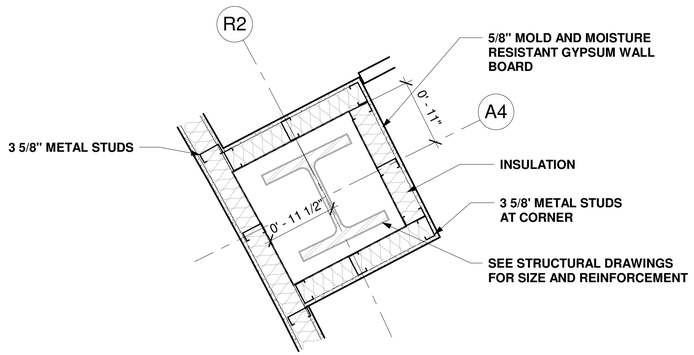
1 LEVEL 2 - PLAN DETAIL - A1 AND R2
1" = 1'-0"



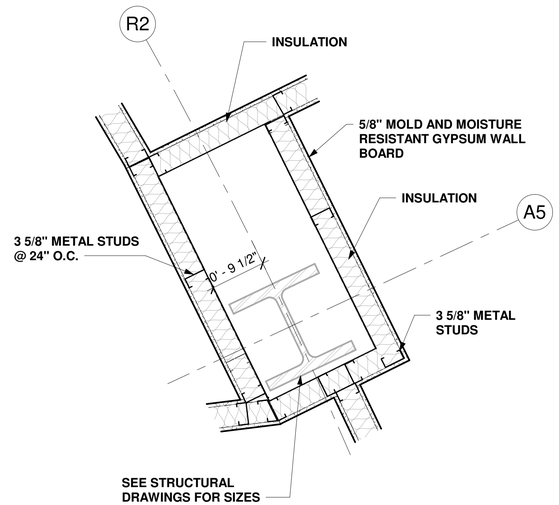
2 LEVEL 2 - PLAN DETAIL - A2 AND R2
1" = 1'-0"



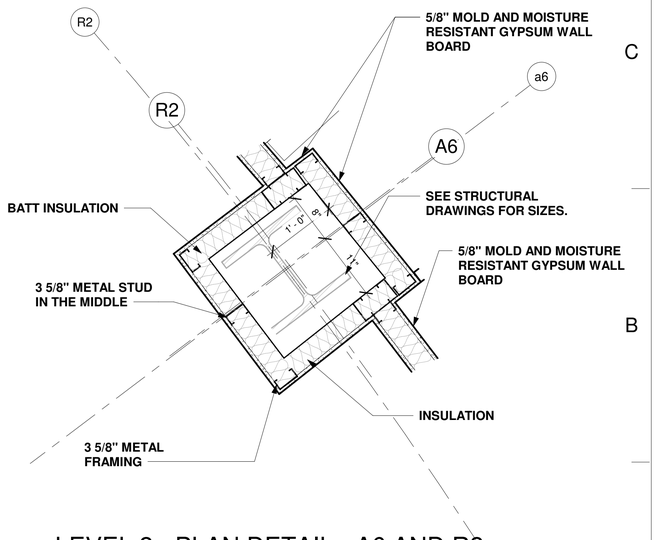
3 LEVEL 2 - PLAN DETAIL - A3 AND R2
1" = 1'-0"



4 LEVEL 2 - PLAN DETAIL - A4 AND R2
1" = 1'-0"



5 LEVEL 2 - PLAN DETAIL - A5 AND R2
1" = 1'-0"



6 LEVEL 2 - PLAN DETAIL - A6 AND R2
1" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

Jaredian Design Group
Architects, Engineers and Construction Managers
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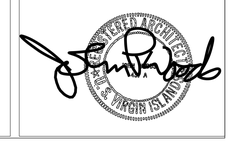
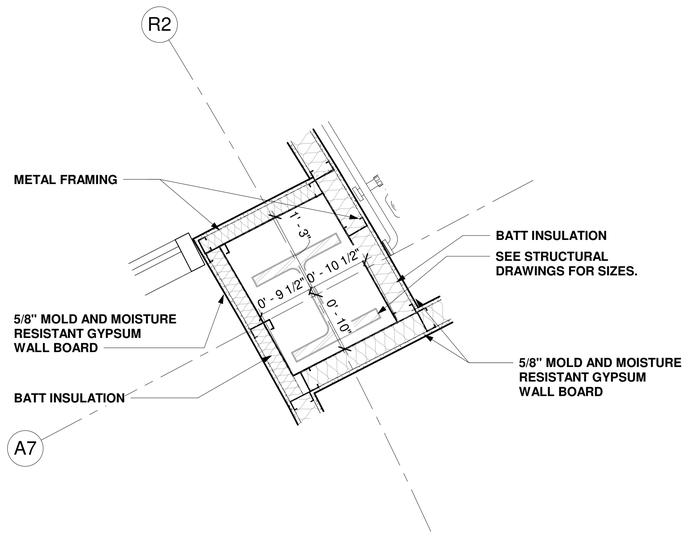
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

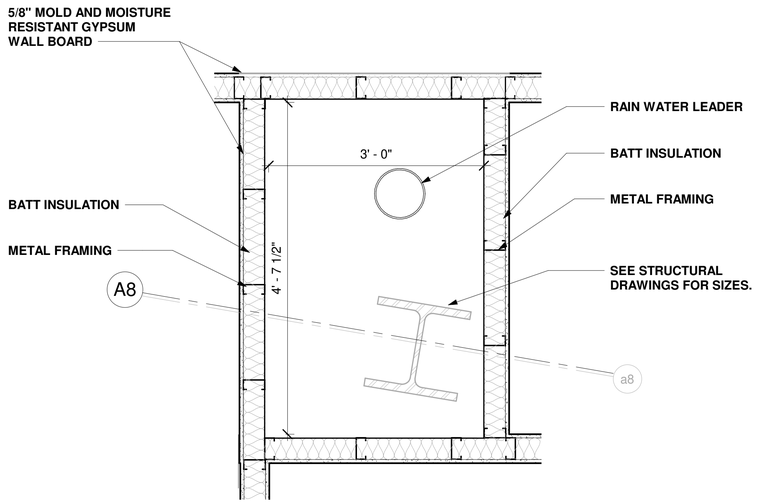
DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.6

ENLARGED COLUMN ENCLOSURES

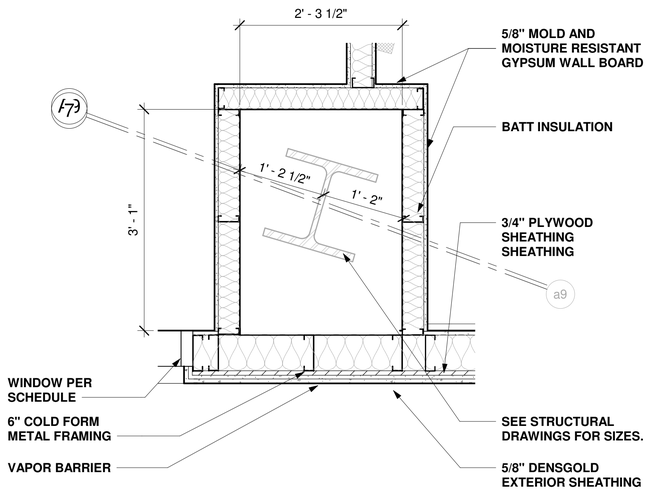




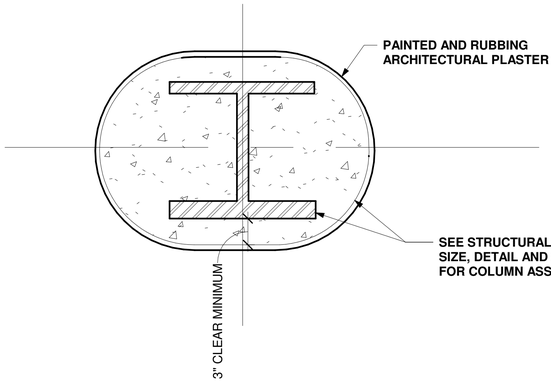
1 LEVEL 2 - PLAN DETAIL - A7 AND R2
1" = 1'-0"



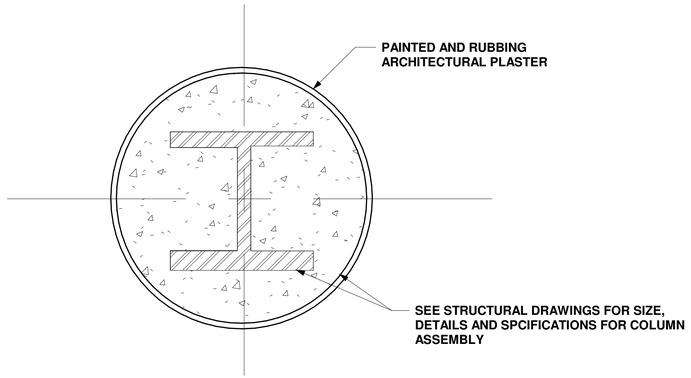
2 LEVEL 2 - PLAN DETAIL - A8
1" = 1'-0"



3 LEVEL 2 - PLAN DETAIL - A9
1" = 1'-0"



4 COLUMN COVER 1
1 1/2" = 1'-0"



5 COLUMN COVER 2
1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

Jaredian Design Group
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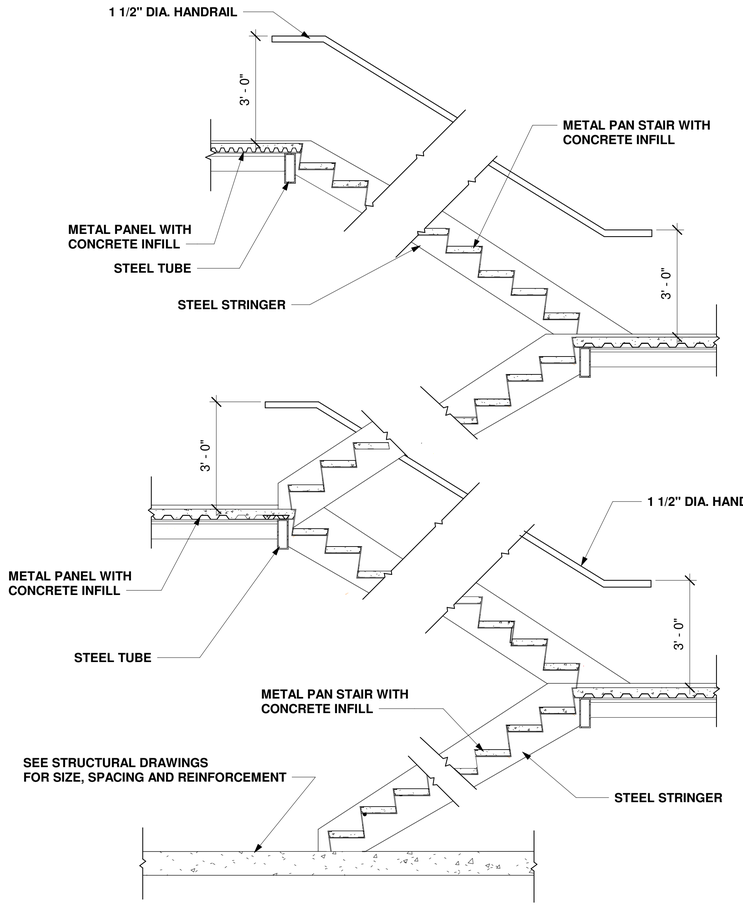
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	B.J.S
CHECKED BY:	J.P.W
APPROVED BY:	J.P.W.
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A7.7

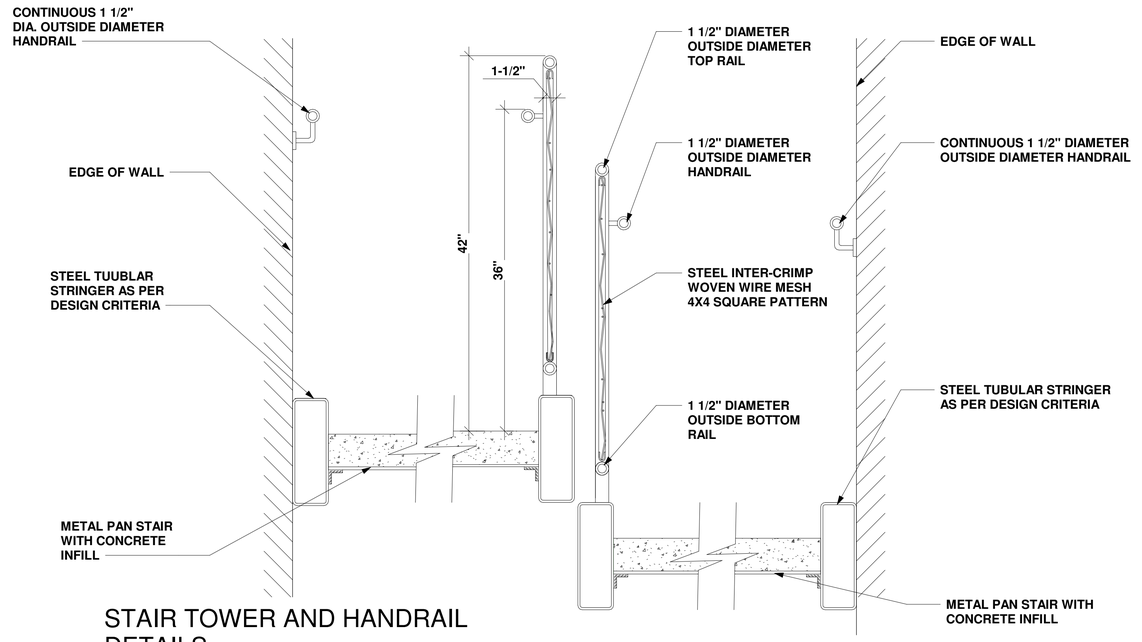
ENLARGED COLUMN ENCLOSURES

REGISTERED ARCHITECT J. PAUL WARD U.S.V.I.



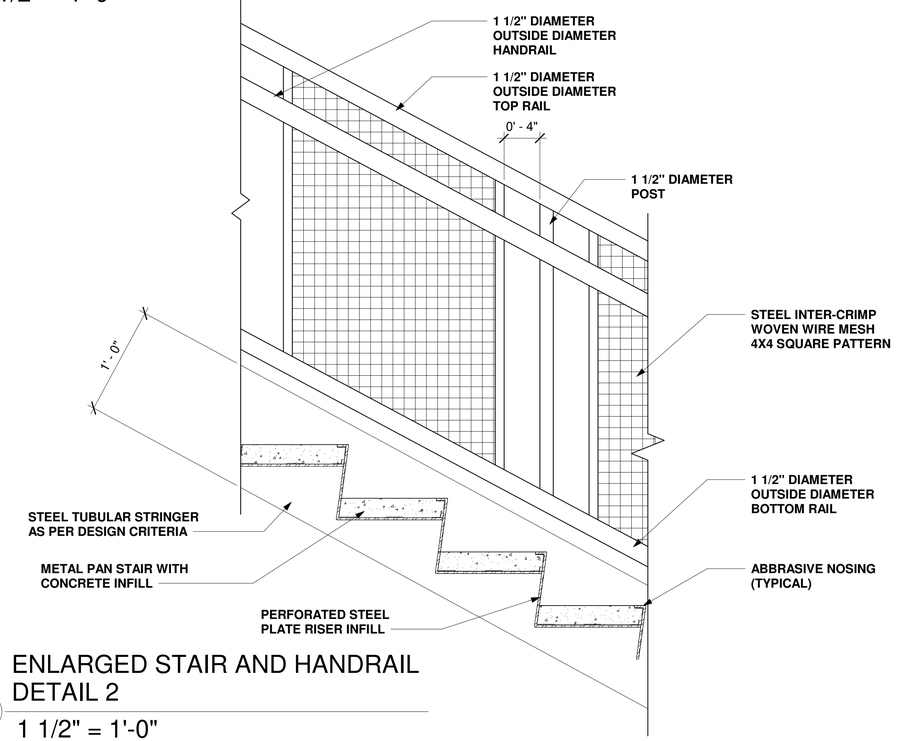
1 STAIR TOWER DETAIL

1/2" = 1'-0"



2 STAIR TOWER AND HANDRAIL DETAILS

1 1/2" = 1'-0"



3 ENLARGED STAIR AND HANDRAIL DETAIL 2

1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers
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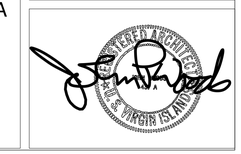
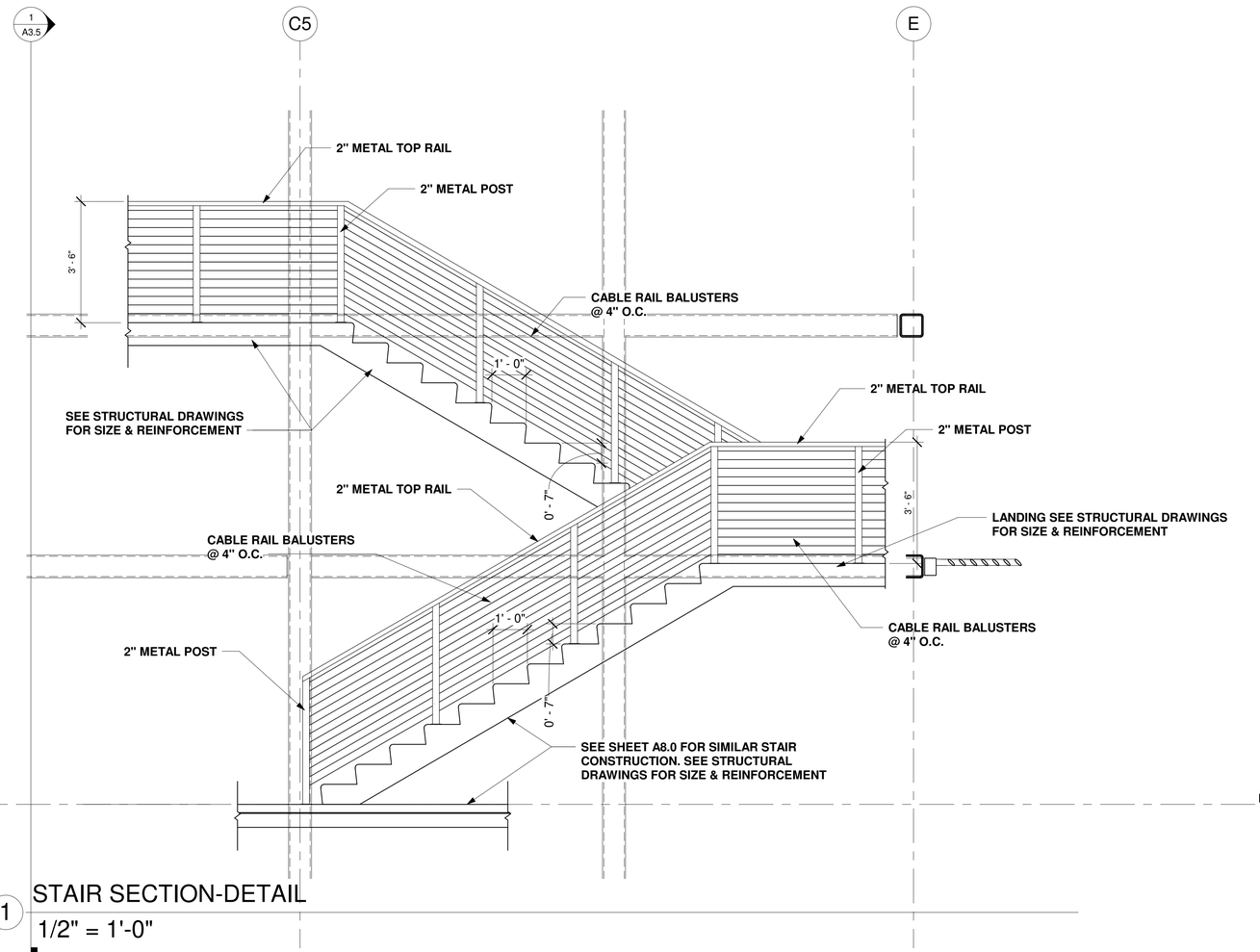
PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE:	02.14.2025
DRAWN BY:	Author
CHECKED BY:	Checker
APPROVED BY:	Approver
PROJECT NO.:	21-003.C
CONTRACT NO.:	-
SHEET NO.:	A8.0

STAIR SECTIONS





1 STAIR SECTION-DETAIL

1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

JDG Jaredian Design Group
Architects, Engineers and Construction Managers

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

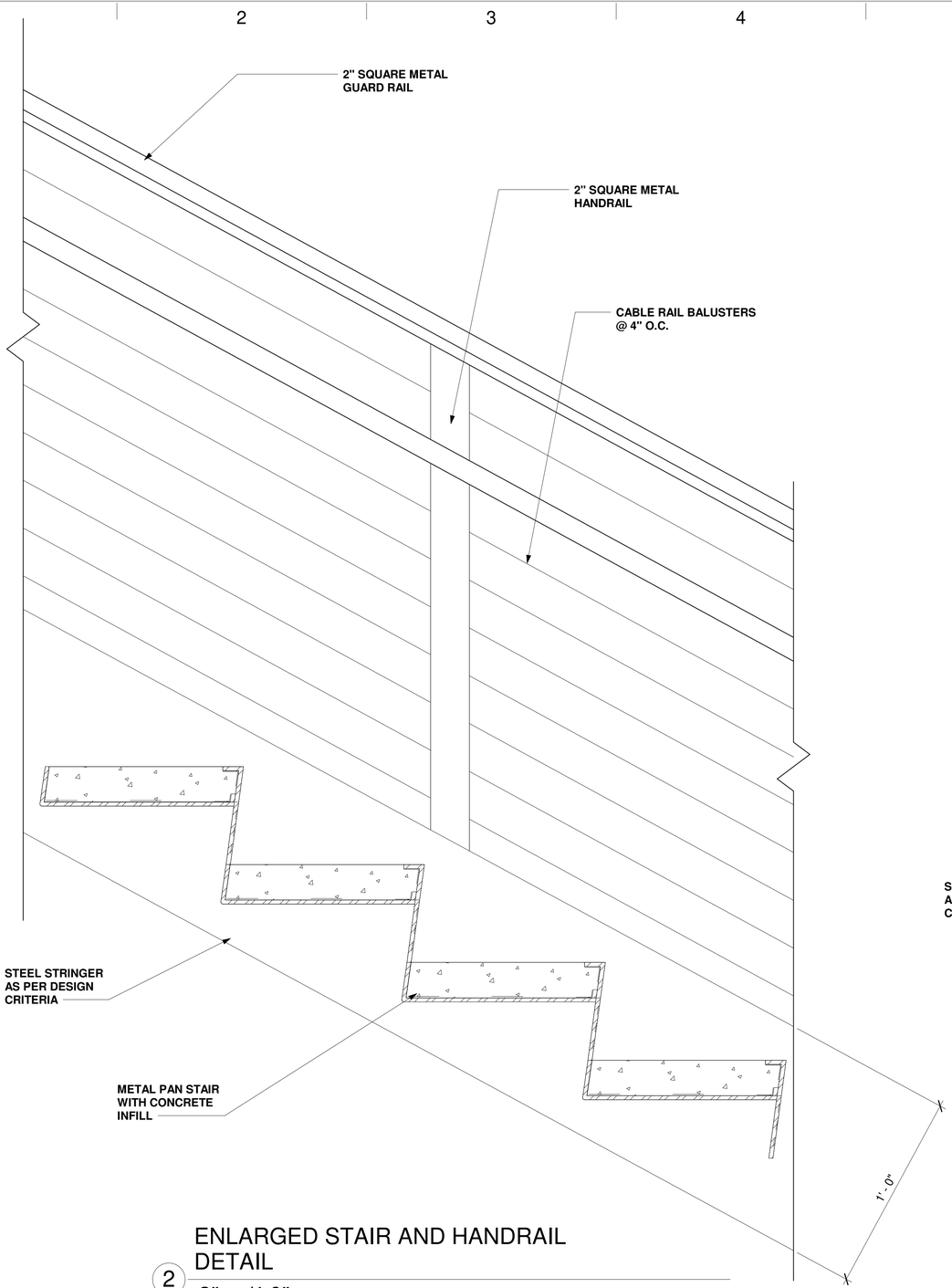
DATE:	02.14.2025
DRAWN BY:	Author
CHECKED BY:	Checker
APPROVED BY:	Approver

PROJECT NO.: 21-003.C

CONTRACT NO.: -

SHEET NO.: A8.1

STAIR SECTIONS

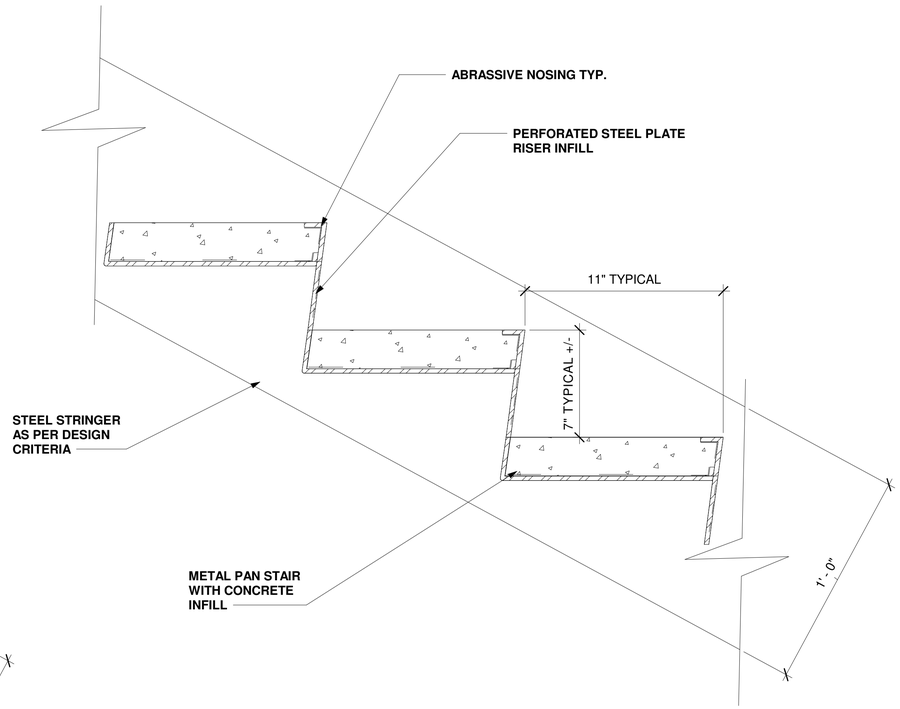


ENLARGED STAIR AND HANDRAIL DETAIL

2 — 3" = 1'-0"

NOTE

1. CONTRACTOR IS RESPONSIBLE FOR PROVIDING SHOP DRAWINGS FOR STAIR FABRICATION.
2. ALL RISERS HEIGHTS SHALL BE EQUAL. THE CONTRACTOR IS RESPONSIBLE FOR ENSURING THAT THERE IS KNOW HEIGHT DIFFERENCE BETWEEN RISERS. THE CONTRACTOR SHALL VERIFY ALL FLOOR TO FLOOR HEIGHTS PRIOR TO THE FABRICATION OF THE STAIR.



ENLARGED STAIR NOSING DETAIL

1 — 3" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

CONSULTANTS

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

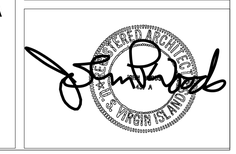
DATE:	02.14.2025
DRAWN BY:	Author
CHECKED BY:	Checker
APPROVED BY:	Approver

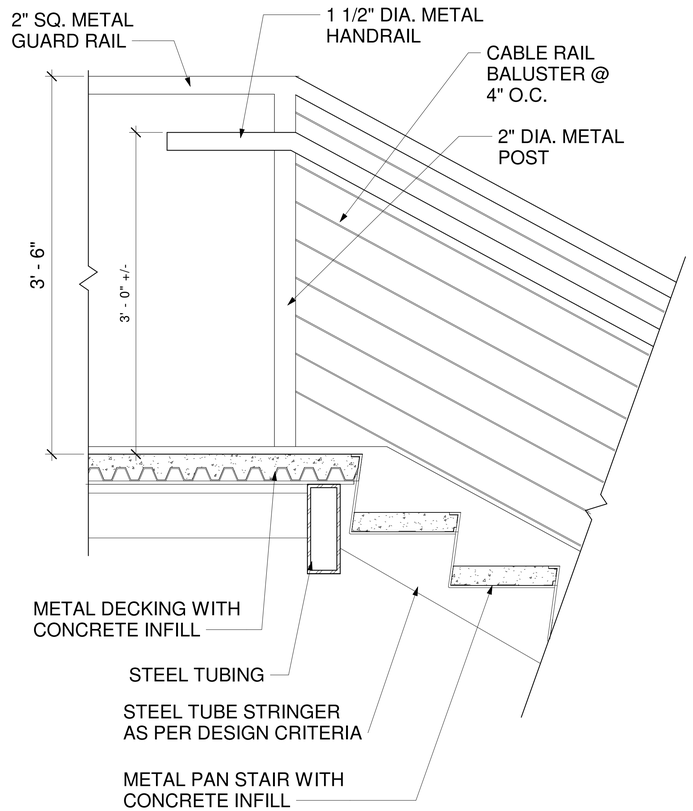
PROJECT NO.: 21-003.C

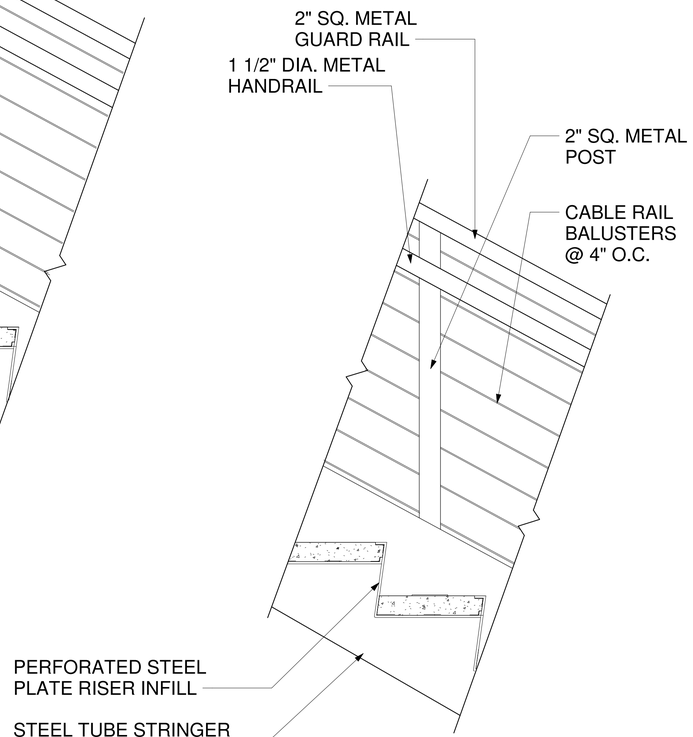
CONTRACT NO.: -

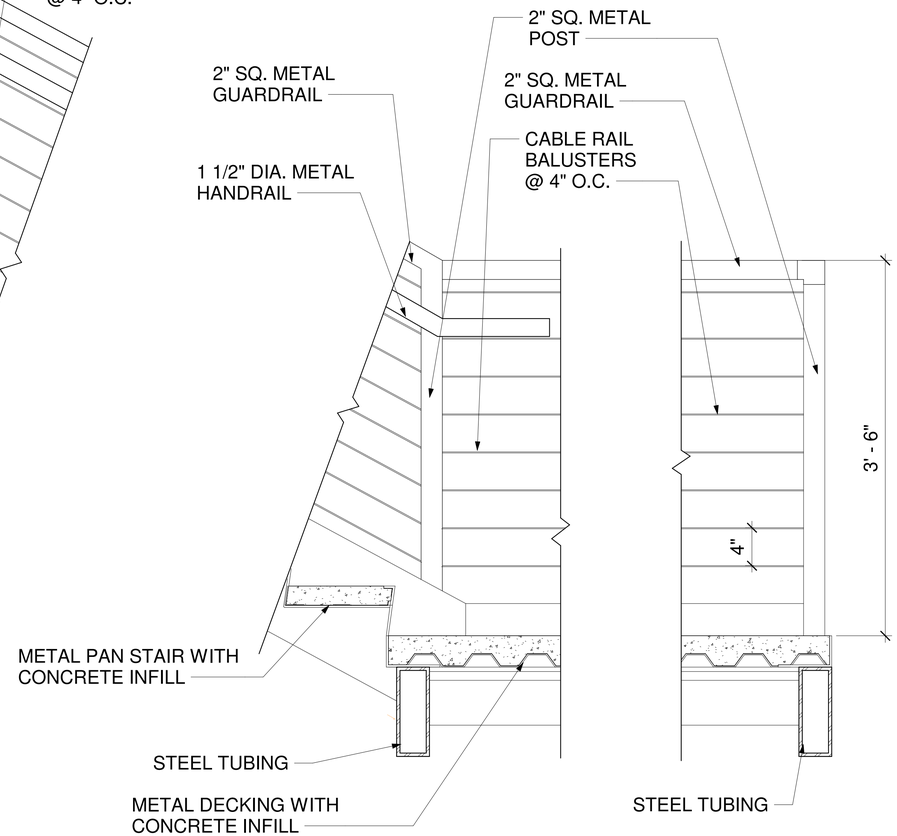
SHEET NO.: A8.2

STAIR DETAILS









ENLARGED STAIR AND HANDRAIL DETAILS

1 1 1/2" = 1'-0"

REVISIONS

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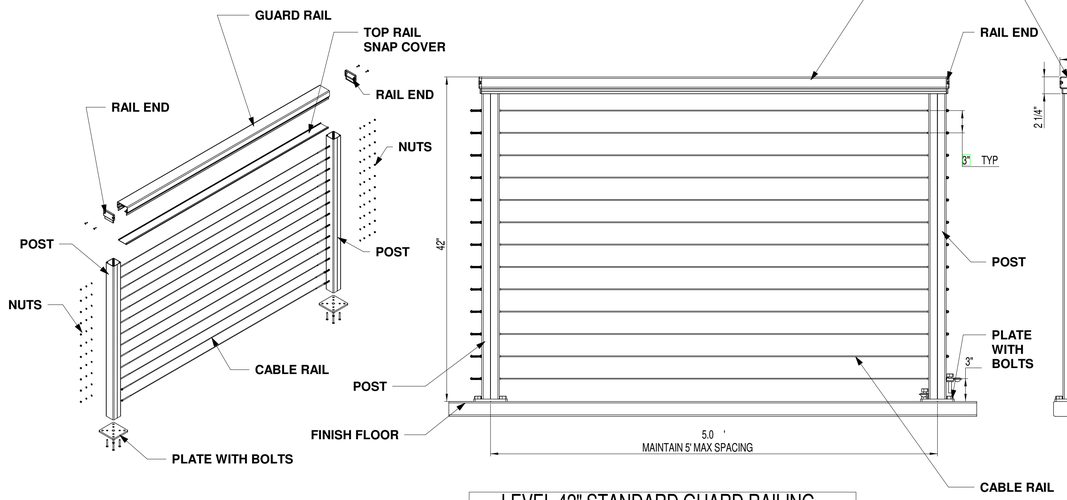
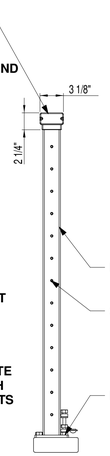
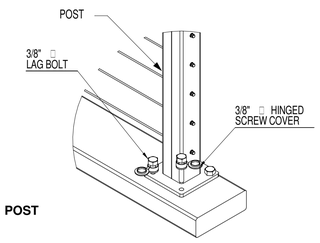
SHEET NO.: A8.3

STAIR AND HANDRAIL DETAILS

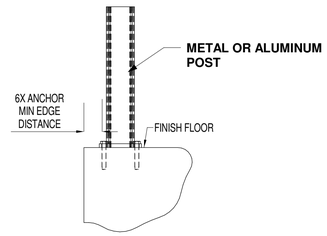
LICENSED ARCHITECT
U.S. VIRGIN ISLANDS







DECK MOUNT



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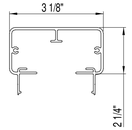
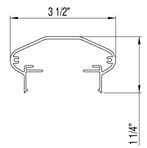
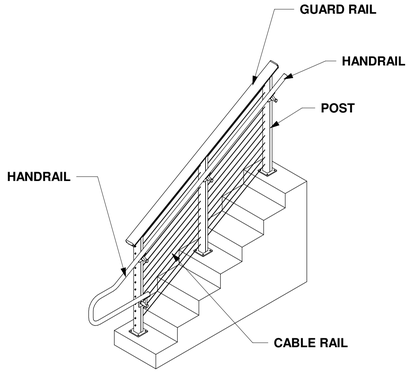
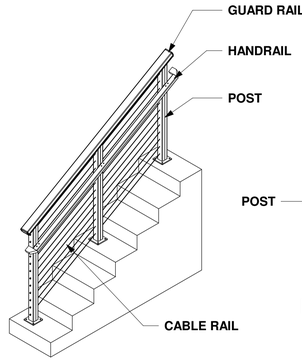




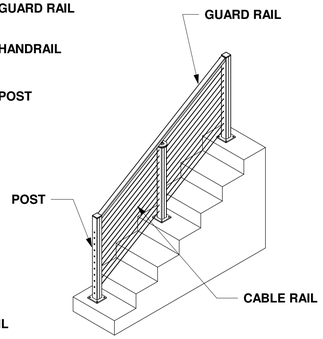
LEVEL 42" STANDARD GUARD RAILING



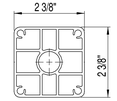
STAINLESS STEEL HAND RAIL



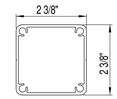
POST-TO-POST STAND ALONE HAND RAIL



POST-TO-POST STAIR HAND RAIL



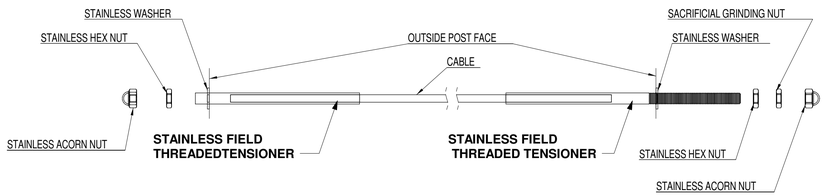
TERMINAL POST



INTERMEDIATE POST

STAIRS 42" STANDARD GUARD RAILING

TOP RAIL OPTIONS



CABLE ATTACHMENT DETAIL

1. CUT EXCESS THREAD.
2. REMOVE SACRIFICIAL NUT.
3. SCREW ON ACORN NUT.

REVISIONS

NO.	DATE	REMARKS

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SUB BASE, CROWN BAY
ST. THOMAS, USVI

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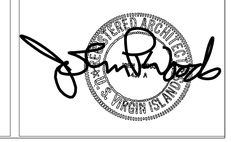
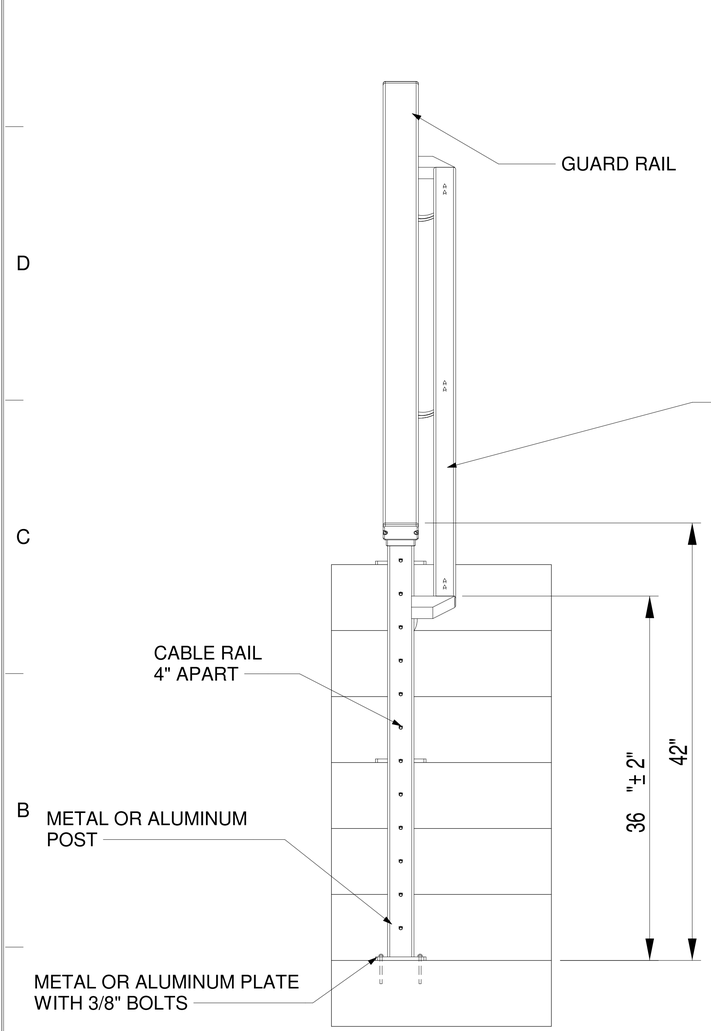
CHECKED BY: Checker
APPROVED BY: Approver

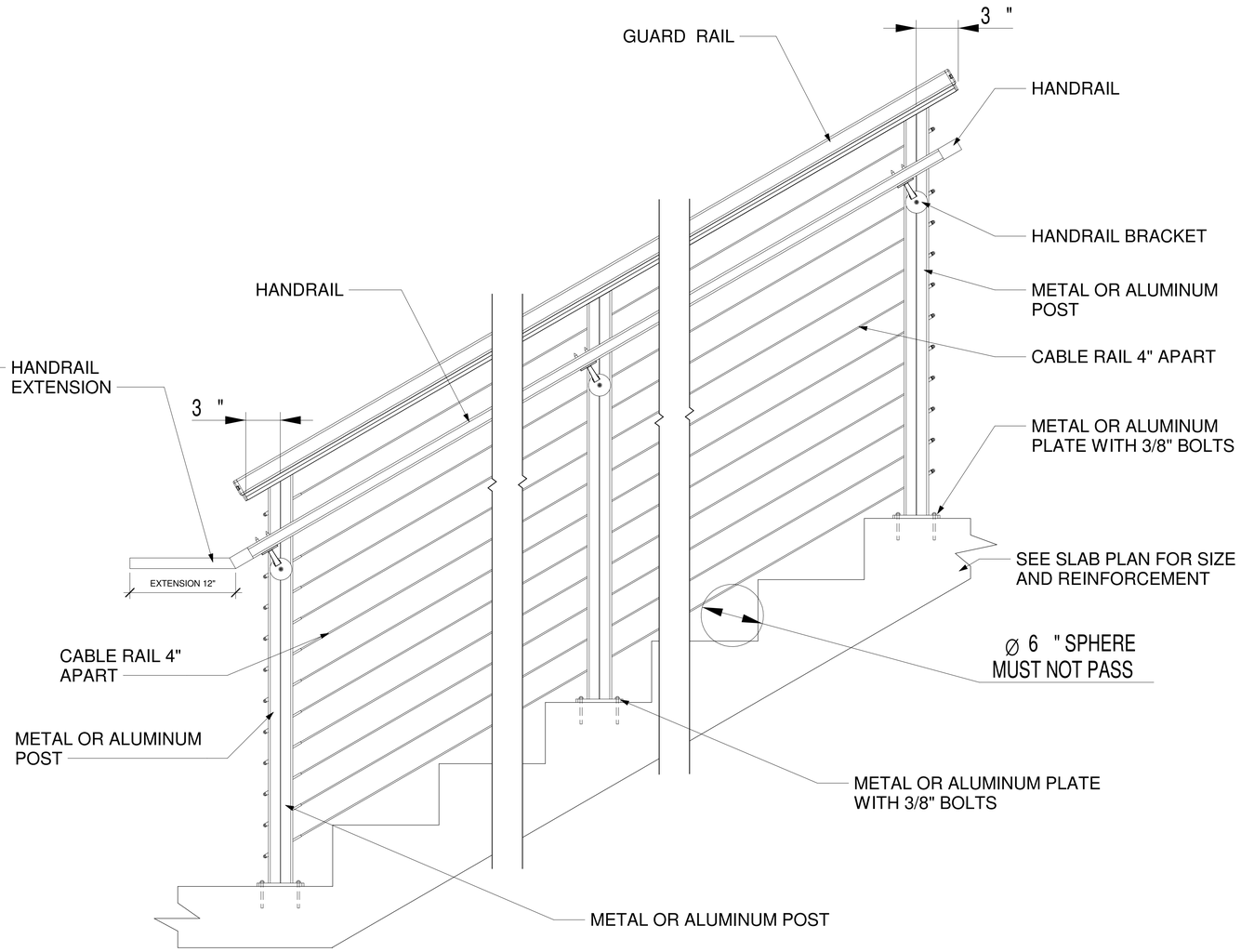
PROJECT NO.: 21-003.C
CONTRACT NO.: -

SHEET NO.: A8.4

RAILING DETAILS







REVISIONS

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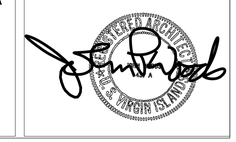
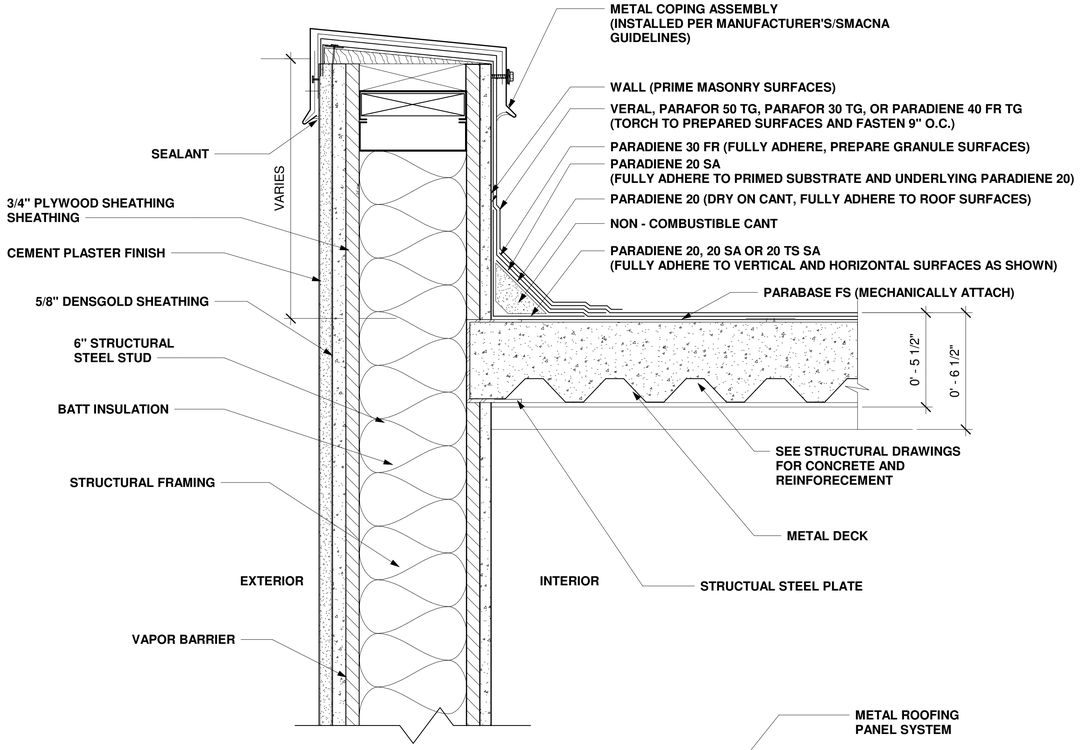
CONTRACT NO.: -

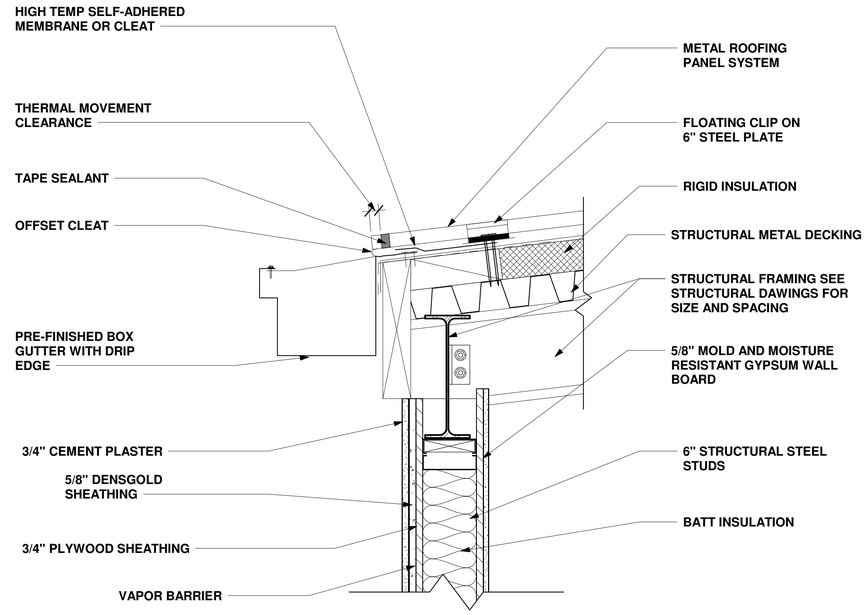
SHEET NO.: A8.5

RAILING DETAILS

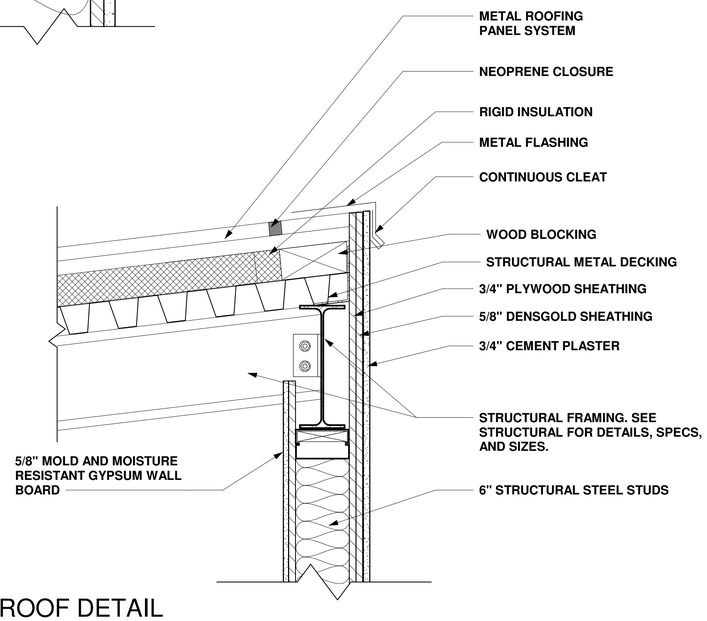




1 PARAPET DETAIL
3" = 1'-0"



3 TYPICAL EAVE WITH GUTTER
1 1/2" = 1'-0"



4 ROOF DETAIL
1 1/2" = 1'-0"

REVISIONS

NO.	DATE	REMARKS

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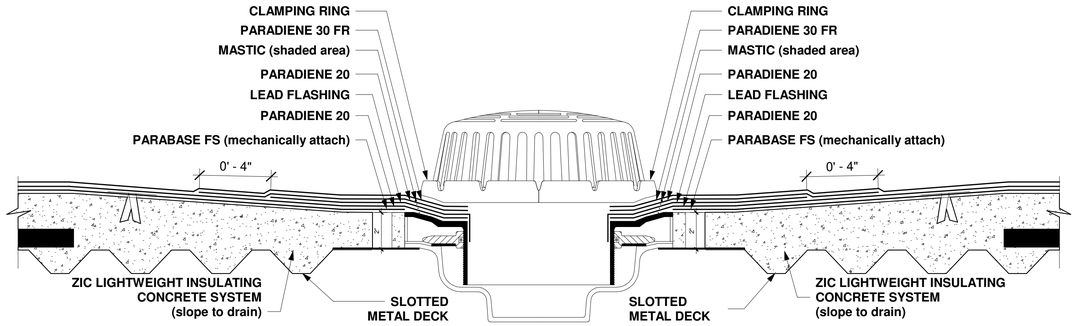
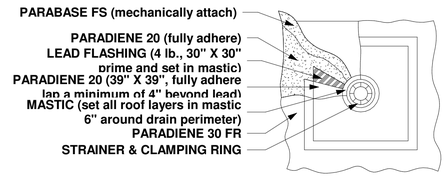
SHEET NO.: A9.0

ROOF DETAILS



1 ROOF DRAIN DETAIL

3" = 1'-0"



REVISIONS

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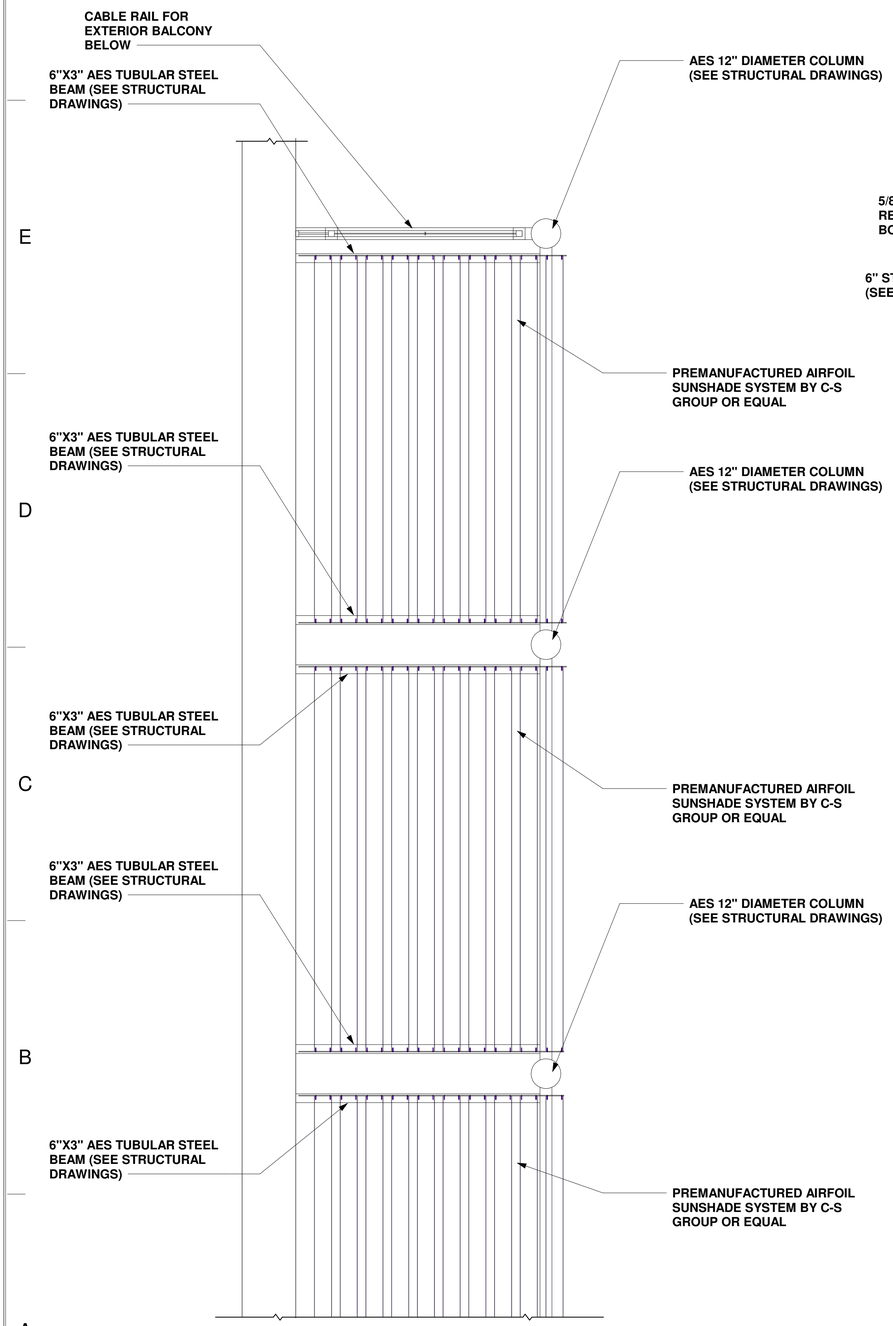
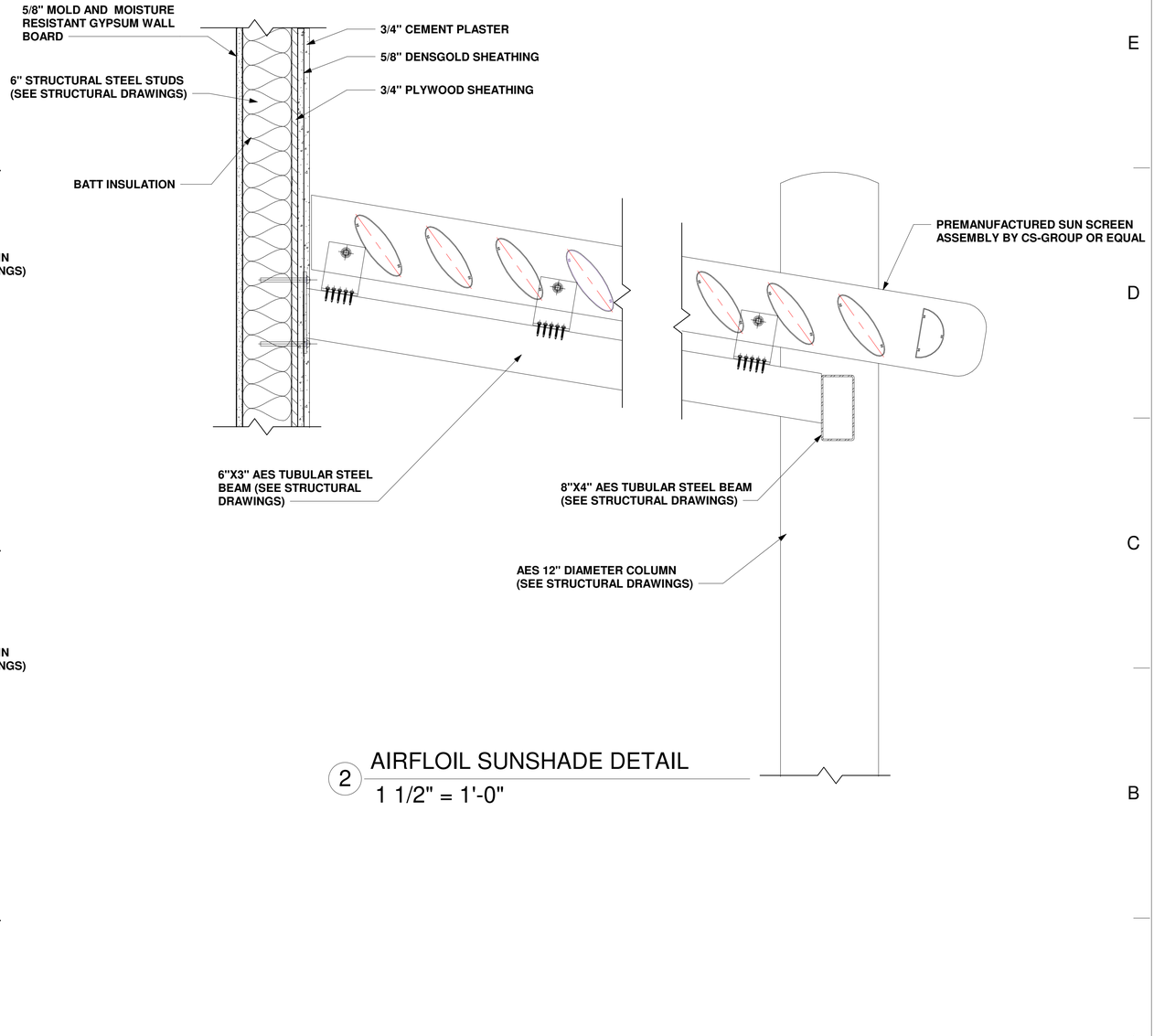
SHEET NO.: A9.1

ROOF DRAIN DETAIL



1 AIRFOIL SUNSHADE
1/2" = 1'-0"



2 AIRFLOIL SUNSHADE DETAIL
1 1/2" = 1'-0"

REVISIONS		
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SHEET NO.:	A9.2

AIRFLOW SUNSHADE DETAILS

ABBREVIATIONS

A/C	ABOVE CEILING
AD	ACCESS DOOR
ADJ	ADJUSTABLE
AFF	ABOVE FINISHED FLOOR
AUTO	AUTOMATIC
AC	AIR CONDITIONING
AHU	AIR HANDLING UNIT
ALSD	ARCHITECTURAL LINEAR SLOT DIFFUSER
ALRS	ARCHITECTURAL LINEAR RETURN SLOT
ALES	ARCHITECTURAL LINEAR EXHAUST SLOT
BAL	BALANCING
BDD	BACKDRAFT DAMPER
B/F	BELOW FLOOR
B/G	BELOW GRADE
B'FLY	BUTTERFLY
BHP	BRAKE HORSEPOWER
BCO	BASE CLEANOUT
CFM	CUBIC FEET PER MINUTE
CBCR	CURVED BLADE CEILING REGISTE
CD	CEILING DIFFUSER
CU	CONDENSING UNIT
CW	COLD WATER (DOMESTIC)
CHWS	CHILLED WATER SUPPLY
CHWR	CHILLED WATER RETURN
CWS	CONDENSER WATER SUPPLY
CWR	CONDENSER WATER RETURN
CON	CONCENTRIC
CO	CLEANOUT
COND	CONDENSATE
db	DRY BULB
DN	DOWN
DR	DRAIN
do	DITTO
dB	DECIBELS
DWG	DRAWING
EA	EACH
EAT	ENTERING AIR TEMPERATURE
ECC	ECCENTRIC
EF	EXHAUST FAN
EOD	EMERGENCY OVERFLOW DRAIN
ER	EXHAUST REGISTER
ESP	EXTERNAL STATIC PRESSURE
EWT	ENTERING WATER TEMPERATUR
EXH	EXHAUST
EFF	EFFICIENCY
F	FAHRENHEIT
FCO	FLOOR CLEANOUT
FCU	FAN COIL UNIT
FSD	FIRE/SMOKE DAMPER
FD	FIRE DAMPER OR FLOOR DRAIN
FL DR	FLOOR DRAIN (only)
FLR	FLOOR
FOB	FLAT ON BOTTOM
FOR	FUEL OIL RETURN
FOS	FUEL OIL SUPPLY
FOT	FLAT ON TOP
FPM	FEET PER MINUTE
FPS	FEET PER SECOND
FT	FEET
G	GATE
GA	GAUGE
GPM	GALLONS PER MINUTE
GL	GLOBE
GCO	GRADE CLEANOUT
HD	HUB DRAIN
HP	HORSEPOWER
HTG	HEATING
HW	HOT WATER (DOMESTIC)
HWR	HOT WATER RETURN
HWRR	HOT WATER REVERSE RETURN
HWS	HOT WATER SUPPLY
Hz	HERTZ
ID	INSIDE DIMENSION
IN	INCHES
KW	KILOWATTS
LAT	AIR TEMPERATURE
LB	
LG	
LRG	GRILLE
LWR	LOOP WATER RETURN
LWS	LOOP WATER SUPPLY
MIN	
MAX	
MD	AMPER
MOD	MOTOR OPERATED DAMPER
MFR	MANUFACTURER
NC	NORMALLY CLOSED
NG	
NFWH	FREEZE WALL HYDRANT
NO	NORMALLY OPEN
NOM	NOMINAL
OA	
OD	OUTSIDE DIMENSION
OBD	OPPOSED BLADE DAMPER
PIU	POWERED INDUCTION UNIT
PSI	POUNDS PER SQUARE INCH
RA	
RAD	
RAG	TURN AIR GRILLE
RED	
RL	
RS	RANT SUCTION
RTU	
RAR	RETURN AIR REGISTER
SP	
SPS	IC PRESSURE SENSOR
SA	PPLY AIR
SAN	
SD	OKE DAMPER
SEN	ENSIBLE
SQ	
SR	SUPPLY REGISTER
ST	
SS	SPLIT SYSTEM
TEMP	JRE
TG	RANSFER GRILLE
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
V	VENT
VA	
VTR	)F
VAV	VARIABLE AIR VOLUME
wb	
WC	WATER COLUMN
WI	ATER HAMMER ARRESTOR
WT	WEIGHT
W	WASTE

LEGEND

	CEILING DIFFUSER
	CEILING RETURN AIR GRILLE or EXHAUST GRILLE
	SIDE-WALL or DUCT MOUNTED REGISTER
	SLOT DIFFUSER
	MANUAL VOLUME DAMPER
	FIRE DAMPER
	TRANSITION
T	THERMOSTAT
H	HUMIDISTAT
CO	CO2 SENSOR
N	NIGHT SET-BACK
M	MOTOR OPERATED DAMPER
J	ELECTRICAL J-BOX
	ACCESS PANEL
	EXISTING WORK
	NEW WORK
	WORK TO BE REMOVED

VRF CONDENSING UNIT SCHEDULE

I.D. TAG	COOLING CAPACITY BTU/H	COOLING CAPACITY AMBIENT TEMP (°F DB)	HEATING CAPACITY BTU/H	HEATING CAPACITY AMBIENT TEMP (°F DB)	VOLTS/ PHASE	BASIS OF DESIGN	REMARKS
VRF-1		95		50	208/3	DAIKIN RXYQ---AATJA	(1)
VRF-2		95		50	208/3	DAIKIN RXYQ---AATJA	(1)

(1) ACCEPTABLE ALTERNATE VRF MANUFACTURERS: MITSUBISHI, CARRIER AND HITACHI.

FAN SCHEDULE

I.D. TAG	CAPACITY (CFM)	S.P. (IN. W.G.)	MOTOR H.P.	DRIVE	MAX. FAN RPM	MAX. NOISE	VOLTS/ PHASE	TYPE OF FAN	WEIGHT (LBS).	BASIS OF DESIGN	REMARKS
TEF-1	650			D			120/1	ROOF CENTRIFUGAL		GREENHECK G	(1) (2)
EF-A	200	0.250	49 W	D	931	1.5 SONES	120/1	INLINE CABINET	25	GREENHECK CSP-A250	(3)

(1) ROOF MOUNTED FAN. PROVIDE WITH 12" HIGH ROOF CURB, BIRD SCREEN, PREMIUM EFFICIENCY MOTOR, AND GRAVITY BACKDRAFT DAMPER.
(2) FAN SHALL RUN CONTINUOUSLY DURING OCCUPIED HOURS. INTERLOCK TO TIMER AS NEEDED.
(3) FAN SUSPENDED FROM STRUCTURE, PROVIDE WITH SPEED CONTROLLER, BACKDRAFT DAMPER, DISCONNECT, VIBRATION ISOLATION AND WALL MOUNTED LINE VOLTAGE THERMOSTAT.

SPLIT VRF DEDICATED OUTDOOR AIR SYSTEM SCHEDULE

I.D. TAG	FAN SECTION SUPPLY AIRFLOW (CFM)	FAN SECTION EXT. S.P. (IN. W.C.) (1)	FAN SECTION MOTOR H.P.	COIL SECTION MINIMUM TOTAL CAPACITY (BTUH)	COIL SECTION MINIMUM SENSIBLE CAPACITY (BTUH)	COIL EAT °F db	COIL EAT °F wb	COIL LAT °F db	COIL LAT °F wb	HEATING (2) TYPE	HEATING (2) CAPACITY	HEATING (2) STAGES	OUTDOOR UNIT	BASIS OF DESIGN	VOLTS/ PHASE	MCA	MOCP	APPROX. UNIT WEIGHT (LBS.)	REMARKS
DOAS-1	1,000	0.75				92.8	79.6	50.6	50.6	ELEC		SCR	95.0	DAIKIN DPSC10B	208/3			2,100	(3)

(1) THIS IS THE S.P. EXTERNAL TO THE ENTIRE UNIT ASSEMBLY (WET COIL, CASING, CLEAN FILTERS, AND FURNACE LOSSES ARE NOT INCLUDED IN THIS EXTERNAL S.P.).
(2) OUTPUT CAPACITY IN BTUH FOR GAS HEAT OR KW FOR ELECTRIC HEAT
(3) UNIT INSTALLED AT GRADE WITH HORIZONTAL SUPPLY DISCHARGE. SUPPLY FAN SHALL BE PROVIDED WITH VFD. PROVIDE THE FOLLOWING ACCESSORIES: CIRCUIT BREAKER, MOTOR STARTER, MODULATING HOT GAS REHEAT DEHUMIDIFICATION, HOT GAS BYPASS ON ALL CIRCUITS, METAL MESH PRE-FILTER & 2" PLEATED MERV-8 FILTERS.

VRF INDOOR UNIT SCHEDULE

I.D. TAG	NOMINAL COOLING CAPACITY (BTUH)	NOMINAL HEATING CAPACITY (BTUH)	FAN COIL UNIT DATA AIRFLOW (CFM)	EXT. S.P. (IN. W.C.) (1)	OUTSIDE AIR (CFM)	COIL EAT °F db	COIL EAT °F wb	VOLTS/ PHASE	TYPE OF UNIT	OUTDOOR UNIT	BASIS OF DESIGN	REMARKS
FCU-1				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-2				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-3				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-4				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-5				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-6				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-7				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-8				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-9				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-10				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)
FCU-11				0.5				208/1	MULTI POSITION	VRF-1	DAIKIN FXTQ--TAVJUA	(3)

(1) THIS IS THE S.P. EXTERNAL TO THE ENTIRE FAN COIL UNIT ASSEMBLY (WET COIL, CASING, CLEAN FILTERS, AND FURNACE LOSSES ARE NOT INCLUDED IN THIS EXT. S.P.).
(2) RATINGS BASED ON AHRI STANDARDS 210 / 240.
(3) PROVIDE THE FOLLOWING ACCESSORIES FOR THE FAN COIL UNIT: VIBRATION ISOLATORS, CONDENSATE PUMP (DIVERSITECH MODEL CVMINI OR APPROVED EQUAL), AND PROGRAMMABLE ELECTRONIC THERMOSTAT.

REVISIONS

NO.	DATE	REMARKS
	01/10/2025	90% Progress Set

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

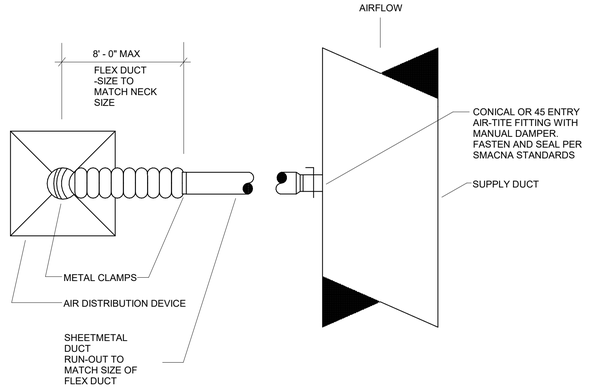
NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 01/10/2025
DRAWN BY: W. CASAL
CHECKED BY: E. CURTIS
APPROVED BY: J. CLARK

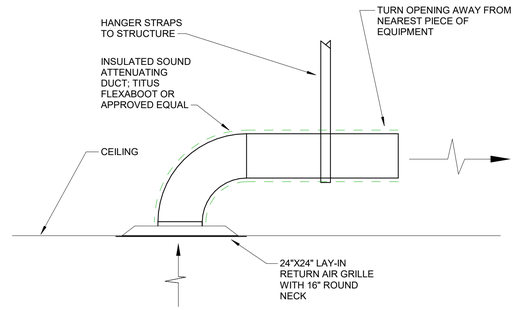
PROJECT NO.: 2022.022.00
CONTRACT NO.: -

SHEET NO.: M0.0

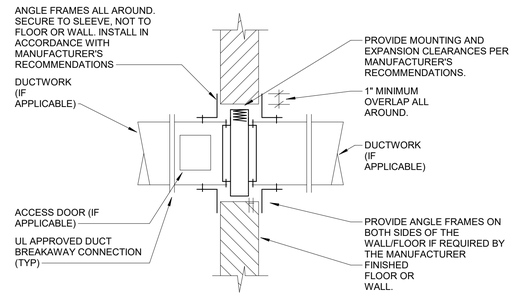
MECHANICAL LEGENDS, ABBREVIATIONS & SCHEDULES



1 M0.1 DIFFUSER CONNECTION DETAIL
12" = 1'-0"

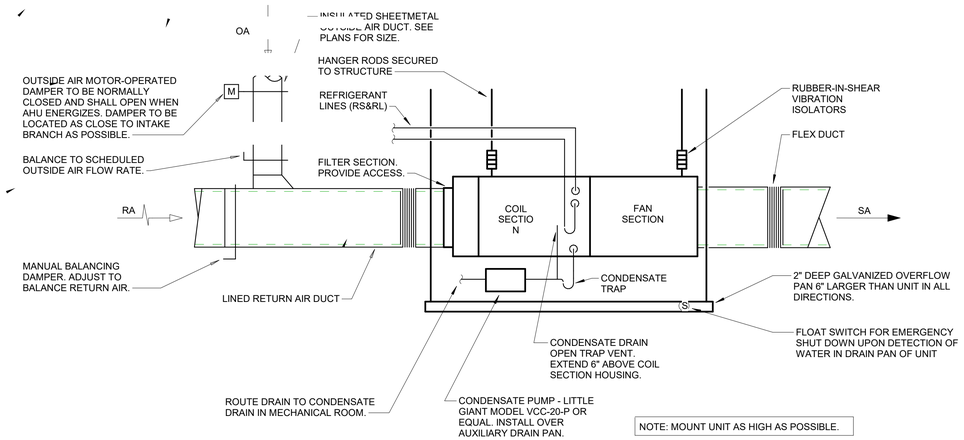


2 M0.1 CEILING RETURN AIR BOOT DETAIL
12" = 1'-0"



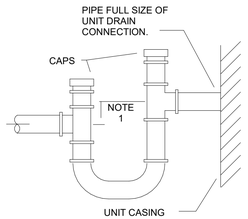
NOTES:
INSTALLATION SHALL SHALL BE IN STRICT ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS AND SHALL MAINTAIN THE MANUFACTURER'S UL LISTING.

3 M0.1 FIRE DAMPER DETAIL
NOT TO SCALE



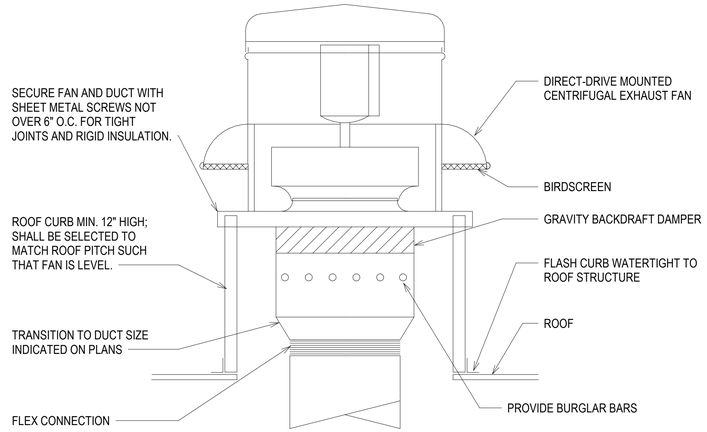
NOTE: MOUNT UNIT AS HIGH AS POSSIBLE.

4 M0.1 HORIZONTAL INDOOR UNIT DETAIL
12" = 1'-0"



NOTE 1: HEIGHT SHALL BE EQUAL TO NEGATIVE STATIC PRESSURE PLUS 1".

5 M0.1 CONDENSATE DRAIN TRAP DETAIL
12" = 1'-0"



6 M0.1 ROOF MOUNTED FAN DETAIL
NOT TO SCALE

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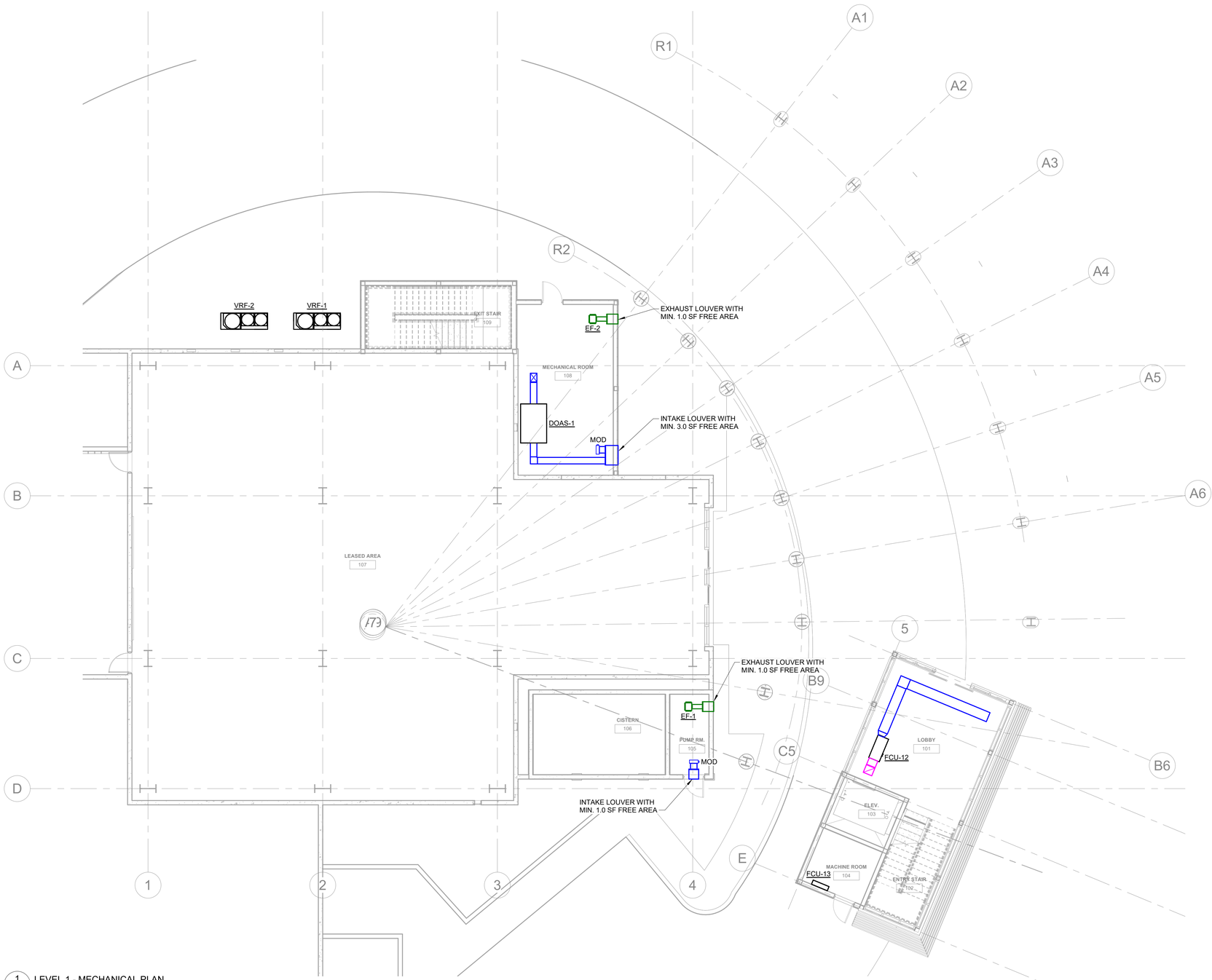
MECHANICAL DETAILS



1 / M1.1 LEVEL 1 - MECHANICAL PLAN
1/8" = 1'-0"

REVISIONS

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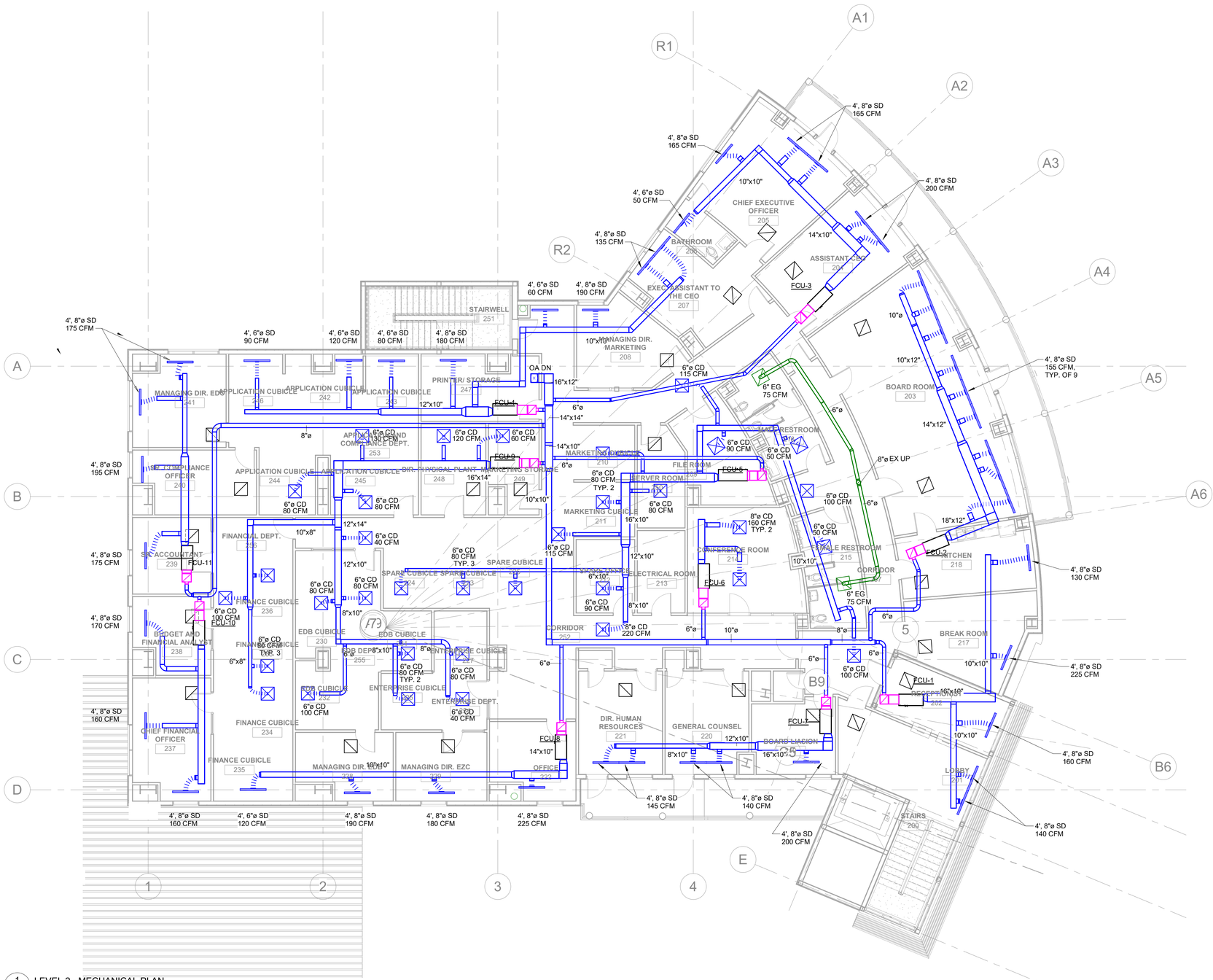
MECHANICAL FLOOR PLAN - LEVEL 1



1 / M1.2 LEVEL 2 - MECHANICAL PLAN
1/8" = 1'-0"

REVISIONS

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	01/10/2025	90% Progress Set

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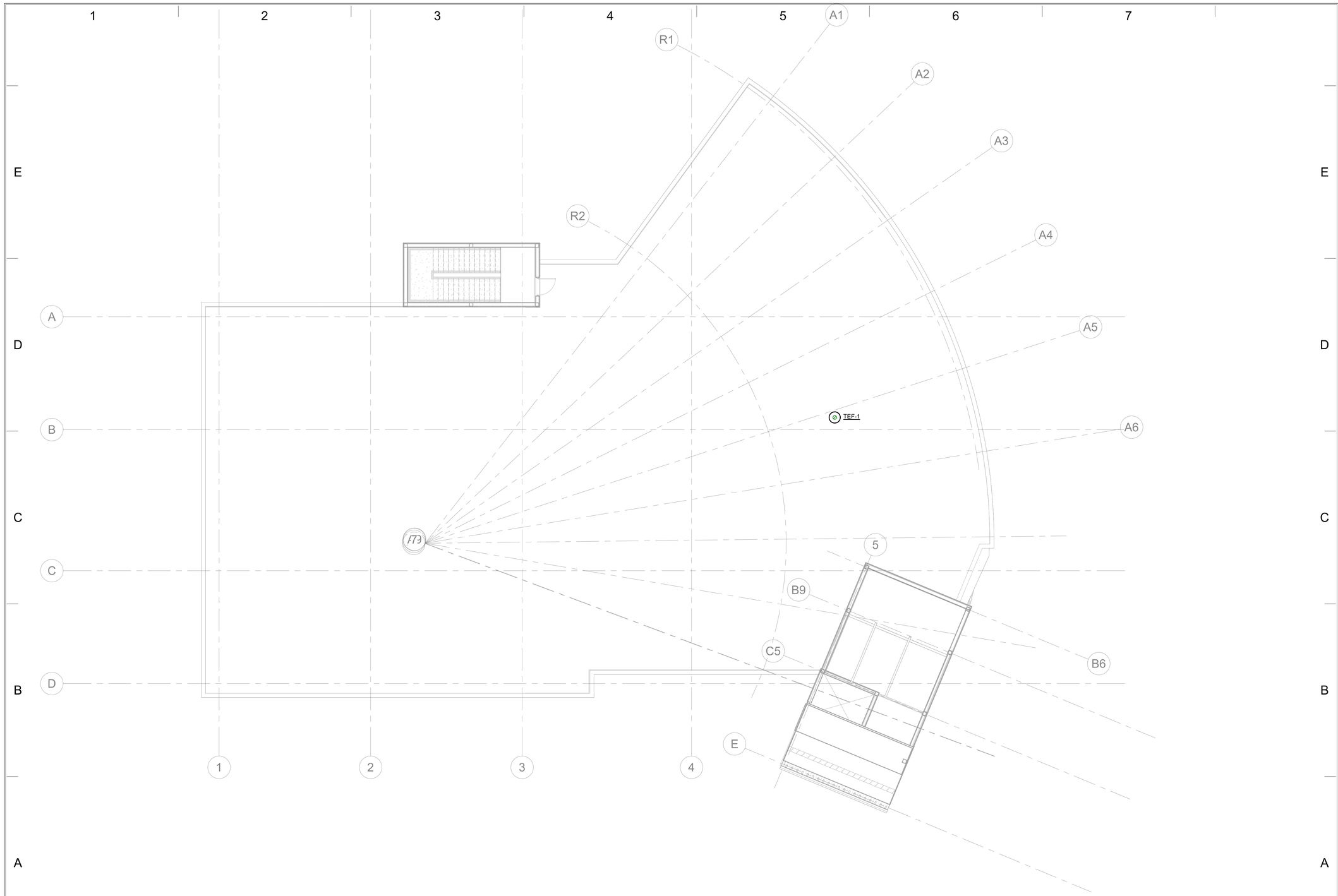
SHEET NO.: M1.2

MECHANICAL FLOOR PLAN - LEVEL 2



1 M1.3 ROOF LEVEL - MECHANICAL PLAN
1/8" = 1'-0"

REVISIONS

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	01/10/2025	90% Progress Set

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MECHANICAL ROOF PLAN

ABBREVIATIONS

A/C	ABOVE CEILING
AD	ACCESS DOOR
ADJ	ADJUSTABLE
AFF	ABOVE FINISHED FLOOR
AUTO	AUTOMATIC
AC	AIR CONDITIONING
AHU	AIR HANDLING UNIT
BAL	BALANCING
BDD	BACKDRAFT DAMPER
B/F	BELOW FLOOR
B/G	BELOW GRADE
B'FLY	BUTTERFLY
BHP	BRAKE HORSEPOWER
BCO	BASE CLEANOUT
CFM	CUBIC FEET PER MINUTE
CBCR	CURVED BLADE CEILING REGISTER
CD	CEILING DIFFUSER
CU	CONDENSING UNIT
CW	COLD WATER (DOMESTIC)
CHWS	CHILLED WATER SUPPLY
CHWR	CHILLED WATER RETURN
CWS	CONDENSER WATER SUPPLY
CWR	CONDENSER WATER RETURN
CON	CONCENTRIC
CO	CLEANOUT
COND	CONDENSATE
db	DRY BULB
DN	DOWN
DR	DRAIN
do	DITTO
dB	DECIBELS
DWG	DRAWING
EA	EACH
EAT	ENTERING AIR TEMPERATURE
ECC	ECCENTRIC
EF	EXHAUST FAN
EOD	EMERGENCY OVERFLOW DRAIN
ER	EXHAUST REGISTER
ESP	EXTERNAL STATIC PRESSURE
EWT	ENTERING WATER TEMPERATURE
EXH	EXHAUST
EFF	EFFICIENCY
F	FAHRENHEIT
FCO	FLOOR CLEANOUT
FCU	FAN COIL UNIT
FSD	FIRE/SMOKE DAMPER
FD	FIRE DAMPER OR FLOOR DRAIN
FL DR	FLOOR DRAIN (only)
FLR	FLOOR
FOB	FLAT ON BOTTOM
FOR	FUEL OIL RETURN
FOS	FUEL OIL SUPPLY
FOT	FLAT ON TOP
FPM	FEET PER MINUTE
FPS	FEET PER SECOND
FT	FEET
G	GATE
GA	GAUGE
GPM	GALLONS PER MINUTE
GL	GLOBE
GCO	GRADE CLEANOUT
HD	HUB DRAIN
HP	HORSEPOWER
HTG	HEATING
HW	HOT WATER (DOMESTIC)
HWR	HOT WATER RETURN
HWRR	HOT WATER REVERSE RETURN
HWS	HOT WATER SUPPLY
Hz	HERTZ
ID	INSIDE DIMENSION
IN	INCHES
KW	KILOWATTS
LAT	LEAVING AIR TEMPERATURE
LB	POUNDS
LG	LINEAR GRILLE
LRG	LINEAR RETURN GRILLE
LWR	LOOP WATER RETURN
LWS	LOOP WATER SUPPLY
MIN	MINIMUM
MAX	MAXIMUM
MD	MANUAL DAMPER
MOD	MOTOR OPERATED DAMPER
MFR	MANUFACTURER
NC	NORMALLY CLOSED
NG	NATURAL GAS
NFWH	NON-FREEZE WALL HYDRANT
NO	NORMALLY OPEN
NOM	NOMINAL
OA	OUTSIDE AIR
OD	OUTSIDE DIMENSION
OBD	OPPOSED BLADE DAMPER
PIU	POWERED INDUCTION UNIT
PSI	POUNDS PER SQUARE INCH
RA	RETURN AIR
RAD	RADIUS
RAG	RETURN AIR GRILLE
RED	REDUCER
RL	REFRIGERANT LIQUID
RS	REFRIGERANT SUCTION
RTU	ROOFTOP UNIT
RAR	RETURN AIR REGISTER
SP	STATIC PRESSURE
SPS	STATIC PRESSURE SENSOR
SA	SUPPLY AIR
SAN	SANITARY
SD	SMOKE DAMPER
SEN	SENSIBLE
SQ	SQUARE
SR	SUPPLY REGISTER
ST	STORM
SS	SPLIT SYSTEM
TEMP	TEMPERATURE
TG	TRANSFER GRILLE
TYP	TYPICAL
UON	UNLESS OTHERWISE NOTED
V	VENT
VA	VALVE
VTR	VENT THRU ROOF
VAV	VARIABLE AIR VOLUME
wb	WET BULB
WC	WATER COLUMN
WHA	WATER HAMMER ARRESTOR
WT	WEIGHT
W	WASTE

PLUMBING LEGEND

	COLD WATER
	HOT WATER
	HOT WATER RECIRCULATION
	VENT
	SANITARY WASTE
	GAS
	WORK TO BE REMOVED
	FLOOR DRAIN
	HOSE BIBB
	FLOOR CLEAN-OUT
	WALL CLEAN-OUT
	GATE VALVE
	BALL VALVE

PUMP SCHEDULE

I.D. TAG	FLUID	TYPE OF PUMP	FLOW (GPM)	HEAD (FT.)	MAX. NPSHR (FT.)	MAX. RPM	MOTOR H.P.	VOLTS/ PHASE	MINIMUM EFFICIENCY (%)	BASIS OF DESIGN	REMARKS
BP-1	WATER	WELL PUMP	20	70	-	-	1	120/1	-	GRUNDFOS JP PS 16 10 187	(4)
HWRP-1	WATER	RECIRC.	5	20	-	-	1/12	120/1	-	B&G SERIES PL	(1)
SP-1	WATER	SUMP	50	20	-	3,400	1/2	120/1	-	B&G 1EC05	(2) (3)

(1) PROVIDE WITH STARTER, DISCONNECT, TIMER AND AQUASTAT.
(2) PROVIDE STARTER/DISCONNECT/CONTROLLER WITH SINGLE POINT POWER CONNECTION. COORD. W/ DIV 26.
(3) SEE DETAIL 4/P0.00. PROVIDE WITH ALARM PANEL, FLOAT SWITCHES, OIL DETECTION SYSTEM, ETC. FOR A COMPLETE SUMP PUMP PACKAGE. ALARM PANEL SHALL BE TIED IN TO BUILDING EMS FOR MONITORING. PANEL SHALL BE POWERED INDEPENDENTLY FROM PUMPS. ALARM PANEL SHALL INCLUDE BUILT-IN AUDIBLE & VISUAL ALARM WHEN PUMP DOES NOT RUN DUE TO OIL IN PIT OR HIGH LIQUID CONDITION. COORD. W/ DIV. 26.
(4) SELF-PRIMING, BASE-MOUNTED WELL PUMP WITH INTEGRAL PRESSURE SWITCH. PROVIDE WITH BLADDER TYPE PRESSURE TANK.

PLUMBING FIXTURE SCHEDULE

TAG	DESCRIPTION	WASTE	VENT	CW	HW	FLOW	BASIS OF DESIGN
P-1	WATER CLOSET	3"	2"	1"	-	1.28 GPF	SLOAN MODEL WETS-2450.1401-CO, OR EQUAL, WATER CLOSET & FLUSHOMETER COMBO. WATER CLOSET SHALL BE WALL HUNG, VITREOUS CHINA, WITH ELONGATED BOWL AND OFFSET EMBODIED CARBON. FLUSH VALVE SHALL BE SENSOR ACTIVATED, BATTERY POWERED. PROVIDE WITH BATTERIES AND SEAT. HANDICAP FIXTURES SHALL BE MOUNTED IN COMPLIANCE WITH ADA REQUIREMENTS.
P-2A	LAVATORY	2"	1-1/2"	1/2"	1/2"	0.35 GPM	JUST MANUFACTURING COMPANY MODEL UCIR-16 UNDERMOUNT CIRCULAR LAVATORY WITH SLOAN MODEL EAF-350-BAT-ISM-GR-0.35GPM AUTOMATIC FAUCET AND SLOAN MODEL ESD-600-GR SOAP DISPENSER. SINK SHALL BE 18 GAUGE, TYPE 304 STAINLESS STEEL. FAUCET SHALL BE BATTERY POWERED, ELECTRONIC FAUCET WITH 0.35 GPM AERATOR, GRAPHITE FINISH, BATTERIES, MIXING VALVE, CHROME-PLATED TAILPIECES, CHROME GRID STRAINER, P-TRAP, SUPPLY STOPS AND ALL OTHER TRIM. FIXTURES SHALL BE MOUNTED IN COMPLIANCE WITH ADA REQUIREMENTS. SOAP DISPENSER SHALL BE BATTERY POWERED, GRAPHITE FINISH, SENSOR ACTIVATED, WITH TWO BOTTLES OF 1,500 mL SOAP.
P-2B	LAVATORY	2"	1-1/2"	1/2"	1/2"	0.35 GPM	SLOAN MODEL SS-3003-CO WALL HUNG LAVATORY WITH SLOAN MODEL EAF-350-BAT-ISM-CP-0.35GPM AUTOMATIC FAUCET AND SLOAN MODEL ESD-600-CP SOAP DISPENSER. SINK SHALL BE VITREOUS CHINA, WALL HUNG, WITH CONCEALED CARRIER AND ANCHORING SCREWS, OFFSET EMBODIED CARBON, AND 1-1/4" DRAIN. COORDINATE FAUCET HOLES WITH FAUCET. FAUCET SHALL BE BATTERY POWERED, ELECTRONIC FAUCET WITH 0.35 GPM AERATOR, FOUR AA BATTERIES, MIXING VALVE, CHROME-PLATED TAILPIECES, CHROME GRID STRAINER, PTRAP, SUPPLY STOPS AND ALL OTHER TRIM. FIXTURES SHALL BE MOUNTED IN COMPLIANCE WITH ADA REQUIREMENTS. SOAP DISPENSER SHALL BE BATTERY POWERED, CHROME FINISH, SENSOR ACTIVATED, WITH TWO BOTTLES OF 1,500 mL SOAP.
P-4	KITCHEN SINK	2"	1-1/2"	1/2"	1/2"	1.5 GPM	JUST MANUFACTURING COMPANY MODEL USN-1824-A UNDERMOUNT SINK WITH DELTA MODEL 4159-BL-DST KITCHEN FAUCET. SINK SHALL BE 18 GAUGE, TYPE 304 STAINLESS STEEL. FAUCET SHALL BE INCLUDE RP61607 1.5 GPM HOSE ADAPTOR, SINGLE HANDLE DECK MOUNT, MATTE BLACK FINISH, 2-FUNCTION PULL-OUT SPRAYER. COORDINATE HOLE PUNCHING WITH FAUCET. FIXTURES SHALL BE MOUNTED IN COMPLIANCE WITH ADA REQUIREMENTS. COORDINATE ESCUTCHEON, FINISH, AND SOAP DISPENSER REQUIREMENTS WITH ARCHITECT.
TP	TRAP PRIMER	-	-	1/2"	-	-	PRECISION PLUMBING PRODUCTS INC., P1-500 PRIME RITE MACHINED BRASS BODY, INLET 1/2" MALE NPT FITTING, OUTLET 1/2" FEMALE NPT. INSTALL IN AN ACCESSIBLE AREA WITH STAINLESS STEEL ACCESS DOOR. COMPLETE WITH A BALL VALVE BEFORE TRAP PRIMER. DISTRIBUTION UNITS AS REQUIRED.
FD	FLOOR DRAIN	(2)	2"	(1)	-	-	FLOOR DRAINS IN MECHANICAL ROOMS AND UNFINISHED CONCRETE FLOORS SHALL BE JR SMITH 2131 SERIES WITH ROUND 11-3/4" CAST IRON GRATE, SEDIMENT BUCKET AND DEEP-SEAL P-TRAP; OR EQUAL PRODUCTS BY JOSAM OR ZURN. PROVIDE VANDALPROOF SECURED TOPS.

(1) PROVIDE WITH 1/2" AUTOMATIC TRAP PRIMERS
(2) REFER TO PLANS FOR CONNECTION SIZE.

ELECTRIC WATER HEATER SCHEDULE

I.D. TAG	TYPE OF WATER HEATER	TANK CAPACITY (GALLONS)	RECOVERY @ 80°F (GPH)	POWER INPUT (KW)	VOLTS/ PHASE	BASIS OF DESIGN	REMARKS
WH-1	ELEC TANK	20	63	3.0	208/3	A.O. SMITH DEL	(1)

(1) PROVIDE EXPANSION TANK ON CW INLET; WATTS SERIES PLT OR EQUAL SIZED PER MANUFACTURER'S RECOMMENDATIONS.

REVISIONS

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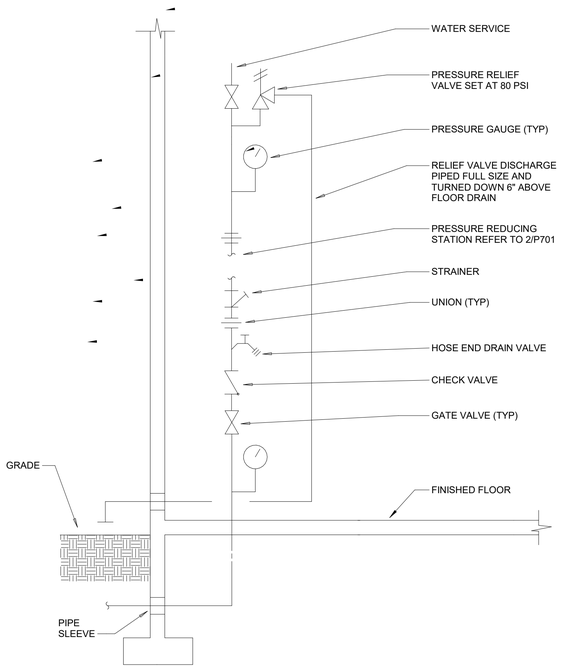
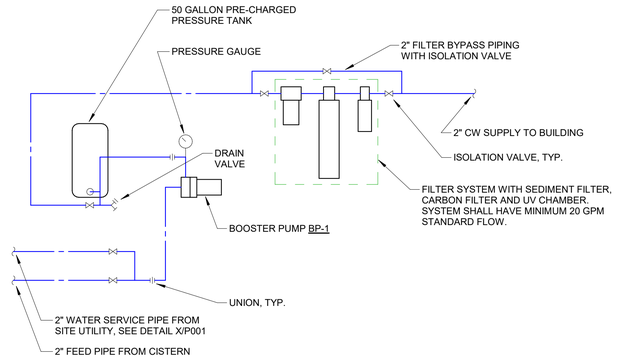
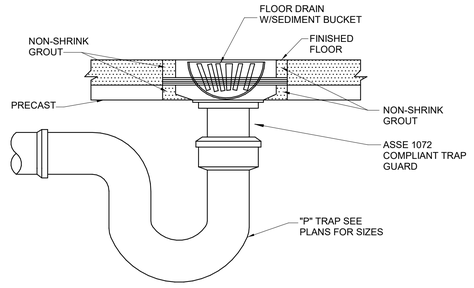


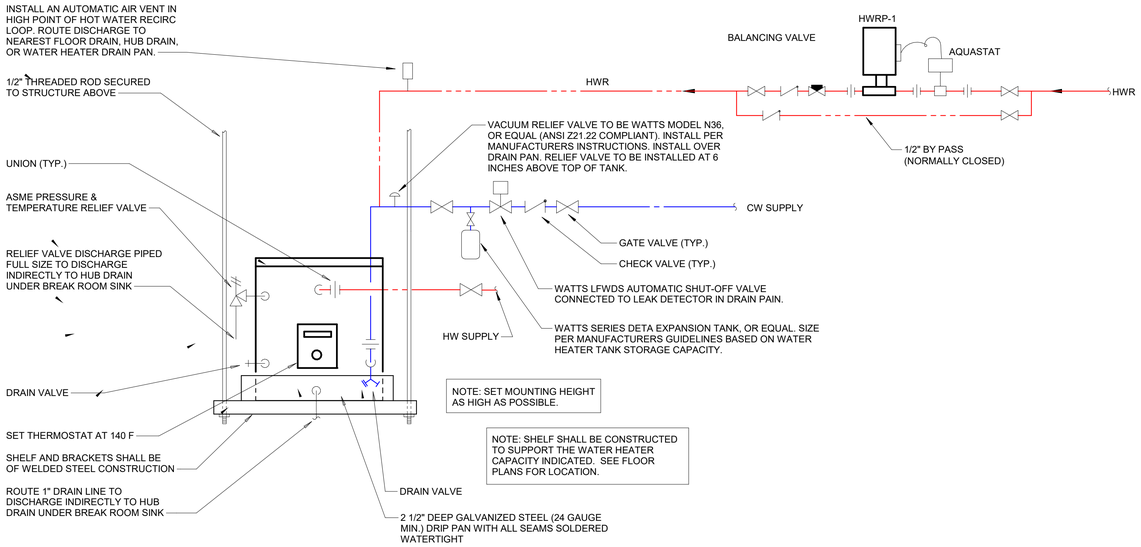
1 P0.1 WATER ENTRANCE DETAIL
NOT TO SCALE



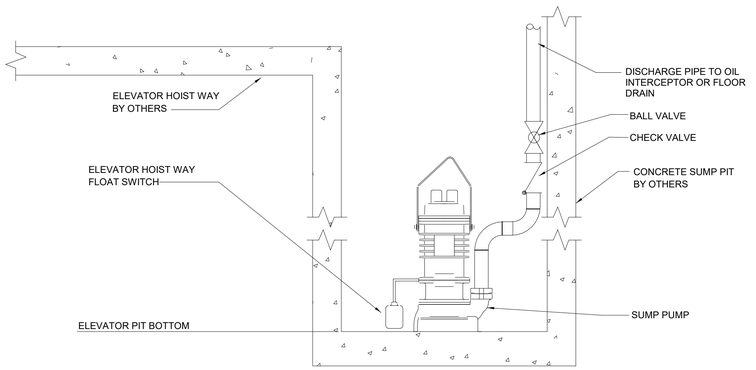
2 P0.1 BOOSTER PUMP AND FILTER DETAIL
3/8" = 1'-0"



5 P0.1 FLOOR DRAIN DETAIL
NOT TO SCALE



3 P0.1 WATER HEATER DETAIL
12" = 1'-0"



4 P0.1 ELEVATOR SUMP PUMP DETAIL
NOT TO SCALE

REVISIONS

NO.	DATE	REMARKS
	01/10/2025	90% Progress Set

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

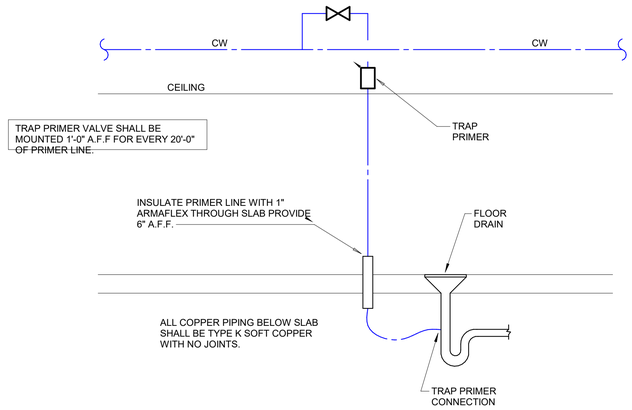
NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

DATE: 01/10/2025
DRAWN BY: W. CASAL
CHECKED BY: E. CURTIS
APPROVED BY: J. CLARK

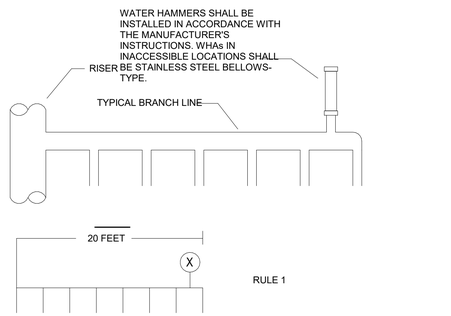
PROJECT NO.: 2022.022.00
CONTRACT NO.: -

SHEET NO.: P0.1

PLUMBING DETAILS



1 P0.2 TRAP PRIMER CONNECTION DETAIL
NOT TO SCALE



RULE 1: COVERS MULTIPLE FIXTURE BRANCH LINES WHICH DO NOT EXCEED

RULE 2: COVERS MULTIPLE FIXTURE BRANCH LINES WHICH DO EXCEED 20' IN LENGTH.

*** NOTE: REFER TO STANDARD PDI-WH 201 FOR SIZING AND INSTALLATION.

2 P0.2 WATER HAMMER ARRESTOR DETAIL
NOT TO SCALE

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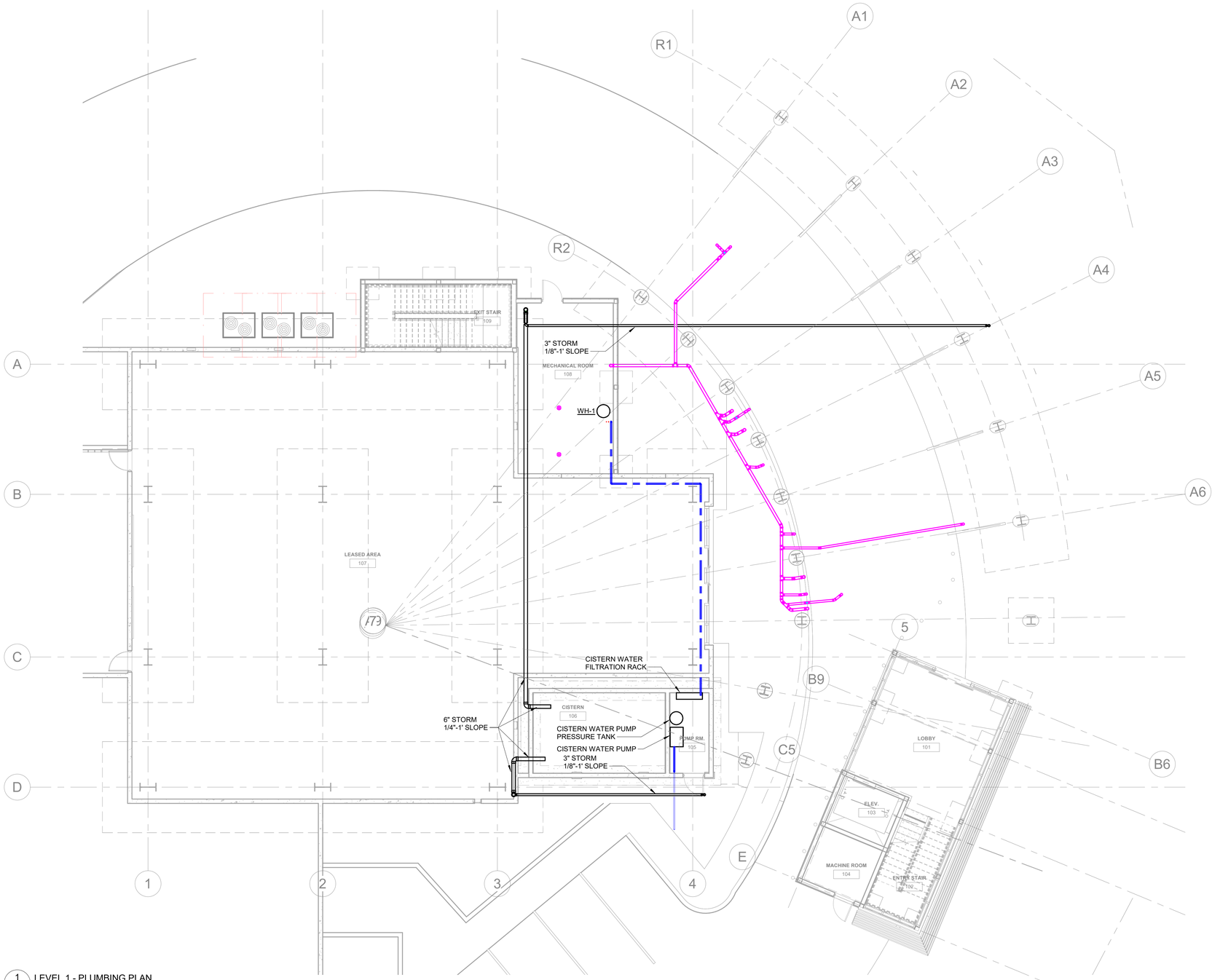
CONTRACT NO.: -

SHEET NO.: P0.2

PLUMBING DETAILS



1 LEVEL 1 - PLUMBING PLAN
P1.1 1/8" = 1'-0"

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PLUMBING FLOOR PLAN - LEVEL 1

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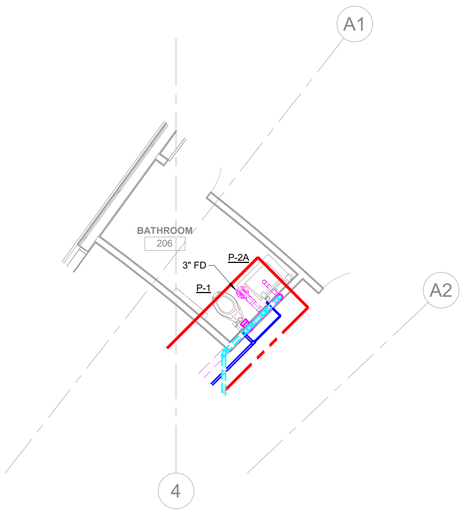
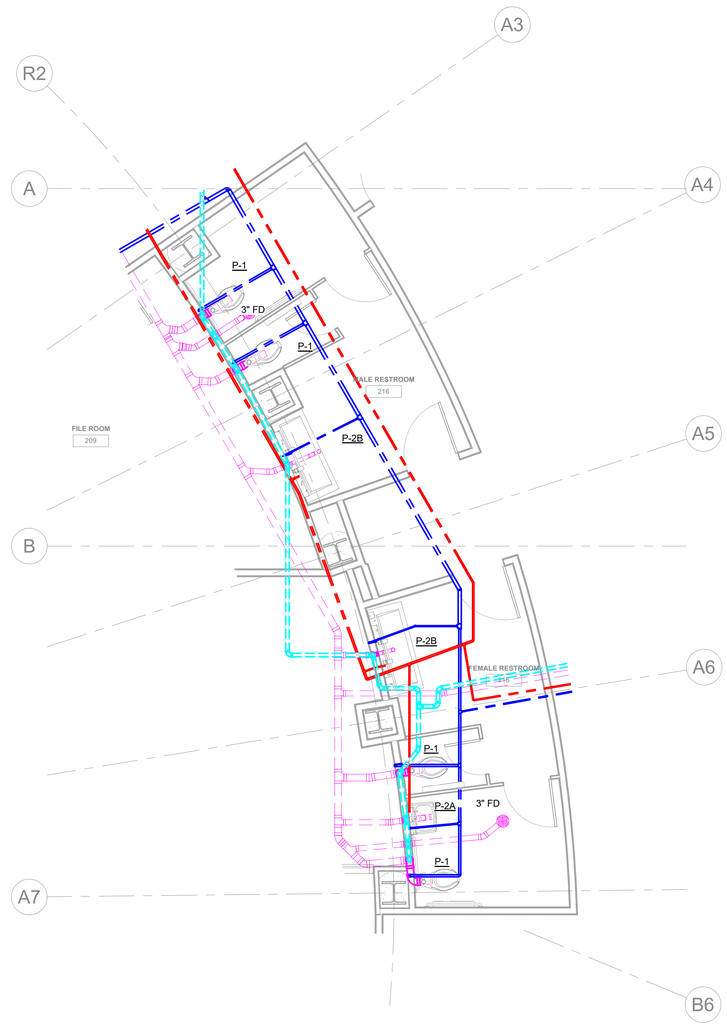
SHEET NO.: P1.2

PLUMBING FLOOR PLAN - LEVEL 2



1 P2.0 ENLARGED CEO BATHROOM PLAN
1/4" = 1'-0"



2 P2.0 ENLARGED GROUP RESTROOM PLAN
1/4" = 1'-0"

REVISIONS

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CONTRACT NO.: -

SHEET NO.: P2.0

PLUMBING ENLARGED VIEWS

LIGHTING FIXTURE SCHEDULE
(SPECIFIED BY ENGINEER)

TYPE	DESCRIPTION
A	2'X4' LED SPECIFICATION GRADE TROFFER. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. LAMPS: TBD MANUF: TBD
AE	SAME AS 'A' BUT WITH 90-MIN BATTERY BACKUP.
B	2'X2' LED SPECIFICATION GRADE TROFFER. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. LAMPS: TBD MANUF: TBD
BE	SAME AS TYPE 'B' BUT WITH 90-MINUTE BATTERY BACKUP.
C	PENDANT MOUNTED CYLINDER DOWNLIGHT. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. LAMPS: TBD MANUF: TBD
CE	SAME AS TYPE 'C' BUT WITH 90-MIN BATTERY BACKUP.
D	4' CHAIN-HUNG LED UTLITY STRIP. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. COORDINATE COLOR TEMPERATURE, FINISH, AND ALL OTHER OPTIONS WITH ARCHITECT. LAMPS: TBD MANUF: TBD
DE	SAME AS TYPE 'D', BUT WITH 90-MINUTE BATTERY BACKUP.
EE	4' WALL MOUNTED LED UTILITY STRIP. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. COORDINATE COLOR TEMPERATURE, FINISH, AND ALL OTHER OPTIONS WITH ARCHITECT. PROVIDE WITH 90-MINUTE BATTERY BACKUP. PROVIDE WITH INTEGRAL MOTION SENSOR SUCH THAT FIXTURE DIMS TO 50% WHEN NO OCCUPANCY IS SENSED. LAMPS: TBD MANUF: TBD
F	4' RECESSED LINEAR. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. LAMPS: TBD MANUF: TBD
FE	SAME AS TYPE 'F' BUT WITH 90-MINUTE BATTERY BACKUP.
G	1'X4' RECESSED LINEAR. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. LAMPS: TBD MANUF: TBD
X	LED EDGE-LIT EXIT SIGN WITH CONTINUOUS ILLUMINATION. PROVIDE WITH UNIVERSAL VOLTAGE DRIVER. PROVIDE MIRRORED BACKING FOR SINGLE FACED. PROVIDE DIRECTIONAL ARROWS, DOUBLE OR SINGLE FACE AS INDICATED ON DRAWINGS. PROVIDE WITH 90-MINUTE BATTERY BACKUP. LAMPS: COOPER ES SERIES

ELECTRICAL GENERAL NOTES

1. ALL WORK IN THIS DIVISION SHALL COMPLY WITH ALL LOCAL BUILDING CODES, LAWS, REGULATIONS, ORDINANCES, AND THE REQUIREMENTS OF THE 2020 NATIONAL ELECTRICAL CODE. ALL WORK SHALL COMPLY WITH BASE BUILDING SPECIFICATIONS. OBTAIN A COPY OF SPECIFICATIONS FROM BUILDING LANDLORD IF NECESSARY.
2. THE CONTRACTOR SHALL KEEP A RECORD OF THE CHANGES WHICH ARE IN CONFLICT WITH THESE DRAWINGS AND SPECIFICATIONS. AT THE COMPLETION OF HIS WORK HE SHALL SUBMIT "AS BUILT" PRINTS TO THE OWNER.
3. DRAWINGS ARE GENERALLY DIAGRAMMATIC AND DO NOT NECESSARILY SHOW EVERY FITTING AND DETAIL. ALL WORK SHALL BE INSTALLED SO THAT JUNCTION BOXES AND COMPONENTS WILL BE ACCESSIBLE FOR SERVICE.
4. ALL SYSTEMS, EQUIPMENT, COMPONENTS, WORK, ETC. PROVIDED UNDER THIS DIVISION SHALL BE COVERED BY A ONE YEAR GUARANTEE STARTING AT THE TIME OF FINAL ACCEPTANCE OF THE WORK BY THE OWNER. ANY DEFECTS IN THE WORK, SYSTEMS, EQUIPMENT, OR COMPONENTS FOUND DURING THIS YEAR SHALL BE CORRECTED AT NO CHARGE. THE GUARANTEE SHALL INCLUDE PROVIDING ALL NECESSARY CUTTING, PATCHWORK, REPAINTING, ETC. TO MAKE THE WORK COMPLETE AND NEW.
5. ALL WORK MUST BE PERFORMED IN A NEAT AND WORKMANLIKE MANNER ACCORDING TO GENERALLY ACCEPTED PRINCIPALS OF FIRST CLASS WORKMANSHIP.
6. ALL ROOF PENETRATIONS SHALL BE AS AUTHORIZED BY ARCHITECT AND OWNER. MAINTAIN ROOF WATERTIGHT INTEGRITY.
7. ALL PENETRATIONS THRU RATED WALLS, FLOORS AND CEILINGS SHALL BE FIRE STOPPED PER NEC.
9. DEVICE BOXES IN RATED WALLS SHALL MEET INTERNATIONAL BUILDING CODE SECTION 706.4.
10. COORDINATE ALL FLOOR CORES AND FLOOR BOXES WITH ARCHITECT AND OWNER.
11. ARC-FLASH HAZARD WARNING SHALL BE PROVIDED ON ALL EQUIPMENT IN AFFECTED ELECTRICAL ROOMS PER NEC 110.16.
12. ALL CONDUCTORS SHALL BE COPPER WITH TYPE "THW" OR "THHN" INSULATION AND THE MINIMUM WIRE SIZE SHALL BE #12 AWG WITH A 167 DEGREE TEMPERATURE RATING.
13. MULTI-WIRE BRANCH CIRCUITS SHALL BE PROVIDED WITH A MEANS THAT WILL SIMULTANEOUSLY DISCONNECT ALL UNGROUNDED CONDUCTORS PER N.E.C. 210.4(B).
14. NEUTRAL CONDUCTORS FOR CONNECTION TO ALL FURNITURE SYSTEMS TO BE #10AWG.
15. PROVIDE ALL GROUNDING AS REQUIRED BY N.E.C.
16. PROVIDE A SEPARATE GREEN, INSULATED, #12 AWG EQUIPMENT GROUNDING CONDUCTOR ROUTED WITH THE BRANCH CIRCUIT HOMERUN CONDUCTORS.
17. PROVIDE #12 AWG GND FOR ALL MECHANICAL EQUIPMENT UNLESS SHOWN OTHERWISE. ALL EQUIPMENT SHALL BE GROUNDED AT THE PANEL WHICH FEEDS THE EQUIPMENT.
18. ALL CONDUIT MUST BE CONCEALED IN THE WALLS OR ABOVE THE CEILING UNLESS OTHERWISE NOTED. MINIMUM CONDUIT SIZE IS 3/4".
19. ALL CABLE USED IN PLENUM SHALL BE PLENUM-RATED.
20. ARMORED CABLE MAY BE USED IN WALLS AND MILLWORK ONLY AND MUST BE MC TYPE (WITH GROUND). ALL CONDUIT TO AND ABOVE THE CEILING SHALL BE EMT.
21. ARMORED CABLE MAY BE USED FOR LIGHTING CIRCUITING ABOVE CEILINGS AND MUST BE MC TYPE (WITH GROUND). PROVIDE EMT FOR CIRCUIT HOMERUNS.
22. PROVIDE A JUNCTION BOX WITH A 1" CONDUIT STUBBED UP 6" ABOVE ACCESSIBLE CEILING OR TO TELEPHONE BACKBOARD FOR ALL NEW DATA AND TELEPHONE OUTLETS. PROVIDE PULL STRING AND BUSHING AT THE TERMINATION ABOVE ACCESSIBLE CEILING. EXISTING DATA CABLES ARE TO BE REMOVED WHERE NOT BEING UTILIZED. COORDINATE WITH THE TENANT.
23. DUPLEX RECEPTACLES SHALL BE PLASTIC, TWO-POLE, THREE WIRE, SELF-GROUNDING, SIDE-WIRED, 125 VOLTS AND 20A RATED, AND MATCH EXISTING IF POSSIBLE.
24. DEDICATED RECEPTACLES TO RECEIVE VISUAL DESIGNATION.
25. COORDINATE RECEPTACLE NEMA TYPE AND VOLTAGE WITH COPIERS AND EQUIPMENT PRIOR TO INSTALLATION.
26. DEVICE MOUNTING HEIGHTS ARE TO BE MEASURED TO THE DEVICE CENTERLINE UNLESS NOTED OTHERWISE.
27. PROVIDE A LAMINATED LABEL ON COVERPLATE FOR ALL OUTLETS IDENTIFYING PANEL AND BREAKER NUMBER FROM WHICH IT IS FED.
28. BLANK FACEPLATES ARE NOT ALLOWED, U.N.O. ANY EXISTING OUTLET OR TELE/DATA LOCATION NOT USED OR SHOWN WITHIN THE SCOPE OF WORK IN THESE PLANS SHOULD BE REMOVED, PATCHED, AND REPAIRED.
29. ALL SWITCHES FOR FANS, LIGHTS, ETC. WHICH ARE SHOWN TO BE MOUNTED IN THE SAME GENERAL AREA SHALL SHARE A MULTI-GANG COVER PLATE AS REQUIRED.
30. ALL DIMMERS SHALL BE LUTRON NOVA T-STAR SERIES UNLESS NOTED OTHERWISE. DIMMER SWITCH SHALL BE FULLy COMPATIBLE WITH THE LIGHT FIXTURES IT CONTROLS. COORDINATE DIMMING PROTOCOL (0-10V, ELV, ETC.) WITH LIGHT FIXTURE MANUFACTURER. COORDINATE FINISH WITH ARCHITECT.
31. FASTEN ALL RECESSED LIGHTING FIXTURES TO STRUCTURE OR GRID PER NEC.
32. PROVIDE A NEUTRAL CONDUCTOR FOR SWITCHES CONTROLLING LIGHTING LOADS PER NEC 404.2(C). CAP NEUTRAL CONDUCTOR WHERE NOT REQUIRED BY MANUFACTURER.
33. LIGHTING CONTROL SYSTEMS SHALL MEET 2009 IECC.
34. WHERE WORK BY THE GENERAL CONTRACTOR (WALL REMOVAL, NEW OR RELOCATED WALL OPENING, ETC.) RESULTS IN THE REMOVAL, RELOCATION OR REFEEDING OF EXISTING TO REMAIN ELECTRICAL DEVICES OR LIGHTING FIXTURES, THE ELECTRICAL CONTRACTOR SHALL DISCONNECT OR RECONNECT AS REQUIRED ALL ACTIVE DEVICES REMAINING ON THAT CIRCUIT SYSTEM.
35. WHERE DEMOLITION DISRUPTS ELECTRICAL CONTINUITY OF EXISTING TO REMAIN RECEPTACLES/LIGHTS, AND NO RECONNECTION IS SHOWN, RECONNECT TO ITS EXISTING CIRCUIT.
36. WHERE RECEPTACLES ARE REMOVED OR MOVED, REUSE EXISTING CIRCUITRY IF POSSIBLE.
37. ALL EXISTING POWER DISTRIBUTION, FIRE ALARM, ETC. JUNCTION BOXES SHALL BE RELOCATED IF LOCATED OVER A CEILING SCHEDULED TO BE GYPSUM BOARD. ALL EXISTING JUNCTION BOXES SHALL BE RELOCATED IF NECESSARY TO AN ACCESSIBLE LOCATION.
39. RING OUT ALL CIRCUITS IN EXISTING PANEL(S) (IF APPLICABLE) AFFECTED BY THIS ALTERATION. WHERE ADDITIONAL CIRCUITS ARE NEEDED, REUSE CIRCUITS AVAILABLE FOR REUSE, OR PROVIDE NEW BREAKERS, TAG ALL UNUSED CIRCUITS AS SPARE, REPLACE ALL INOPERATIVE OR DEFECTIVE CIRCUIT BREAKERS. TIGHTEN ALL CONNECTIONS.
40. REUSE EXISTING HOMERUNS WHERE APPLICABLE WITHIN THE DESIGN CRITERIA OF THESE DRAWINGS. ALL HOMERUNS ARE TO BE PROTECTED BY 20A/1P BREAKERS UNLESS OTHERWISE NOTED.
41. REMOVE ALL ABANDONED CIRCUITING, WIRING, CABLING, AND CONDUIT SYSTEMS FOR POWER, LOW VOLTAGE CONTROLS AND COMMUNICATIONS BACK TO SOURCE.
42. PROVIDE ENGRAVED NAMEPLATE ON ALL NEW PANELS INDICATING PANEL NAME AND SOURCE PER NEC 408.4(B).
43. PROVIDE A UPDATED, PRINTED PANEL SCHEDULE FOR ALL PANELS MODIFIED WITHIN SCOPE OF WORK. CORRECTLY LABEL ALL EXISTING CIRCUITS, NEW CIRCUIT, SPARES AND SPACES IN ACCORDANCE WITH NEC 408.4.
44. CONTRACTOR SHALL CLEAN SITE AT END OF PROJECT. ALL DUST, DEBRIS, OILS, SPRAYS, FINGERPRINTS, AND LABELS SHALL BE REMOVED FROM ALL EXPOSED FINISHED SURFACES. ELECTRICAL AND TELEPHONE ROOMS SHALL BE PUT BACK AS FOUND; FLOORS ARE TO BE SWEPT, MOPPED, AND REPAINTED.

ELECTRICAL SYMBOL LEGEND

SYMBOL	DESCRIPTION	ON CENTER MTG. HT.
	CONCEALED CONDUIT IN CEILING OR WALL	
	CONCEALED CONDUIT IN FLOOR OR UNDERGROUND	
	CIRCUIT HOMERUN TO PANEL: EACH ARROWHEAD = 1 CIRCUIT 2#12,1#12G.,1/2"C. UNLESS NOTED OTHERWISE	
	NO. OF CONDUCTORS IN CONDUIT; EACH CROSSHATCH = 1 WIRE	
	CONDUIT STUBBED UP OR TURNED DOWN	
	MULTI OUTLET SURFACE MOUNTED RACEWAY	
	PLYWOOD BACKBOARD	
	WALL MOUNTED DUPLEX RECEPTACLE OUTLET	18"
	WALL MOUNTED G.F.C.I. DUPLEX RECEPTACLE OUTLET	18"
	WALL MOUNTED DEDICATED DUPLEX RECEPTACLE OUTLET	18"
	WALL MOUNTED DOUBLE DUPLEX RECEPTACLE OUTLET	18"
	WALL MOUNTED SPECIAL RECEPTACLE OUTLET	18"
J	JUNCTION BOX – SIZE AND MOUNTING AS REQUIRED	AS REQ'D.
J CP	JUNCTION BOX FOR CONNECTION OF CONDENSATE PUMP. COORDINATE WITH DIVISION 23.	AS REQ'D.
	WALL MOUNTED DATA/TELEPHONE OUTLET	18"
	120/208 VOLT PANELBOARD	
S	WALL MOUNTED S.P.S.T. TOGGLE SWITCH	48"
S LVD	WALL MOUNTED LOW VOLTAGE DIMMER SWITCH. INTERFACE WITH LOCAL SWITCHPACK SERVING THE AREA.	48"
S OS	WALL MOUNTED OCCUPANCY SENSOR SWITCH (GREENGATE ONW-D-1001-MV)	48"
S VS	WALL MOUNTED VACANCY SENSOR SWITCH (GREENGATE ONW-D-1001-MV)	48"
S LV	WALL MOUNTED LOW VOLTAGE MOMENTARY SWITCH. INTERFACE WITH LOCAL SWITCHPACK SERVING THE AREA. (GREENGATE GDMS SERIES)	48"
S D	WALL MOUNTED DIMMER SWITCH (WATTAGE AS REQ'D)	48"
MS	DUAL TECHNOLOGY CEILING MOUNTED OCCUPANCY SENSOR (GREENGATE OAC-DT-2000 SERIES)	
PP	OCCUPANCY SENSOR ON/OFF SWITCHPACK (GREENGATE SP20 SERIES)	
D	OCCUPANCY SENSOR DIMMING SWITCHPACK (GREENGATE SP20 SERIES)	
F	FIRE ALARM PULL STATION	42"
FF	FIRE ALARM ADA APPROVED AUDIO/VISUAL ALARM	84" A.F.F. or 6" BELOW CEILING, WHICHEVER IS LOWER
F	FIRE ALARM ADA APPROVED VISUAL ALARM – WALL MOUNTED	
S M	MOTOR RATED TOGGLE SWITCH	AS REQ'D.
30/3	DISCONNECT SWITCH (FRAME/POLES/FUSE-IF REQUIRED)	
	EXIT SIGN – CEILING,WALL MT.	
AFF/AFG	ABOVE FINISHED FLOOR/ABOVE FINISHED GRADE	
BC	BELOW CEILING	
AC	ABOVE COUNTER	
WP	WEATHER PROOF	
EX/R/N	EXISTING/RELOCATED/NEW	
M	MOTORIZED DAMPER. COORDINATE FINAL LOCATION AND CONNECTION REQUIREMENTS WITH DIVISION 23.	

REVISIONS

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CHECKED BY: J.P.C
APPROVED BY: J.P.C

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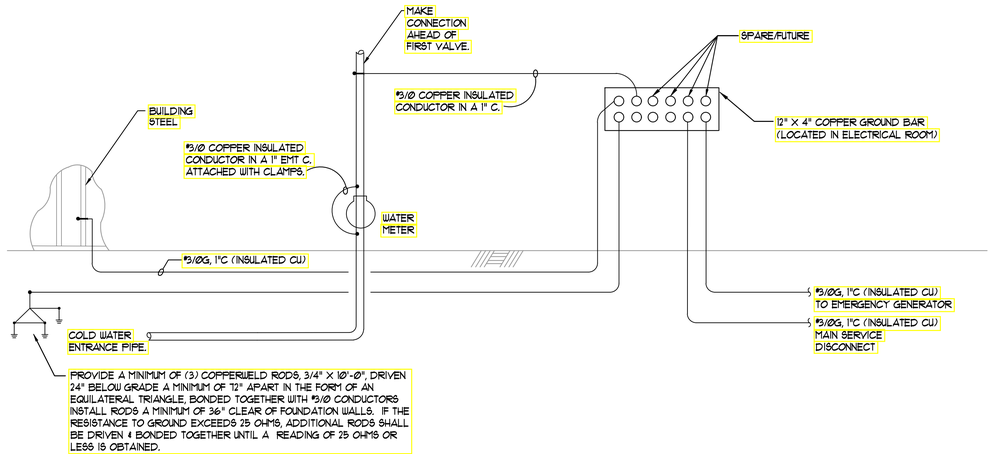
SHEET NO.: E0.0

ELECTRICAL LEGEND, SCHEDULES, AND GENERAL NOTES

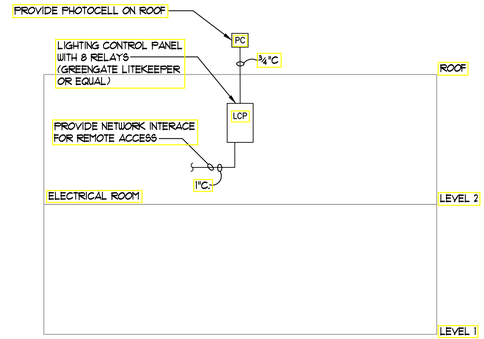


1 E001 **ELECTRICAL GROUND BAR DETAIL**

NOT TO SCALE

GENERAL NOTES:
(APPLY TO THIS DETAIL ONLY)

1. ALL GROUNDING CONDUCTORS SHALL BE INSULATED COPPER UNLESS NOTED OTHERWISE.
2. SEE ELECTRICAL RISER DIAGRAM FOR MORE INFORMATION REGARDING EQUIPMENT.
3. GROUND PER ARTICLE 250 OF THE NEC.
4. DRAWING IS DESIGN INTENT ONLY. FIELD COORDINATE EXACT LOCATION AND MOUNTING HEIGHT OF GROUND BAR.



2 E001 **LIGHTING CONTROL RISER DIAGRAM**

NO SCALE

NOTE:
THIS LIGHTING CONTROL SYSTEM SHALL AUTOMATICALLY SWITCH LIGHTING CIRCUITS ON AND OFF AT PREPROGRAMMED TIMES AS SCHEDULED BY THE OWNER. WHEN LIGHTS ARE IN THE OFF STATE, ACTUATION OF ANY OVERRIDE SWITCH WILL TURN THE LIGHTS IN THAT SWITCHES ZONE ON FOR UP TO (BUT NOT EXCEEDING) TWO HOURS.

LIGHTING CONTROL NOTES:

1. COORDINATE SETTINGS OF CONTROL SYSTEM WITH OWNER.
2. CONTRACTOR SHALL PROVIDE A COMPLETE INSTALLATION. PROVIDE ANY ADDITIONAL COMPONENTS AND ACCESSORIES AS NECESSARY. COORDINATE PROGRAMMING WITH OWNER.

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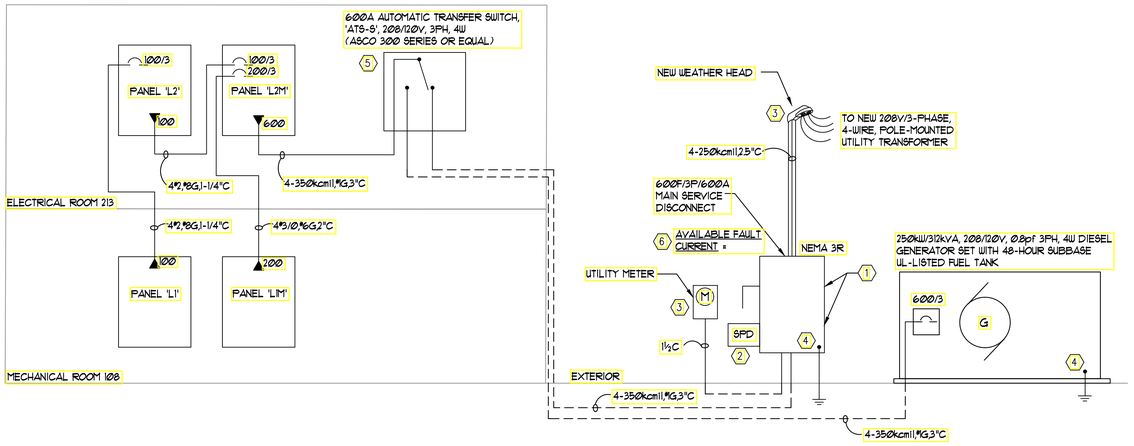
APPROVED BY:	J.P.C

PROJECT NO.: 2022.022.00

CONTRACT NO.: -

SHEET NO.: E0.1

ELECTRICAL DETAILS



1 E002 ELECTRICAL RISER DIAGRAM

NO SCALE

GENERAL NOTES:
(APPLY TO THIS SHEET ONLY)

1. WIRE SIZES ARE FOR COPPER CONDUCTORS.
2. CONTRACTOR TO VERIFY EQUIPMENT WILL FIT IN LOCATIONS AS SHOWN TO INSTALLATION.
3. REFER TO PANELBOARD SCHEDULES FOR ADDITIONAL INFORMATION.
4. PROVIDE ARC-FLASH RATINGS ON ALL PANELBOARDS.
5. AVAILABLE FAULT CURRENTS ARE INDICATED ADJACENT TO EACH PANELBOARD. WHERE NO VALUE IS SHOWN, THE CALCULATED VALUE IS LESS THAN THE MINIMUM RATING. PROVIDE FULLY RATED PANELS/BREAKERS THAT EQUAL OR EXCEED VALUES INDICATED FOR ALL PANELS RATED 800 AMPS AND ABOVE. ALL PANELS RATED LESS THAN 800 AMPS SHALL BE U.L. SERIES RATED TO MEET OR EXCEED VALUES INDICATED. SHOP DRAWINGS SHALL INCLUDE ALL RATINGS.
6. PROVIDE ENGRAVED NAMEPLATES FOR ALL ELECTRICAL EQUIPMENT INCLUDING TRANSFER SWITCH, PANELBOARDS, DISCONNECTS, ETC.

LEGEND NOTES:
(APPLY TO THIS SHEET ONLY)

1. PROVIDE WEATHERHEAD AND SERVICE-RATED FUSIBLE MAIN DISCONNECT SWITCH.
2. PROVIDE WITH TYPE 1 SURGE PROTECTION DEVICE EXTERNAL TO DISCONNECT. MAXIMUM FEEDER LENGTH SHALL BE 2', MINIMIZE BENDING OF CONNECTION. DEVICE SHALL BE CURRENT TECHNOLOGY SEL3 OR EQUAL BY EATON, ABB, OR SCHNEIDER ELECTRIC. PROVIDE WITH NEMA 3R ENCLOSURE.
3. COORDINATE ALL METERING REQUIREMENTS WITH POWER COMPANY. PROVIDE WEATHERHEAD AND CONDUIT WITH PIGTAIL AS SHOWN. PROVIDE AND INSTALL CURRENT TRANSFORMERS (CT'S) AND ALL ADDITIONAL MATERIAL AND/OR EQUIPMENT AS REQUIRED.
4. PROVIDE CONNECTION BETWEEN ALL SYSTEM GROUNDS (MAIN DISCONNECT AND GENERATOR) TO MAKE ONE GROUNDING SYSTEM.
5. PROVIDE START CONTACT MONITORING SOLUTION THAT CHECKS THE INTEGRITY OF THE GENERATOR REMOTE START CIRCUIT. THE CIRCUIT SHOULD BE MONITORED FOR BROKEN, DISCONNECTED, OR SHORTED WIRES. LOSS OF INTEGRITY SHALL ANNUNCIATE AN ALARM AT THE GENERATOR CONTROLLER AND SHALL START THE GENERATOR.
6. COORDINATE EXACT FAULT CURRENT WITH UTILITY COMPANY PRIOR TO RUNNING STUDIES AS REQUIRED BY SPECIFICATIONS.

REVISIONS

NO.	DATE	REMARKS
	01/10/2025	90% Progress Set

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PHASE 2 ECONOMIC DEVELOPMENT AUTHORITY INDUSTRIAL PARK DEVELOPMENT

NISKY 35 SOUTHSIDE QUARTER
SUB BASE, CROWN BAY
ST. THOMAS, USVI

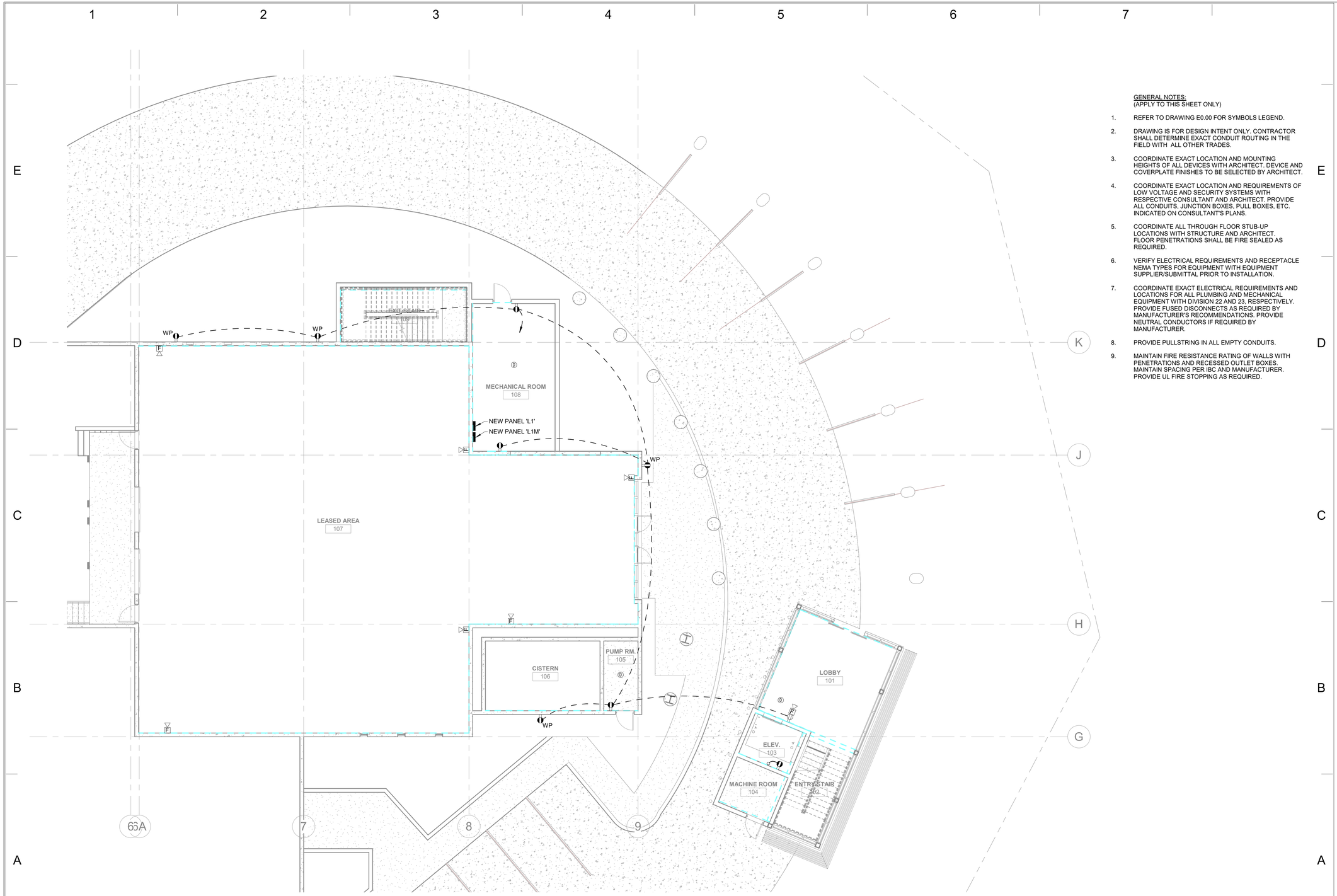
DATE:	01/10/2025
DRAWN BY:	L.D.O
CHECKED BY:	J.P.C
APPROVED BY:	J.P.C

PROJECT NO.: 2022.022.00

CONTRACT NO.: -

SHEET NO.: E0.2

ELECTRICAL RISER DIAGRAM



1 / E1.1 LEVEL 1 - ELECTRICAL PLAN
1/8" = 1'-0"

GENERAL NOTES:
(APPLY TO THIS SHEET ONLY)

1. REFER TO DRAWING E0.00 FOR SYMBOLS LEGEND.
2. DRAWING IS FOR DESIGN INTENT ONLY. CONTRACTOR SHALL DETERMINE EXACT CONDUIT ROUTING IN THE FIELD WITH ALL OTHER TRADES.
3. COORDINATE EXACT LOCATION AND MOUNTING HEIGHTS OF ALL DEVICES WITH ARCHITECT. DEVICE AND COVERPLATE FINISHES TO BE SELECTED BY ARCHITECT.
4. COORDINATE EXACT LOCATION AND REQUIREMENTS OF LOW VOLTAGE AND SECURITY SYSTEMS WITH RESPECTIVE CONSULTANT AND ARCHITECT. PROVIDE ALL CONDUITS, JUNCTION BOXES, PULL BOXES, ETC. INDICATED ON CONSULTANT'S PLANS.
5. COORDINATE ALL THROUGH FLOOR STUB-UP LOCATIONS WITH STRUCTURE AND ARCHITECT. FLOOR PENETRATIONS SHALL BE FIRE SEALED AS REQUIRED.
6. VERIFY ELECTRICAL REQUIREMENTS AND RECEPTACLE NEMA TYPES FOR EQUIPMENT WITH EQUIPMENT SUPPLIER/SUBMITTAL PRIOR TO INSTALLATION.
7. COORDINATE EXACT ELECTRICAL REQUIREMENTS AND LOCATIONS FOR ALL PLUMBING AND MECHANICAL EQUIPMENT WITH DIVISION 22 AND 23, RESPECTIVELY. PROVIDE FUSED DISCONNECTS AS REQUIRED BY MANUFACTURER'S RECOMMENDATIONS. PROVIDE NEUTRAL CONDUCTORS IF REQUIRED BY MANUFACTURER.
8. PROVIDE PULLSTRING IN ALL EMPTY CONDUITS.
9. MAINTAIN FIRE RESISTANCE RATING OF WALLS WITH PENETRATIONS AND RECESSED OUTLET BOXES. MAINTAIN SPACING PER IBC AND MANUFACTURER. PROVIDE UL FIRE STOPPING AS REQUIRED.

REVISIONS

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	01/10/2025	90% Progress Set

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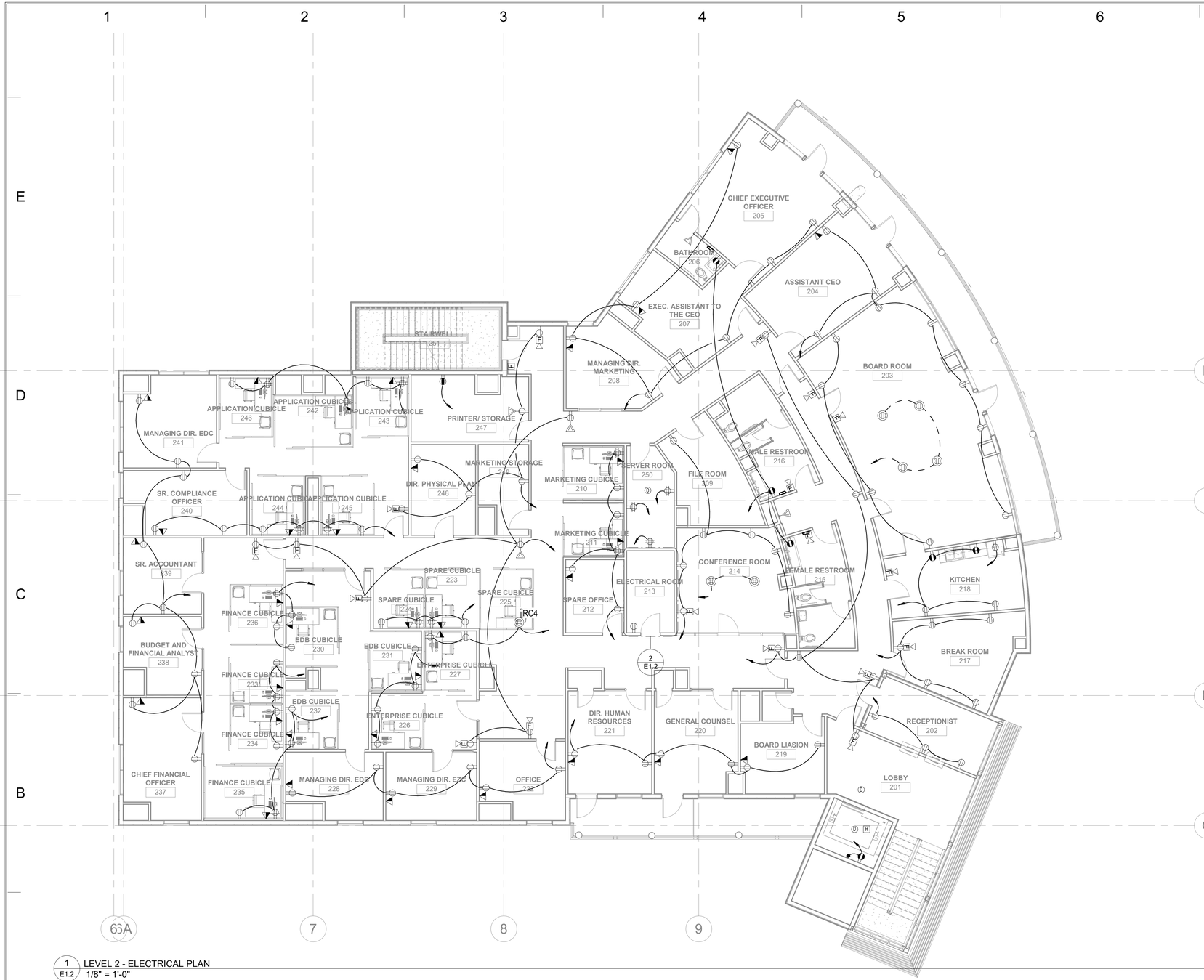
ELECTRICAL FLOOR PLAN - LEVEL 1



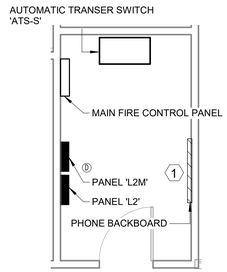
1 E1.2 LEVEL 2 - ELECTRICAL PLAN
1/8" = 1'-0"

GENERAL NOTES:
(APPLY TO THIS SHEET ONLY)

1. REFER TO DRAWING E0.00 FOR SYMBOLS LEGEND.
2. DRAWING IS FOR DESIGN INTENT ONLY. CONTRACTOR SHALL DETERMINE EXACT CONDUIT ROUTING IN THE FIELD WITH ALL OTHER TRADES.
3. COORDINATE EXACT LOCATION AND MOUNTING HEIGHTS OF ALL DEVICES WITH ARCHITECT. DEVICE AND COVERPLATE FINISHES TO BE SELECTED BY ARCHITECT.
4. COORDINATE EXACT LOCATION AND REQUIREMENTS OF LOW VOLTAGE AND SECURITY SYSTEMS WITH RESPECTIVE CONSULTANT AND ARCHITECT. PROVIDE ALL CONDUITS, JUNCTION BOXES, PULL BOXES, ETC. INDICATED ON CONSULTANT'S PLANS.
5. COORDINATE ALL THROUGH FLOOR STUB-UP LOCATIONS WITH STRUCTURE AND ARCHITECT. FLOOR PENETRATIONS SHALL BE FIRE SEALED AS REQUIRED.
6. VERIFY ELECTRICAL REQUIREMENTS AND RECEPTACLE NEMA TYPES FOR EQUIPMENT WITH EQUIPMENT SUPPLIER/SUBMITTAL PRIOR TO INSTALLATION.
7. COORDINATE EXACT ELECTRICAL REQUIREMENTS AND LOCATIONS FOR ALL PLUMBING AND MECHANICAL EQUIPMENT WITH DIVISION 22 AND 23, RESPECTIVELY. PROVIDE FUSED DISCONNECTS AS REQUIRED BY MANUFACTURER'S RECOMMENDATIONS. PROVIDE NEUTRAL CONDUCTORS IF REQUIRED BY MANUFACTURER.
8. PROVIDE PULLSTRING IN ALL EMPTY CONDUITS.
9. MAINTAIN FIRE RESISTANCE RATING OF WALLS WITH PENETRATIONS AND RECESSED OUTLET BOXES. MAINTAIN SPACING PER IBC AND MANUFACTURER. PROVIDE UL FIRE STOPPING AS REQUIRED.

LEGEND NOTES:
(APPLY TO THIS SHEET ONLY)

1. PROVIDE A 3/4" THICK, 8' HIGH PLYWOOD TELEPHONE BACKBOARD ACROSS WALL AS SHOWN. PROVIDE WITH (1) 12"X4" GROUNDING BUS BAR CONNECTED VIA A #2 GROUND WIRE TO BUILDING GROUND. PROVIDE GROUND CONNECTIONS TO LADDER RACK, ANTI-STATIC FLOOR, AND ALL EQUIPMENT IN ROOM (AS APPLICABLE). PAINT BACKBOARD WITH (2) COATS OF FLAME RETARDANT SEALANT TO MATCH WALL COLOR. PROVIDE (1) 2" CONDUIT WITH PULLSTRING TO CORE TELEPHONE ROOM ON THIS LEVEL. COORDINATE EXACT LOCATION WITH ARCHITECT/TENANT.



2 E1.2 LEVEL 2 - ELECTRICAL ROOM 213 ENLARGED PLAN
1/4" = 1'-0"

REVISIONS

NO.	DATE	REMARKS
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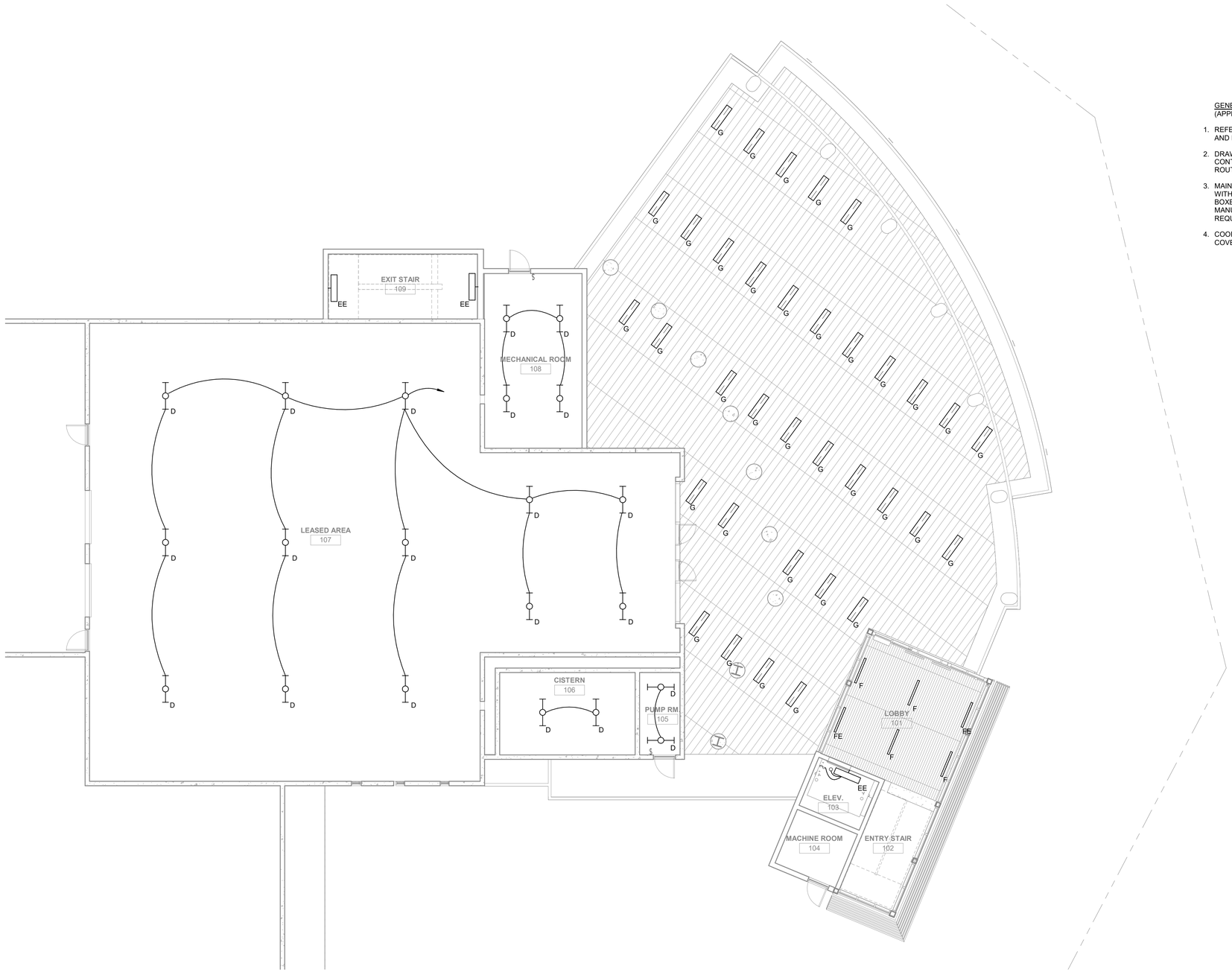
ELECTRICAL FLOOR PLAN - LEVEL 2



1 E2.1 LEVEL 1 - LIGHTING PLAN
1/8" = 1'-0"

GENERAL NOTES:
(APPLY TO THIS SHEET ONLY)

1. REFER TO DRAWING E0.00 FOR SYMBOLS LEGEND AND LIGHTING FIXTURE SCHEDULE.
2. DRAWING IS FOR DESIGN INTENT ONLY. CONTRACTOR SHALL DETERMINE EXACT CONDUIT ROUTING IN THE FIELD.
3. MAINTAIN FIRE RESISTANCE RATING OF WALLS WITH PENETRATIONS AND RECESSED OUTLET BOXES. MAINTAIN SPACING PER IBC AND MANUFACTURER. PROVIDE UL FIRE STOPPING AS REQUIRED.
4. COORDINATE COLOR/FINISH OF FIXTURES AND COVERPLATES WITH ARCHITECT.

REVISIONS

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LIGHTING FLOOR PLAN - LEVEL 1

1 E2.2 LEVEL 2 - LIGHTING PLAN
1/8" = 1'-0"

GENERAL NOTES:
(APPLY TO THIS SHEET ONLY)

1. REFER TO DRAWING E0.00 FOR SYMBOLS LEGEND AND LIGHTING FIXTURE SCHEDULE.
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LIGHTING FLOOR PLAN - LEVEL 2